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**SEDIMENT TRANSPORT CALCULATION  
IN TWO DIMENSIONS USING THE  
EINSTEIN'S PROCEDURE  
A CASE STUDY : THE ZAIRE RIVER**

by

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## SUMMARY

Several one dimensional models for sediment transport calculations in open channel flows have been developed. Einstein model (1950) is one of the most advanced method for the computation of bed-load and suspended-load, because the beginning and the end of the particle motion have to be expressed with the concept of probability, which relates instantaneous hydrodynamic lift forces to the particles weight.

The bed load equation expresses an equilibrium condition of the exchange of bed particles between the bed layer and the bed. This implies that the number of particles deposited per unit time and per unit bed area must be equal to the number of particles eroded per unit time and unit bed area.

Hence the goal of this thesis is limited to the study of the application of the Einstein model in the traditional way and in the two-dimensional approach based on measured hydraulic data of nine cross sections in the Zaire river; the results have been quite successful.

From the result obtained by the Einstein model, it was found that the calculated sediment transport rate underestimates the measured transport rate.

## C O N T E N T

1. INTRODUCTION	1
2. LITERATURE REVIEW	2
2.1. PROPERTIES OF SEDIMENT	2
2.1.1. Size and Shape	2
2.1.2. Fall Velocity	4
2.2. FORMS OF BED ROUGHNESS	5
2.2.1. Bed Configuration without Sediment Movement	5
2.2.2. Ripples	5
2.2.3. Dunes	6
2.2.4. Plane Bed with Sediment Movement	7
2.2.5. Antidunes	7
2.2.6. Chutes and Pools	7
2.3. BEGINNING OF SEDIMENT MOTION	8
2.3.1. Shields Diagram	9
2.4. BED LOAD TRANSPORT	11
2.4.1. Du Boys' formula	11
2.4.2. Meyer-Peter and Muller formula	12
2.4.3. Einstein-Brown formula	12
2.4.4. Bagnold's stream power approach	13
2.4.5. Schoklitsch formula	14
2.4.6. Shield's formula	14
2.4.7. Kalinske's formula	14
2.5. SUSPENDED LOAD TRANSPORT	15
2.5.1. Lane and Kalinske's approach	15
2.5.2. Brook's approach	16

2.5.3.	Bagnold's approach	16
3.	EINSTEIN MODEL FOR SEDIMENT TRANSPORTATION IN OPEN CHANNEL FLOWS	18
3.1.	APPROACH TO THE PROBLEM	18
3.2.	LIMITATION OF THE BED LOAD FUNCTION	19
3.2.1.	The undetermined function	19
3.2.2.	The alluvial stream	19
3.2.3.	The sediment mixture	20
3.3.	HIDRAULIC OF THE ALLUVIAL CHANNEL	21
3.3.1.	The friction formula	21
3.3.2.	The friction factor	24
3.3.3.	Resistance of the Bars	24
3.3.4.	The transition between hidraulically rough and smooth beds	27
3.4.	THE BED LOAD CONCEPT	27
3.5.	THE BED LOAD EQUATION	28
3.5.1.	The exchange time	30
3.5.2.	The exchange probability	30
3.5.3.	Determination of the probability	32
3.6.	SUSPENSION	44
3.7.	INTEGRATION OF THE SUSPENDED LOAD	48
3.8.	TRANSITION BETWEEN BED LOAD AND SUSPENDED LOAD	50
3.9.	THE TOTAL SEDIMENT TRANSPORT	52
4.	GENERAL DESCRIPTION OF THE SITES AND DATA	55
4.1.	GENERAL DESCRIPTION OF THE SITES	55
4.2.	GENERAL DESCRIPTION OF DATA	59

5.	APPLICATION OF EINSTEIN MODEL TO THE DATA OF ZAIRE RIVER	80
5.1.	TRADITIONAL EINSTEIN PROCEDURE	80
5.1.1.	Hydraulic calculations	80
5.1.2.	Sediment transport calculations	83
5.2.	TWO-DIMENSIONAL APPROACH	88
5.2.1.	Calculation of mean velocity	88
5.2.2.	Calculation of shear velocity	88
5.2.3.	Calculation of sediment transport rate	89
5.3.	GENERAL DESCRIPTION OF COMPUTER PROGRAMS	89
6.	RESULTS	94
7.	DISCUSSION OF RESULTS	116
8.	CONCLUSION	120
	REFERENCES	122
	LIST OF FIGURES	123
	LIST OF SYMBOLS	125
	APPENDIX	128

## 1. INTRODUCTION

One of the most difficult problems encountered in open channel hydraulics is the determination of the rate of movement of the bed material by a stream. The movement of bed material is a complex function of flow duration, sediment supply, and channel characteristics.

Every sediment particle which passes a particular cross section of the stream must satisfy the following two conditions: (1) it must have been eroded somewhere in the watershed above the cross section; (2) it must be transported by the flow from the place of erosion to the cross section.

Each of these two conditions may limit the sediment rate at the cross section, depending on the relative magnitude of two controls: The availability of the material in the watershed and the transporting ability of the stream.

The goal of this thesis does not attempt to give the specific solutions for all sediment problems in alluvial channels. It attempts only to study the application of EINSTEIN MODEL (1950) to nine cross sections in the ZAIRE RIVER, using the traditional procedure and the two dimensional approach.

The Zaire river is the second largest river in the world and the most important river in Africa. It is used as a mean of transportation interconnecting different important cities in central Africa. These sediment studies will help in estimating the amount of sediment transported in the river and later will mitigate the navigation problems.

## 2. LITERATURE REVIEW

### 2.1 PROPERTIES OF SEDIMENT

#### 2.1.1 SIZE AND SHAPE

Sediment are broadly classified as cohesive and noncohesive. Noncohesive sediment particles react to fluid forces and their movements are affected by the physical properties of the particles such as size, shape, specific weight, density and fall velocity.

It is necessary to make statistical analysis of these characteristics, as determined from the adequate number of samples of sufficient size in order to define fully the representative particle characteristics of any sediment mixture as a whole. The statistical analysis may consist of frequency curves, quartile and moment measures.

The most commonly used method to determine size frequency is mechanical or sieve analysis. In general the results are presented as cumulative-size frequency curves. The fraction or percentage by weight of a sediment that is smaller or larger than a given size is plotted against particle size. From the size frequency curve it is possible to obtain:

D35, The size of sediment for which 35 percent of the sample is finer. This size was specified by EINSTEIN as a representative grain size of the sediment mixture.

D40, The size used by SCHOKLITSCH, to represent the mixture.

D50, The median diameter. SHIELDS used this size in



his analysis of beginning of motion.

D65, The size used by EINSTEIN to express the roughness of a sediment mixture. It is also used by SENTURK.

D70, The size chosen by MEYER-PETER and MULLER to represent the roughness of a sediment mixture.

D85, The size used by SIMONS and RICHARDSON in the formula for the computation of the resistance to flow in sand bed channels.

D84.1 and D15.9, The sizes derived from a probabilistic analysis.

The sediment size frequency distribution is essentially a probabilistic approach used to help describe the transported sediment and the sediment mixture forming the bed of a river. The distribution of particle size usually follows the standardized normal (Gaussian) distribution.

$$F(u) = \int_{-\infty}^u \phi(u) \cdot du \quad \dots\dots\dots (2.1a)$$

$$f(u) = \frac{1}{\sqrt{2\pi}} e^{-z^2/2} \quad \dots\dots\dots (2.1b)$$

EINSTEIN, (1950) used this function to help explain the motion of particles on the bed of an alluvial channel. The use of normal probability paper provides a means of making the transformation. The characteristics of the cumulative distribution function are such that between the ordinates 0.8413 and 0.1587 the normal distribution can be approximated by a straight line. The fifty percent value of the ordinate is midway between these two values. The diameter corresponding to the fifty percent ordinate is called the

geometric mean size.

Particle shape may be considered statistically as a distribution of the geometric properties of the grain, rather than a cataloging of the individual shape of each particle. Various schemes have been devised to take measurements and present data are useful in defining particle shape.

Shape factor (sphericity or roundness) can be computed for bulk sediments and presented as frequency curves when either moment or quartile measures are used to describe the shape factor of the sediment.

### 2.1.2 FALL VELOCITY

The primary variable defining the interaction of sediment transport with the bed, banks or suspended in the fluid is the fall velocity of sediment particles.

A particle falling at terminal velocity in a quiescent fluid is driven by the resulting force considering the particle buoyant weight and the resisting force resulting from fluid drag. The general drag equation is:

$$F_D = C_D \cdot f \cdot A \cdot W^2 / 2 \quad \dots\dots\dots (2.2)$$

Where:  $F_D$  , is the drag force

$C_D$  , is the drag coefficient

$A$  , projected area of the particle in the direction of fall.

For natural sands with the Reynolds number less than 0.1, the coefficient of drag ( $C$ ), is independent of the shape factor and equal to  $24/Re$ ; where  $Re$  is the Reynolds

number of the particle.

For spheres the equation for fall velocity according to STOKES is:

$$W = \frac{(\rho_s - \rho) \cdot g \cdot D^2}{18 \cdot \mu} \quad \dots\dots\dots (2.3)$$

Where:  $\rho_s$  ; is the density of the particle.

W ; the fall velocity

$\mu$  ; the Dynamic Viscosity

D ; the particle diameter.

When Reynolds number is greater than 0.1, the equation for fall velocity is:

$$W = \frac{4 \cdot g \cdot (\gamma_s - \gamma) \cdot D}{3 \cdot C_D \cdot \gamma} \quad \dots\dots\dots (2.4)$$

Where:  $\gamma_s$  ; is the specific weight of the particle.

$\gamma$  ; is the specific weight of the water.

## 2.2. FORMS OF BED ROUGHNESS

### 2.2.1 BED CONFIGURATION WITHOUT SEDIMENT MOVEMENT

Plane bed without movement has been studied to determine the flow conditions for the beginning of motion and the bed profiles that would form after beginning of motion. In general, SHIELD'S (1936) relation for the incipient motion of sediment is adequate.

With plane bed without bed material transport the values of CHEZY'S discharge coefficient range from 15 to 20 and the MANNING'S roughness coefficient vary from 0.012 to 0.016.

### 2.2.2 RIPPLES

In fine sand, ripples will usually occur as soon as particle show appreciable movement. SIMONS (1959) and KNORZ (1959) observed that the length of the separation zone when the bed form was either ripples or dunes was about 10 times the height of the ripple or the dune. The separation zone downstream from a ripple cause very little, if any disturbance on the water surface. The concentration of sediment is small ranging from 10 to 200 ppm. Resistance to flow is large with the Manning's coefficient varying from 0.018 to 0.050 and the resulting discharge coefficient  $C/\sqrt{g}$  ranges from 6 to 12. As the depth of flow increases, resistance to flow due to bed roughness decreases.

### 2.2.3 DUNES

When the shear stress ( $\tau_{RS}$ ) and stream power ( $\Omega_{RS}$ ) are gradually increased over a bed of ripples, or if the bed material is coarser than 0.6 mm. over a plane bed, a new flow condition will be achieved that causes dunes to form. Dunes cause large separation zones in the flow. These zones, in turn, cause large boils to form on the surface of the stream. The sediment transport rate is relatively small. The concentration of bed materials ranges from approximately 100 to 1200 ppm.

For dunes, the Manning's coefficient ( $n$ ) varies from 0.018 to 0.035 and the discharge coefficient  $C/\sqrt{g}$  varies from 8 to 12-15. Resistance to flow increases with an increase in

depth for coarser sand ( $D_{50} > 0.3$  mm.) and decreases with an increase in depth for finer sands ( $D_{50} < 0.3$  mm.).

#### 2.2.4 PLANE BED WITH SEDIMENT MOVEMENT

If the shear stress or the stream power is continuously increased, the size of dunes increases until the dunes reach a maximum height at a certain stream power. Finally, the dunes completely disappear and a flat bed is formed. In this case, the concentration of bed material ranges from about 1500 to 3000 ppm.

#### 2.2.5 ANTIDUNES

When the shear stress or the stream power is further increased, water and sand waves gradually build up, from a plane bed and from a plane water surface the waves may grow in height until they become unstable and break like the sea surface or they may gradually subside and subsequently reform. As the antidunes form and increase in height, they may move upstream, downstream, or remain stationary. Their upstream movement led GILBERT (1914) to name them antidunes. This is the positive indication that the local flow is rapid ( $Fr > 1$ ). KENNEDY (1961), studied the antidune flow and found that the wave length of antidunes is given by  $L = 2\pi U^2/g$  where:  $U$  is the mean velocity and  $g$  the gravity.

#### 2.2.6 CHUTES AND POOLS

At very steep slope, sand-bed channel flow changes to chutes and pools. Chutes and pools flow could not be attained using coarser sand because the steeper slope

required could not be set up in the flume. This type of flow consisted of a log chute (3.048-9.144 mm.) in which the flow accelerate rapidly, a hydraulic jump at the end of the chute, and then a long pool (3.048-9.144 mm.) in which the flow was tranquil but was accelerating. The chutes and pools moved upstream at velocities of about 0.3-0.6 m/min. The elevation of the sand bed varied within wide limits, although at no time was the flume floor exposed. Resistance to flow was large, and  $C/\sqrt{g}$  ranged from 9 to 16. This type of flow was frequently accompanied by a decrease in the mean velocity of the flow in the flume even though there was an increase in stream power.

### 2.3 BEGINNING OF SEDIMENT MOTION

When the drag force is less than some critical value the bed material of a channel remain motionless. Then the bed can be considered as rigid. But when the shear stress over the bed attains or exceeds its critical value, particle motion begin.

KRAMER (1965), has defined three types of motion of bed material:

- (1) weak movement: only a few particles are in motion on the bed. The grain "moving on one square centimeter of the bed can be counted"
- (2) Medium movement: the grain of mean diameter begin to move. The motion is not local in character but the bed continues to be plane.
- (3) General movement: All the mixture is in motion; "the movement is occurring in all parts of the

bed at all times".

### 2.3.1 SHIELDS DIAGRAM

Many experiments have been conducted to develop an explicit solution of theoretical equation:

$$\frac{\rho U_*^3}{\gamma_s' D_s} = f(U_*^3 D_s / \nu) \quad \dots \dots \dots (2.5)$$

Where:  $\gamma_s' = (\gamma_s - \gamma)$

$\gamma_s$  = specific weight of sediment

$\gamma$  = specific weight of water

$D_s$  = characteristic diameter of the particle

$U_*$  = shear velocity at the threshold condition.

The earliest one is the graphical presentation given by Shields (1936) is widely accepted and  $\tau_c / (\gamma_s - \gamma) D_s$  is often referred to as Shields parameter. ( $\tau_c$  = critical shear stress)

Shields determined this relationship by measuring bed load transport for various values of  $\tau_c / (\gamma_s - \gamma) D_s$ .

Shields diagram can be divided in three regions:

REGION 1:  $U_*^3 D_s / \nu < 3.63 - 5.0$

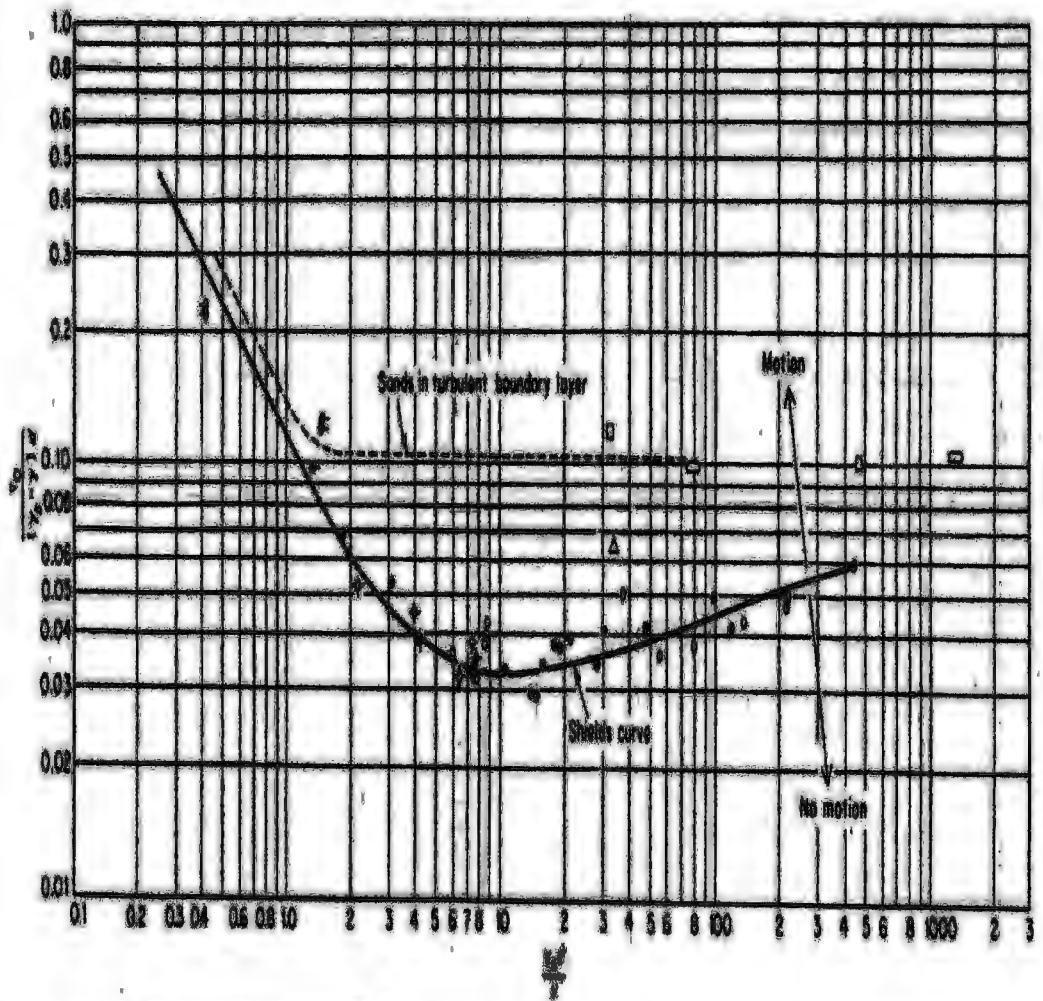
In this region  $D_s < 3\delta$ , and the boundary is considered hydraulically smooth ( $\delta$  is the thickness of the laminar boundary layer).

REGION 2:  $3.63 - 5.0 < U_*^3 D_s / \nu < 68.0 - 70.0$

In this region the boundary is in a transitional state and  $\delta/3 < D_s < 6\delta$

REGION 3:  $U_*^3 D_s / \nu > 70 - 500$

In this region the boundary is completely rough and:



Fully developed turbulent velocity profile

Sym	Description	$\rho_s, g/cm^3$
o	Amber	1.06
•	Lignite	1.27
•	Granite (Shields)	2.7
•	Quartz	4.25
•	Sand (Coody)	2.65
•	Sand (Kramer)	2.65
•	Sand (U.S.W.E.S.)	2.65
•	Sand (Gilbert)	2.65

Turbulent boundary layer

•	Sand (Vansoni)	2.65
•	Glass beads (Vansoni)	2.49
o	Sand (White)	2.65
o	Sand in air (White)	2.49
Δ	Steel shot (White)	7.9

Fig. 2-1 Shields' diagram; dimensionless critical shear stress vs. shear Reynolds number. [After VANSONI (1956).]



$$F* = \frac{\tau_c}{(\gamma_s - \gamma) D_s} = 0.06$$

F\* = dimensionless shear stress

## 2.4 BED LOAD TRANSPORT

When the flow over movable boundaries of a channel has hydraulic conditions exceeding the critical condition for motion of the bed material, sediment transport will start. If the motion of entrained particles is one of rolling, sliding, and sometimes jumping in the bed layer, this kind of sediment transport is commonly referred to as bed-load transport or contact load.

### 2.4.1 DU BOYS' FORMULA (1879)

Advances in the theory of sediment transportation may be assumed to have taken place with the introduction of the tractive force formula by Du Boys. SCHOKLITSCH (1914), proved Du Boys model of sliding layers to be wrong, but his experimental data could be well represented by Du Boys' equation. O'BRIEN and RINDLAUB (1934) generalized Du Boys' equation:

$$q_b = K \tau_o (\tau_o - \tau_c) \dots\dots\dots (2.6)$$

Where:  $q_b$  = Volume transport rate of the bed load per unit width (Kg/m.sec)

K = Parameter considered to depend largely upon sediment characteristics.

$\tau_o$  = Unit tractive force exerted by the flow on the bed of a wide channel (Kg/m<sup>2</sup>)

$\tau_c$  = Critical shear stress (Kg/m<sup>2</sup>).

#### 2.4.2 MEYER - PETER AND MULLER FORMULA

Meyer - Peter and Muller (1948), based on experiments with sand particles of uniform sizes, natural gravel, lignite and barita developed a formula similar in the form to the Du Boys' formula.

$$q_b = \frac{(\gamma_s)(g)^{1/2}}{\gamma_s - \gamma} \frac{1}{\gamma^{1/2}} \left[ \frac{0.0661 (Q') (D90)^{1/6} d \cdot S - 0.0076 (\gamma_s - \gamma) D_m}{Q} \right]^{3/2} \dots (2.7)$$

Where:  $q_b$  = Sediment discharge in Ton/day.ft

$\gamma_s$  = Specific weight of particle

$\gamma$  = Specific weight of water

$g$  = gravity

$D90$  = Particle size at which 90% of the bed material is finer (m)

$D_m$  = Effective diameter of sediment (mm)

$Q'$  = Water discharge determining bed load transport

$Q$  = Total water discharge

$n_b$  = Roughness coefficient

$d$  = Water depth

$S$  = Energy slope.

#### 2.4.3 EINSTEIN - BROWN FORMULA

This formula is a modification developed by BROWN (1950) of a formula by EINSTEIN (1942).

$$\phi = f(1/\psi) \dots \dots \dots (2.8)$$

Where:

$$\phi = \frac{q_b}{K_s K \sqrt{g \left( \frac{\rho_s}{\rho} - 1 \right) D_s^3}} \dots\dots\dots (2.8a)$$

$$\frac{1}{\psi} = \frac{\tau}{(\rho_s - \rho) D_s} \dots\dots\dots (2.8b)$$

$$K = \sqrt{\frac{\rho}{3} + \frac{36 \phi^2}{g D_s^3 \left( \frac{\rho_s}{\rho} - 1 \right)}} = \sqrt{\frac{36 \phi^2}{g D_s^3 \left( \frac{\rho_s}{\rho} - 1 \right)}} \dots\dots (2.8c)$$

- where:  $1/\psi$  = Shields parameter  
 $q_b$  = Sediment discharge per unit width  
 $\phi$  = Transport rate function  
 $K_s$  = Sediment specific weight  
 $\rho$  = Water specific weight  
 $D_s$  = Representative diameter of the sediment  
 $\nu$  = Kinematic viscosity of water  
 $\tau$  = Shear stress

**2.4.4 BAGNOLD'S STREAM POWER APPROACH**

One of the most recent methods relating the work of energy expenditure of a stream and the quantity of sediment transported by the flow is that of BAGNOLD (1966).

$$q_b = A_1 \cdot B_1 \cdot K_1 \cdot \nu^{0.5} \sqrt{\cos \beta} (\theta_p - \theta_c) \cdot \theta_p^{0.5} \dots\dots (2.9)$$

- In which:  $A_1, B_1$  and  $K_1$  are dimensionless constant  
 $\theta_p$  = dimensionless tangential shear stress  
 $\theta_c$  = dimensionless tangential shear stress in which sediment particle begin to move on rippled sand bed.

$\beta$  = inclination of the bed surface

2.4.5 SCHOKLITSCH (1914)

$$q_b = K^1 (\tau_o - \tau_c)^m \dots\dots\dots(2.10)$$

Where:  $K^1$  and  $m$  are a function of the median diameter of sediment ( $1.5 < m < 1.8$ )

$\tau_o, \tau_c$ , the same defined in Du Boys formula.

2.4.6 SHIELDS (1936)

Proposed from his experimental results a dimensionally homogeneous transport function of the form:

$$\frac{q_b \gamma_s}{9.5 \delta} = \frac{10 (\tau_o - \tau_c)}{(\gamma_s - \delta) D_s} \dots\dots\dots (2.11)$$

with:  $q_b$  = Sediment discharge per unit width

$q$  = Water discharge per unit width

$\tau_o$  = Intensity of bed shear

$\tau_c$  = Critical shear stress

$D_s$  = Representative diameter

$\gamma$  = specific weight of water

$\gamma_s$  = specific weight of sediment

$\delta$  = slope of the energy gradient.

2.4.7 KALINSKE (1942)

$$q_b = \alpha \cdot P \cdot D_s \cdot \bar{u}_s \dots\dots\dots (2.12)$$

Where:  $\alpha$  = shape factor for packing ( $\alpha = 2/3$  for uniform spheres)

- $P$  = the fraction of bed covered by particles  
 $D_s$  = representative diameter  
 $\bar{u}_s$  = the time average velocity of  $u$   
 $q_b$  = sediment discharge per unit width.

Kalinske developed a bed equation considering turbulent fluctuations of flow. He first assumed that the velocity of a sediment grain moving on the bed is given by:

$$u_s = b(u - U_c)$$

Where:  $u$  = is the instantaneous fluid velocity

$U_c$  = the critical velocity for incipient motion

$b$  = is a constant close to one

## 2.5 SUSPENDED LOAD TRANSPORT

A part of the sediment transported by the flow in streams is suspended in the flow. The weight of these sediment particles is continuously supported by the fluid. Turbulence is the most important factor in the suspension of sediment.

Most of the earlier studies concerned with the suspension of sediment are unacceptable when considered in relation to the present state of knowledge of fluid motion.

### 2.5.1 LANE AND KALINSKE'S APPROACH (1941)

Lane and Kalinske (1941), concluded that the relation appears to be sufficiently accurate for practical purposes is

$$q_s = q \cdot C_a \cdot P_b \cdot e^{15. W \cdot a / (d \cdot U^3)} \quad \dots \dots \dots (2.13)$$

where:  $q_s$  = suspended bed material discharge per unit width

$C_a$  = concentration by weight

$a$  = level of reference above the bed

$q$  = water discharge per unit width

$W$  = fall velocity of the suspended sediment

$d$  = water depth

$F_c$  = function of  $(W/U_*')$

$U_*'$  = shear velocity

### 2.5.2 BROOKS' APPROACH

By assuming the velocity defect relation and a concentration distribution, Brooks (1963) obtained the following equations:

$$q = D_c q \left[ 1 + \frac{U_*'^2}{K_c U_*'^2} \int_0^1 \frac{(1-y)^{2k_c}}{y} dy + \frac{U_*'^2}{K_c U_*'^2} \int_0^1 \frac{(1-y)^{2k_c}}{y} \ln y dy \right] \dots (2.14)$$

$C$  = concentration at  $y = d/2$

$U_*'$  = shear velocity

$U$  = mean velocity

$q$  = discharge per unit width

$q_s$  = suspended sediment discharge per unit width

$k_c, k_1$  = constants

$E$  =  $a/d$  ( $a = 2D$ )

$d$  = water depth

$D$  = diameter of particle

### 2.5.3 BAGNOLD'S APPROACH (1966)

Bagnold noted that the sediment transport rate could be

equated to a work rate. The suspended load work equation is:

$$\frac{(\delta_s - \delta)}{\delta} q_s = (1 - e_b) e_s \tau_o U \frac{\bar{u}_s}{w} \dots\dots\dots (2.15)$$

Where:  $q_s$  = suspended load discharge per unit width

$\bar{u}_s$  = the mean transport velocity of solids  
moving in suspension

$e_s$  = suspended load transport efficiency

$e_b$  = bed load transport efficiency

$\tau_o U$  = stream power

$w$  = settling velocity

### 3 EINSTEIN MODEL FOR SEDIMENT TRANSPORTATION IN OPEN CHANNEL FLOWS

#### 3.1 APPROACH TO THE PROBLEM

The term "Bed - load Function" has proved to be useful in the description of the sediment movement in stream channels. It gives the rates at which flows of any magnitude in a given channel will transport the individual sediment sizes of which the channel bed is composed.

The bed load function developed by EINSTEIN (1950), is based on a large amount of experimental evidence on the existing theory of turbulent flow, and beyond the limits of existing theory, on reasonable speculation. Some universal constant of the various transportation equations were obtained from flume experiments. Some terms which Einstein recured frequently are defined as follows:

**Bed Load:** Bed particles moving in the bed layer. This motion occurs by rolling, sliding, and sometimes by jumping.

**Suspended Load:** particles moving outside the bed layer. The weight of suspended particles is continuously supported by the fluid.

**Bed Layer:** A flow layer, 2 grain diameter thick, immediately above the bed. The thickness of the bed layer varies with the particle size.

**Bed material:** The sediment mixture of which the moving bed is composed.

**Wash Load:** That part of the sediment load which consists of grain sizes finer than those of the bed.

**Bed Material Load:** That part of the sediment load which



consists of grain sizes represented in the bed.

**Bed Load Function:** The rate at which various discharges will transport the different grain sizes of the bed material in a given channel.

**Bed Load Equation:** The general relationship between bed load rate, flow condition, and composition of the bed material.

### 3.2 LIMITATION OF THE BED LOAD FUNCTION

#### 3.2.1 THE UNDETERMINED FUNCTION

Functions often become constant or even equal to zero under a wide range of conditions. Such functions may not have any value in certain ranges of conditions and is mathematically demonstrated wherever the solution of the equation which defines the function becomes imaginary. Functions that become indeterminate under a wide range of conditions seem to be rather unusual. Unfortunately, the bed load function has this character. The critical parameter deciding the significance of the function in a given flow is the the grain diameter of the sediment.

#### 3.2.2 THE ALLUVIAL STREAM

Experience in concrete-lined channel shows that sediment up to a certain particle size may be fed into such a flow at any rate up to a certain limit without causing any deposits in the channel. An observer who examines the channel after the flow has passed can state only that the rate of sediment flow must have been below this limiting rate; that

is, below the "SEDIMENT TRANSPORT CAPACITY" of the concrete channel. If the rate of sediment supply is larger than the capacity of the channel to move it, the surplus sediment drops out and begins to cover the channel bottom. More and more sediment is dropped if the supply continues to exceed the capacity until the channel profile is sufficiently changed to reach an equilibrium whereby at every section the transport is just reduced to the capacity value.

### 3.2.3 THE SEDIMENT MIXTURE

This problem is highly complicated by the fact that the sediment entering any natural river reach is never uniform in size, shape, and specific gravity but represents always a rather complex mixture of different grain types. It has been found experimentally that the shape of the different sediment particles with few exceptions is much less important than the particle size. As the derivations which follow do not introduce any molecular forces between sediment particles, they are automatically restricted to the larger particles, in general to those coarser than 250 mesh sieve (Tyler scale) or 0.061 millimeter in diameter. Again, beginning with the assumption of a flow in a concrete channel. Assume a sediment supply at the upper end of the channel, consisting of all different sizes from a maximum size down through the silt and clay range. If the maximum grain is not too large to be moved by the flow, the channel will again stay clear at low rates of sediment supply. But an increase of the supply rate will eventually cause sediment deposition.

Under most conditions only the coarse sizes of sediment will be deposited. It is true that a small percentage of the finer sediments may be found between the larger particles when the flow is fast, but this amount is generally so small that one is tempted to conclude that small particles are caught accidentally between the larger ones rather than primarily deposited by the flow itself.

Another factor influencing the bed-load function is the shape of the channel cross section. If this section is not influenced either structurally or by vegetation it is only a function of sediment and of the flow.

### 3.3 HYDRAULIC OF THE ALLUVIAL CHANNEL

The transport of bed sediment in such a reach always equal its capacity to transport such sediment. It is easy to conclude from this that the flow is uniform or at least nearly so. The open channel hydraulics of nonuniform flow or the calculation of backwater curves is not particularly important in this connection. Where such calculations are necessary for channels that are very actively aggrading or degrading, they are based on the use of the Bernoulli equation as it is applied to channels with solids beds.

#### 3.3.1 THE FRICTION FORMULA

The hydraulics of uniform flow include basically the description of the velocity distributions and of the frictional loss for turbulent flow. EINSTEIN found that the velocity distribution in open channel flow over a sediment bed is best described by the logarithmic formulas based on  $V$ .

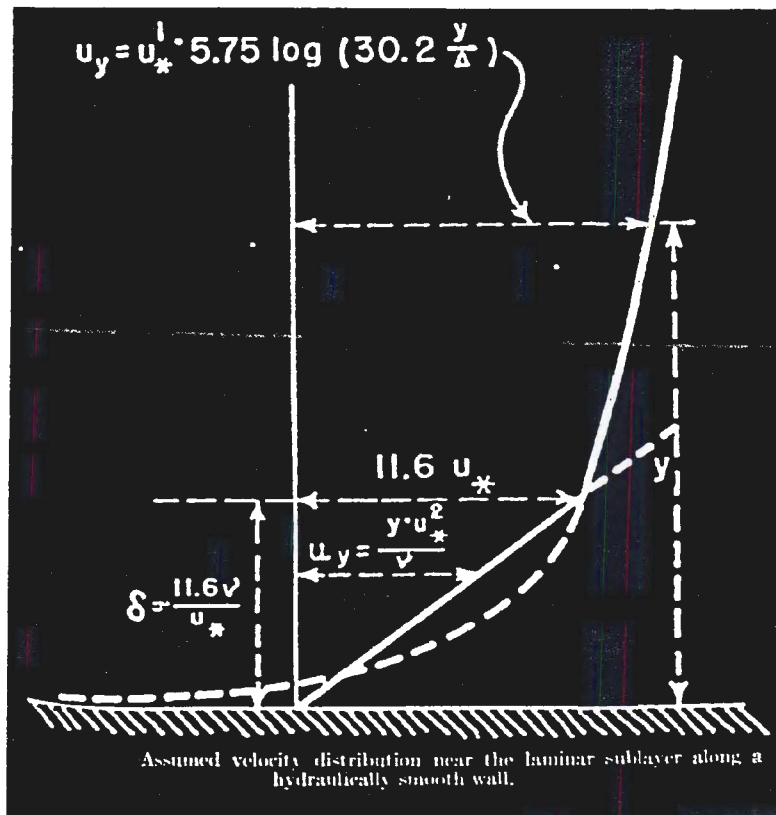


FIGURE 3.1

KARNAN'S similarity theorem with the constants as proposed by KELLEBAN. He gives the vertical velocity distribution as:

$$\frac{\bar{v}}{u_*} = 5.58 + 5.75 \operatorname{Log} \left( \frac{y - \delta_0}{\delta} \right) = 5.75 \operatorname{Log} \left( \frac{7.05 y - \delta_0}{\delta} \right) \dots \quad (3.1)$$

The equation (3.1) is applied for smooth boundaries and the equation (3.2) for hydraulically rough boundaries.

$$\frac{\bar{v}}{u_*} = 5.58 + 5.75 \operatorname{Log} (y/K_s) = 5.75 \operatorname{Log} (39.2 y/K_s) \dots \quad (3.2)$$

The transition between the two, including the rough and smooth conditions, may all be combined in the form:

$$\frac{\bar{v}}{u_*} = 5.75 \operatorname{Log} \left( 39.2 \frac{y - \delta}{K_s} \right) = 5.75 \operatorname{Log} \left( 39.2 \frac{y}{\Delta} \right) \dots \quad (3.3)$$

Herein are:

$\bar{v}$  = the average point velocity at distance  $y$  from the bed

$u_* = \sqrt{\tau_0/\rho} = \sqrt{S.R.g}$ , the shear velocity

$S$  = the slope of the energy grade line

$\rho$  = the density of the water

$R$  = the hydraulic radius

$g$  = the acceleration due to gravity

$y$  = the distance from the bed

$x$  = a corrective parameter (read from figure 3.2)

$\Delta = K_s/x$ , the apparent roughness of the surface

$\delta = 11.6 \nu / u_* g$  the thickness of the laminar sublayer of a smooth wall.

$\nu$  = the kinematic viscosity of water

### 3.3.2 THE FRICTION FACTOR

For uniform sediment,  $K_s$  is equal to the grain diameter as determined by sieving. Comparative flume experiments have shown that the representative grain diameter of a sediment mixture is given by the sieve size of which 65 percent of the mixture (by weight) is finer.

A sediment bed in motion usually does not remain flat and regular but shows ripples or bars of various shapes and sizes. These irregularities have some effect on the roughness of the bed.

### 3.3.3 RESISTANCE OF THE BARS

EINSTEIN has described a method by which the influence of side-wall friction on the results of bed-load experiments may be estimated. It is assumed that on such a bed friction develops in two distinctly different ways: (1) along the sediment grains of the surface as a rough wall with the representative grain diameter equal to  $K_s$  and in addition (2) by the separation of the flow from the surface at characteristic points of the ripples or bars.

Einstein justified in dividing the cross-sectional area into two parts. One will contribute the shear which is transmitted to the boundary along the roughness of the grainy sand surface. The other part will contribute the shear transmitted to the wall in the form of normal pressures at the different sides of the bars. These may be designated  $A'$  and  $A''$  respectively. Both types of shear action are more or

less evenly distributed over the entire bed surface and act, therefore, along the same perimeter. Two hydraulic radius may be defined as  $R' = A'/P$  and  $R'' = A''/P$ , where naturally; the total hydraulic radius is:

$$R = R' + R'' \quad \dots\dots\dots (3.4)$$

This entire procedure of division may appear to be rather artificial since both actions occur along the same perimeter. The significance of this division becomes apparent, however, when one recalls that the transmission of shear to the boundary is accompanied by a transformation of flow energy into energy of turbulence. This energy transformation caused by the rough wall occurs at the grains themselves. This newly created turbulence stays at least for a short time in the immediate vicinity of the grains and, as will be shown later, has a great effect on the bed-load motion. The part of the energy which corresponds to the shape resistance is transformed into turbulence at the interface between wake and free stream flow, or at a considerable distance away from the grains. This energy does not contribute to the bed-load motion of the particles. Therefore, it may be largely neglected in the entire sediment picture. This may explain why the division of the shear into the two parts  $u_*'$  and  $u_*''$  is of first importance.

$$u_*' = \sqrt{S \cdot R' \cdot g} \quad \dots\dots\dots (3.5)$$

$$u_*'' = \sqrt{S \cdot R'' \cdot g} \quad \dots\dots\dots (3.6)$$

From this it is understandable that the velocity

distribution near a sediment grain in the bed surface is described by equations (3.1), (3.2) and (3.3) whereby  $u_*$  assumes the value of  $u_*'$ . The average velocity in the vertical may be determined according to KEULEGAN for hydraulically smooth bed as:

$$\frac{\bar{u}}{u_*'} = 3.25 + 5.75 \text{ Log} \left( \frac{R' \cdot u_*'}{K_s} \right) = 5.75 \text{ Log} \left( 3.67 \frac{R' \cdot u_*'}{K_s} \right) \dots (3.7)$$

And for a hydraulically rough bed:

$$\frac{\bar{u}}{u_*'} = 4.25 + 5.25 \text{ Log} (R' / K_s) = 5.75 \text{ Log} (12.27 R' / K_s) \dots (3.8)$$

The entire transition between the two cases inclusive of the extremes may be expressed by:

$$\frac{\bar{u}}{u_*'} = 5.75 \text{ Log} \left( 12.27 \frac{R' \cdot x}{K_s} \right) = 5.75 \text{ Log} \left( 12.27 \frac{R'}{K_s} \right) \dots (3.9)$$

where  $x$  is the same function of  $K_s / u_*'$ , as given in figure 3.2 and  $\phi = 31.6 \bar{u} / u_*'$ .

A corresponding expression  $u/u_*'$  may be calculated, and this expression must be expected to be a function of the ripple or bar pattern, basically corresponding to equation (3.9). The ripples and bars change considerably and consistently with different rates of sediment motion on the bed.



### 3.3.4 THE TRANSITION BETWEEN HYDRAULICALLY ROUGH AND SMOOTH BEDS

Einstein and El-Samni experimentally showed that the theoretical boundary from which the distance  $y$  of equation (3.3) must be measured  $0.2K_s$  below the plane which connects the most prominent points of the roughness protrusions. It is generally known, that the wall acts hydraulically rough if  $K_s/\delta > 5$ . The laminar sublayer as calculated for a smooth wall has then, a thickness of  $\delta < K_s/5$ .

### 3.4 THE BED LOAD CONCEPT

Bed-Load motion has been studied principally in laboratory flumes under conditions where suspension may be neglected. EINSTEIN has shown that the motion of the particles is fully governed by statistical laws which can be stated as follows:

1.- The probability of a given sediment being moved by the flow from the bed surface depends on the size of the particle, shape and weight and on the flow pattern near the bed.

2.- The particles move if the instantaneous hydrodynamic lift force overcomes the particle weight.

3.- Once in motion the probability of the particle being redeposited is equal in all points of the bed where the local flow would not immediately remove the particle again.

4.- The average distance travelled by any bed-load

particle between consecutive points of deposition in the bed is a constant for any particle and is independent of the flow condition, the rate of transport and the bed composition.

5.- The motion of bed particles by saltation as described by BAGNOLD, may be neglected in water, as proved by KALINSKE.

6.- The disturbance of the bed surface by moving sediment particles may be neglected in water.

### 3.5 THE BED LOAD EQUATION

The bed-load equation by definition is the equation which relates the motion of the bed material per unit width of bed layer to the local flow. For each unit of time and of bed area the same number of a given type and size of particles must be deposited in the bed as are scoured from it. Let  $q_b$  equal the rate at which bed load moves through the unit width of cross section and let  $i_b$  equal the fraction of  $q_b$  in a given grain size or size range. Thus  $q_b \cdot i_b$  is the rate at which the given size moves through the unit width per unit of time. All the particles with a particular diameter  $D$  are just performing an individual step of  $100 \cdot D$  or more generally, of  $A_L \cdot D$  length ( $A_L =$  constant of the bed-load unit-step). When they pass through the particular cross section where  $q_b$  is measured, it is not known what part of  $A_L \cdot D$  the individual particles have already travelled. They must be assumed to be deposited anywhere from zero to  $A_L \cdot D$  downstream of the section. The area of deposition is  $A_L \cdot D$  long and has unit width. If  $q_b$  is measured in dry weight per unit time and width and if  $A_L \cdot D^3$  is the volume of a

particle, ( $A_2 =$  constant of grain volume),  $\gamma_s$  its density, and  $g$  the acceleration of gravity the number of such particles deposited per unit time in the unit of bed area may be expressed as:

$$\frac{q_b \cdot i_b}{A_L \cdot D \cdot A_2 \cdot D^3 \cdot f_s \cdot g} = \frac{i_b \cdot q_b}{A_2 \cdot A_L \cdot g \cdot f_s \cdot D^4}$$

The rate at which sediment particles of this size are eroded from the bed per unit of time is proportional to the number of particles exposed at the bed surface per unit of area and to the probability  $p_s$  of such a particle being eroded during a second. If  $i_b$  is the fraction of the bed sediment in the given size range it may be assumed that this represents also the fraction of the surface covered by particles in the same size. The number of the particles  $D$  in a unit area of bed surface is:  $i_b / A_1 \cdot D^3$ ; and the number of particles eroded per unit area and time is:  $i_b \cdot p_s / A_1 \cdot D^3$ .

In the time  $t_1$ , necessary to replace a bed particle by a similar one were known, the probability of removal  $p_s$  per second could be replaced by the absolute probability  $p$  to be exchanged as  $p_s \cdot t_1 = p$ . Thus it follows that  $p_s$  is the number of exchanges per second,  $t_1$  the time consumed by each exchange, and  $p_s \cdot t_1$  the total exchange time per second, or the fraction of the total time during which an exchange occurs, which is the definition of  $p$ .

The number of particles eroded per unit area and time is then:  $i_b \cdot p / A_1 \cdot D^3 \cdot t_1$

### 3.5.1 THE EXCHANGE TIME

The time  $t$  may be assumed to be proportional to the time necessary for it to settle in the fluid through a distance equal to its own size:

$$t_1 = A_3 \frac{D}{W} = A_3 \sqrt{\frac{D \cdot \rho}{g(\rho_s - \rho)}} \quad \dots\dots\dots (3.10)$$

and the number of particles eroded per unit of area and time is:

$$\frac{i_b \cdot \rho}{A_1 \cdot D^2 \cdot A_3} \sqrt{\frac{g(\rho_s - \rho)}{D \cdot \rho}}$$

Where:  $A_3$  is the constant time scale

$W$  is the settling velocity

$\rho_s$  is the sediment density

$\rho$  is the water density

The bed-load equation (3.11) shows that this rate of scour equals the corresponding rate of deposit:

$$\frac{i_b \cdot q_b}{\rho_s \cdot A_2 \cdot A_4 \cdot g \cdot D^4} = \frac{i_b \cdot \rho}{A_3 \cdot A_1 \cdot D^2} \sqrt{\frac{g(\rho_s - \rho)}{D \cdot \rho}} \quad \dots\dots\dots (3.11)$$

### 3.5.2 THE EXCHANGE PROBABILITY

The probability,  $p$ , of being eroded has been defined as the fraction of the total time during which at any one spot the local flow conditions cause a sufficiently large lift on the particles to remove it with all points of the bed

statistically equivalent,  $p$  may be also interpreted as the fraction of the bed on which at any time the lift on a particle of a given diameter  $D$  is sufficient to cause motion.

With this interpretation,  $p$  may be used to calculate the distance  $A_L \cdot D$  that a particle travels between consecutive places of rest. As long as  $p$  is small, deposition of the particle is practically everywhere possible and  $A_L$  equals a general constant,  $\lambda$ , which has about the value 100. If  $p$  is not small, however it must be recognized that deposition can not occur on that part ( $p$ ) of the bed where the lift force exceeds the particle weight. By averaging the distances travelled by the individual particles until they are able to settle out, the value  $A_L \cdot D$  can be expressed as:

$(1-p)$  particles are deposited after traveling  $\lambda \cdot D$

$p$  particles are not deposited after travelling  $\lambda \cdot D$   
of these,  $p(1-p)$  particles are deposited after travelling  $2 \cdot \lambda \cdot D$ .

$p^2$  particles are not deposited after travelling  $2 \lambda \cdot D$   
of these,  $p(1-p)$  particles are deposited after travelling  $3 \cdot \lambda \cdot D$  and so on.

The total (and average) distance traveled by the unit is obtained by additions

$$A_L \cdot D = \sum_{n=1}^{\infty} (1-p)^{n-1} (np) \lambda \cdot D = \lambda \cdot D / (1-p) \dots (3.12)$$

In this value is introduced in the above bed-load equation it may be rewritten:

$$\frac{q_p \cdot \lambda \cdot D}{A_L \cdot \lambda \cdot D \cdot D^3} = \frac{\lambda \cdot D}{A_L \cdot A_L \cdot D^3} \sqrt{\frac{g(R-P)}{D^3}} \dots (3.13)$$

or, separating p on one side of the equations

$$\frac{p}{1-p} = \frac{A_1 - A_2}{A_2 - \lambda} \frac{z_1}{z_2} \frac{(\frac{z_1}{z_2})^2}{(\frac{z_1}{z_2})^2} \frac{(\frac{z_1}{z_2})^3}{(\frac{z_1}{z_2})^3} \dots \quad (3.14)$$

$$\frac{p}{1-p} = [A_2] \frac{z_1}{z_2} \phi = A_2 \phi \quad (3.15)$$

Therefore: p is the probability of a particle being eroded from the bed and  $\phi$  is defined as:

$$\phi = \frac{z_1}{z_2} \frac{(\frac{z_1}{z_2})^2}{(\frac{z_1}{z_2})^2} \frac{(\frac{z_1}{z_2})^3}{(\frac{z_1}{z_2})^3} \dots \quad (3.16)$$

Thus  $\phi$  is a dimensionless measure of the bed-load transport; it may be called the intensity of bed-load transport. Being a dimensionless parameter it does not change with the scale and is, therefore invariant between model and prototype. This relation may also be expressed as follows: if  $\phi$  is equal in two different flows, the two rates of bed-load transport are dynamically similar.

### 3.5.3 DETERMINATION OF THE PROBABILITY p

As already noted, p is the probability of a particle being eroded from the bed, which means that the probability of the dynamic lift on the particle is larger than its weight (under water). The weight of the particle under water is:

$$W' = \rho (V - f) A_2 g \quad (3.17)$$

While the lift force may be expressed as:

$$L = C_L \cdot \rho \cdot u^2 \cdot A_1 \cdot D^2 / 2 \quad \dots\dots\dots (3.18)$$

In these expression:

$W'$  = weight of particle under water

$A_1$  = constant of grain area

$A_2$  = constant of grain volume

$L$  = lift force on bed particle

$\rho_s$  = density of particle

$\rho$  = density of water

$D$  = diameter of the particle

$u$  = velocity in direction of the main flow

$C_L$  = lift coefficient.

Einstein and El Samni found by measurement that the value of  $C_L = 0.178$ , and the velocity  $u$  near the bed which El Samni found must be measured at a distance  $0.35D$  from the theoretical bed for uniform sediment.

The forces acting on individual particles of a natural sediment mixture in a bed cannot be measured very well. They must be determined from their effects on the movement of particles. In analyzing the experiments the following general results were found:

1.- The velocity acting on all particles of a mixture must be measured at a distance  $0.35x$  from the theoretical bed, whereby:

$$x = 0.77 \Delta \quad \text{if } \Delta/\delta > 1.80 \quad \dots\dots\dots (3.19)$$

$$x = 1.39 \delta \quad \text{if } \Delta/\delta < 1.80$$

2.- The particles smaller than  $x$  ( $x > D$ ) seem to hide

between the other particles or in the laminar sublayer, respectively, and their lift must thus be corrected by division with a parameter  $\eta$  which itself is a function of  $D/X$  (Figure 3.5).

3. An additional correction factor  $Y$  was found to describe the change of the lift coefficient in mixtures with various roughness conditions. Figure 3.6, gives the correction factor  $Y$  in terms of  $Ks/\delta$ ,  $Y$  being unity for uniform sediment. Using these assumptions the velocity in the expression for the average lift  $L$  may be written as:

$$u = u_* 5.75 \text{Log} \left( \frac{30.2 + 0.35 X}{\Delta} \right)$$

$$u^2 = K_s^2 \cdot 0.8 \cdot 5.75^2 \text{Log}^2 (10.6 X/\delta)$$

Then at any instance the lift force may be described by:

$$L = 0.178 \rho \cdot A_s \cdot u^2 \cdot (1/2) K_s^2 \cdot 0.8 \cdot 5.75^2 \text{Log}^2 (10.6 X/\delta) (1+\eta) \dots (3.20)$$

Where;  $\eta$  is a parameter varying with time.

Now  $p$  may be expressed as the probability of  $W^2/L$  to be smaller than unity:

$$1 > \frac{W^2}{L} = \frac{1}{1+\eta} \left[ \frac{\rho \cdot A_s \cdot p}{\rho_s \cdot 0.8 \cdot 5.75^2 \text{Log}^2 (10.6 X/\delta)} \right] \dots (3.21)$$

The value of  $\eta$  in this inequality may be either positive or negative. In both cases the lift is actually positive and must, therefore be understood on an absolute basis.

The inequality may be written in absolute values:

$$1 + \eta > 0.178 \cdot \frac{\rho \cdot A_s \cdot p}{\rho_s \cdot 0.8 \cdot 5.75^2 \text{Log}^2 (10.6 X/\delta)} \dots (3.22)$$



Introducing the abbreviations:

$$\left. \begin{aligned} \psi &= \frac{\rho_s - \rho}{\rho} \frac{D}{R_B^2 \cdot S} \\ B &= \frac{2 \cdot A_2}{0.178 A_1 \cdot 5.75^2} \\ \beta_x &= \text{Log}(10.6 X/\Delta) \end{aligned} \right\} \dots\dots\dots (3.23)$$

Introducing the two correction factors  $\xi$  and Y according to the previously quoted assumptions, the inequality (3.22) may be generalized.

$$[1 + \eta] > \xi \cdot Y \cdot B' \cdot \psi \cdot \beta^2 / \beta_x^2 \dots\dots\dots (3.24)$$

Where:  $\xi$  is a function of D/X (Figure 3.5)

Y is a function of  $Ks/\delta$  (Figure 3.6)

$$B' = B/\beta^2$$

$$\beta = \text{Log}(10.6)$$

and:  $\beta^2 / \beta_x^2 = 1$  for uniform grain and X=1

Y = 1 for uniform grain and X=1

$\xi = 1$  for uniform grain and X=1

Inequality (3.22) may be written more conveniently by squaring and division by  $\eta_0$ , the standard deviation of  $\eta$ .

Introducing  $\eta = \eta_0 \cdot \eta_*$

$$[1/\eta_0 + \eta_*]^2 > \xi^2 \cdot Y^2 \cdot B_*^2 \cdot \psi (\beta^2 / \beta_x^2)^2 = B_*^2 \cdot \psi_*^2 \dots\dots (3.25)$$

If  $B_* = B' / \eta_0$  ..... (3.26)

$$\text{and } \Psi_x = \int_0^x \gamma (\theta^2 / \theta_x^2) \Psi$$

Using these symbols, the limiting cases of motion may be written as:

$$[1/\eta_0 + \eta_x] = [B_x \cdot \Psi_x]^2$$

$$[\eta_x] \text{ LIMIT} = \pm B_x \cdot \Psi_x = 1/\eta_0 \quad \dots\dots\dots (3.27)$$

As the probability for  $\eta_x$  values is distributed according to the normal error law, the probability  $p$  for motion is:

$$p = 1 - \frac{1}{\sqrt{\pi}} \int_{B_x \cdot \Psi - 1/\eta_0}^{B_x \cdot \Psi_x - 1/\eta_0} e^{-t^2} dt \quad \dots\dots\dots (3.28)$$

Where  $t$  is only a variable of integration.

By combination with equation (3.15) the final bed-load equation is obtained:

$$p = \frac{A_x \cdot \phi(i_B/i_b)}{1 - A_x \cdot \phi(i_B/i_b)} = \frac{A_x \cdot \phi_x}{1 - A_x \cdot \phi_x} \quad \dots\dots\dots (3.29)$$

This equation is easy to use although it appears to be very complicated.  $\eta_0$ ,  $A_x$  and  $B_x$  are universal constants such that the equation may be represented by a single curve between the flow intensity  $\Psi_x$  and the intensity of bed-load transport  $\phi_x$ . This relationship may be calculated from tables of the probability integral for the value  $1/\eta_0 = 2.0$  as determined by El Sami. The constants  $A_x$  and  $B_x$  were determined from bed-load experiments with uniform grain for which  $\phi_x = \phi$  and  $\Psi_x = \Psi$ .

Figure J.7 shows a plot of some experimental points with the curve using  $A_s = 27.0$  and  $B_s = 0.156$ . The formula can unquestionably be applied to coarse sediment as it is almost identical with most other bed-load formulas for low intensities. For the higher intensities which occur only with small particle sizes, some applications to actual rivers have given encouraging results, while more applications under a wider range of conditions are still necessary to prove its universal applicability.

Although the correction  $Y$  as a function of  $Ks/g$  does not require any explanation. The correction factor  $Y$  which gives the effect on the transport when the small particles of the bed hide either behind and between larger particles or in the laminar sublayer, needs some comment.

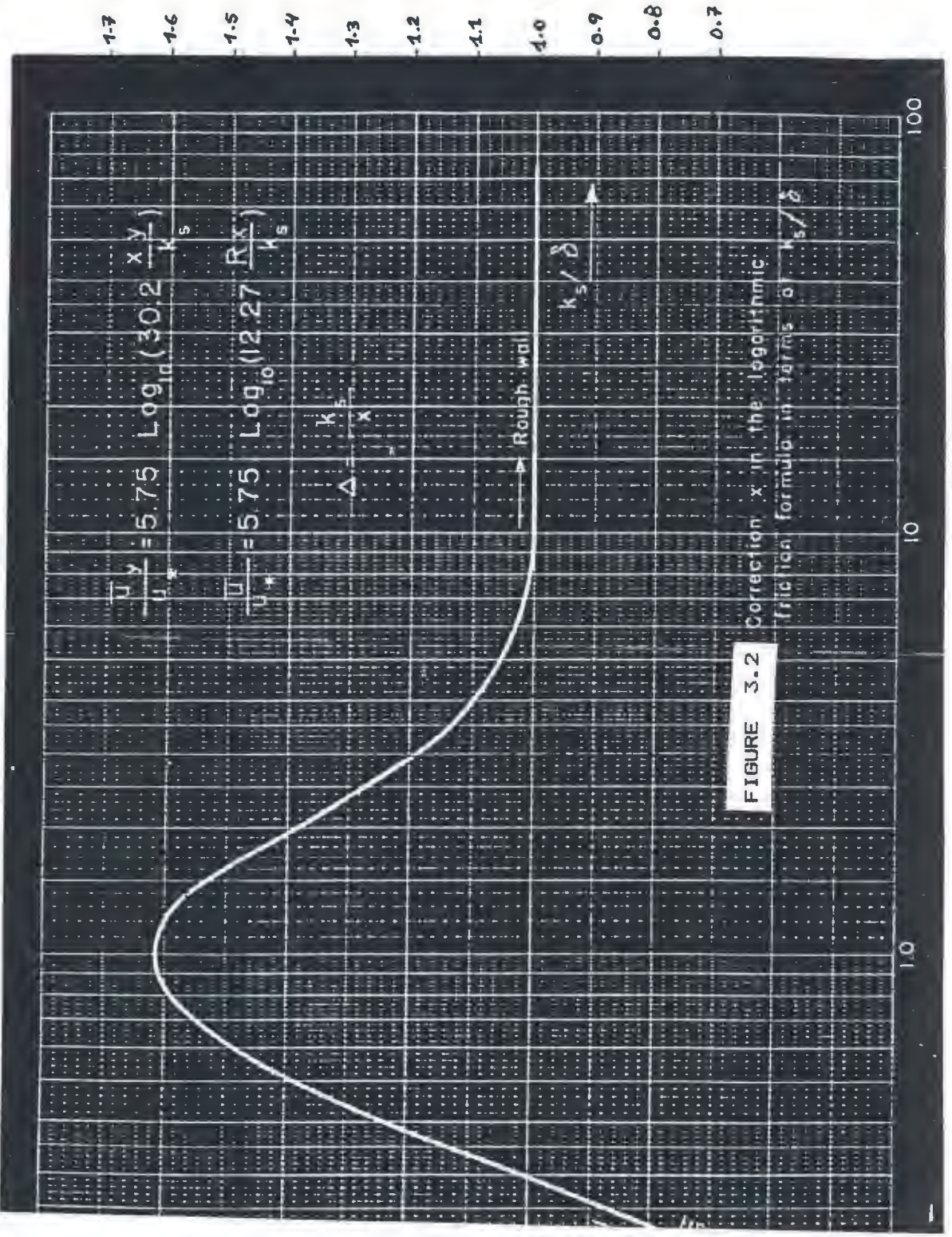


FIGURE 3.2 Correction  $x$  in the logarithmic friction formula in terms of  $k_s/\delta$ .

1.0 10 100

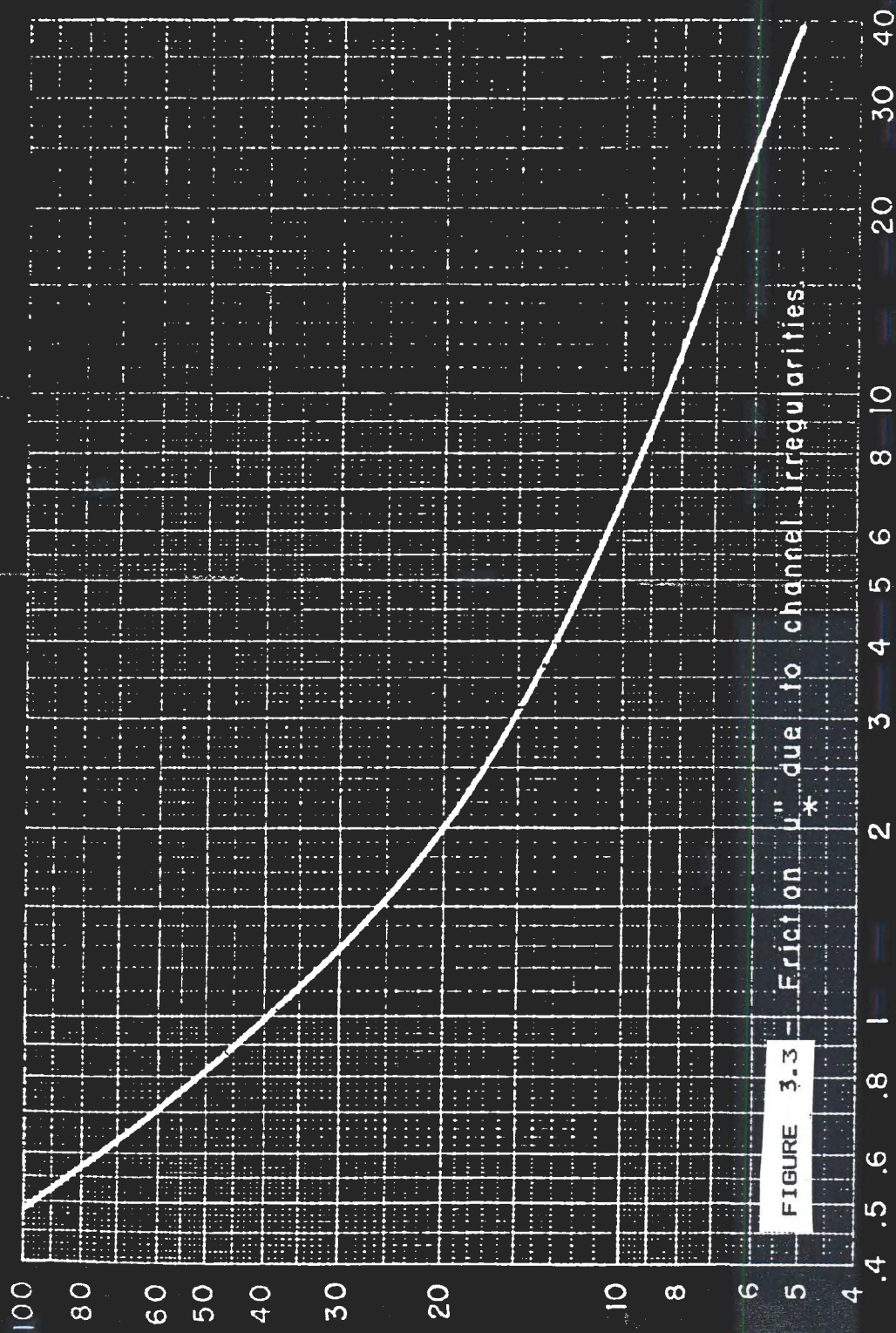


FIGURE 3.3 - Friction  $u^*$  due to channel irregularities

$$\psi' = 1.68 \frac{D^{3.5}}{R^1 S_e}$$

$\frac{u^*}{U}$

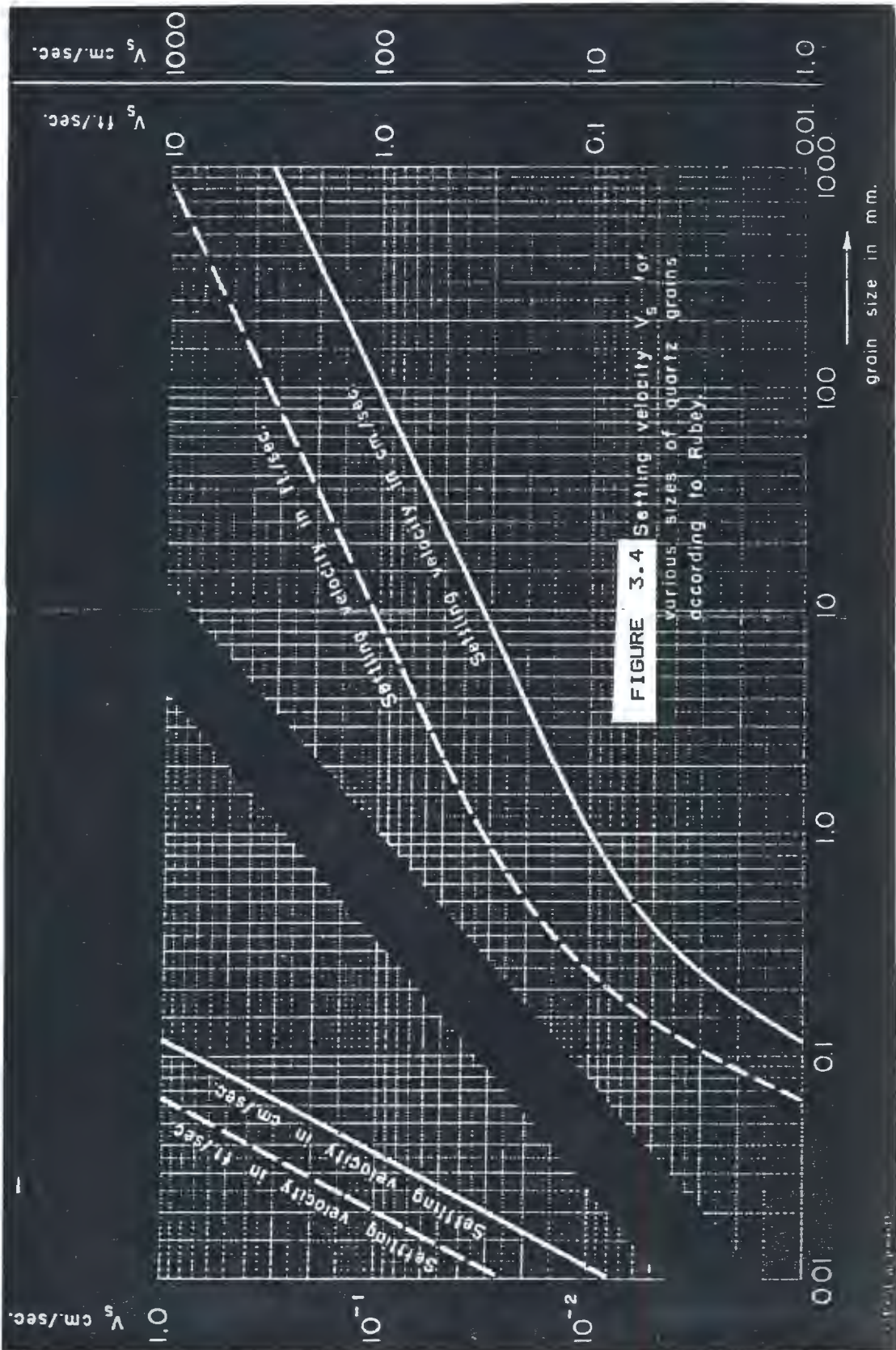
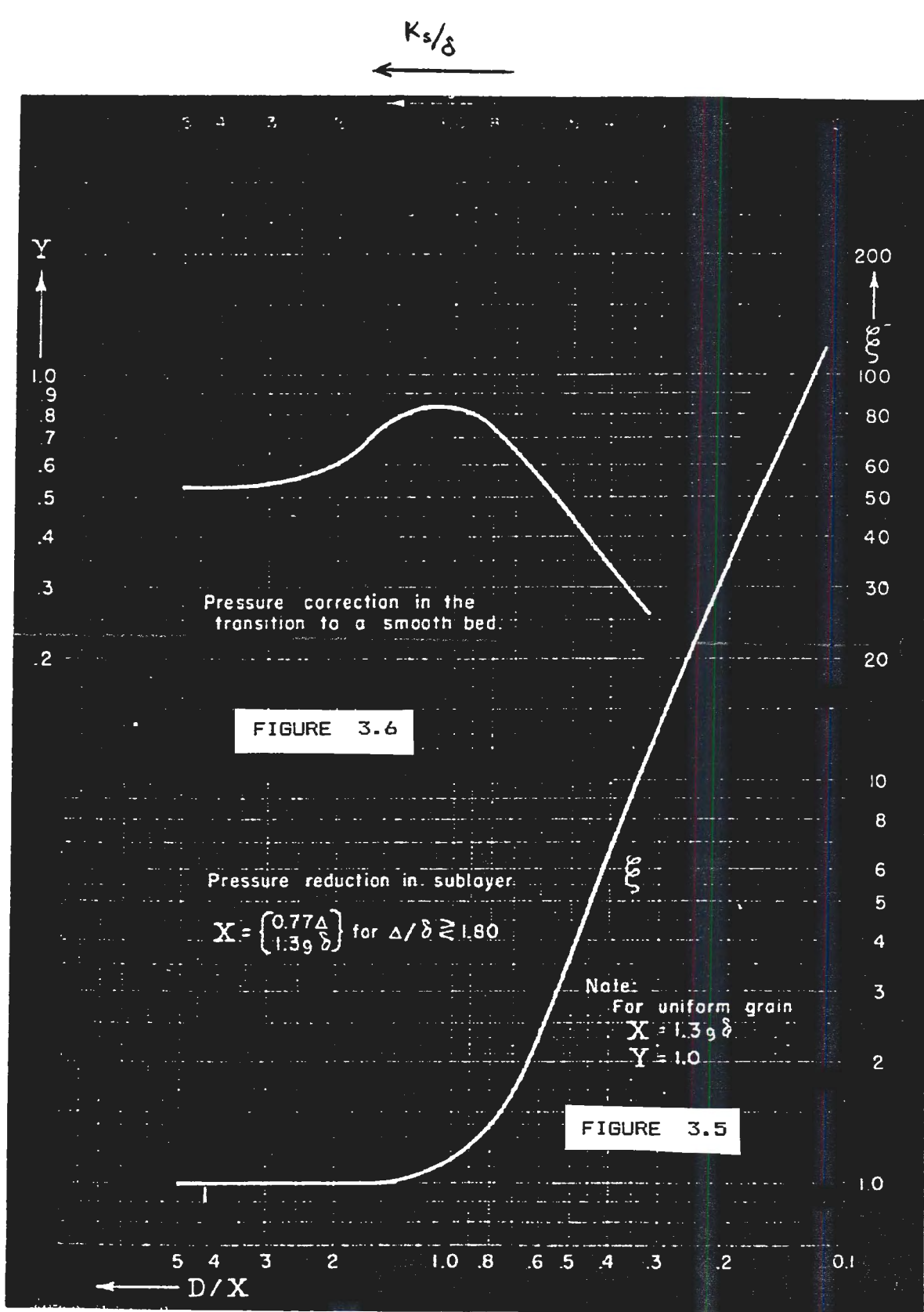
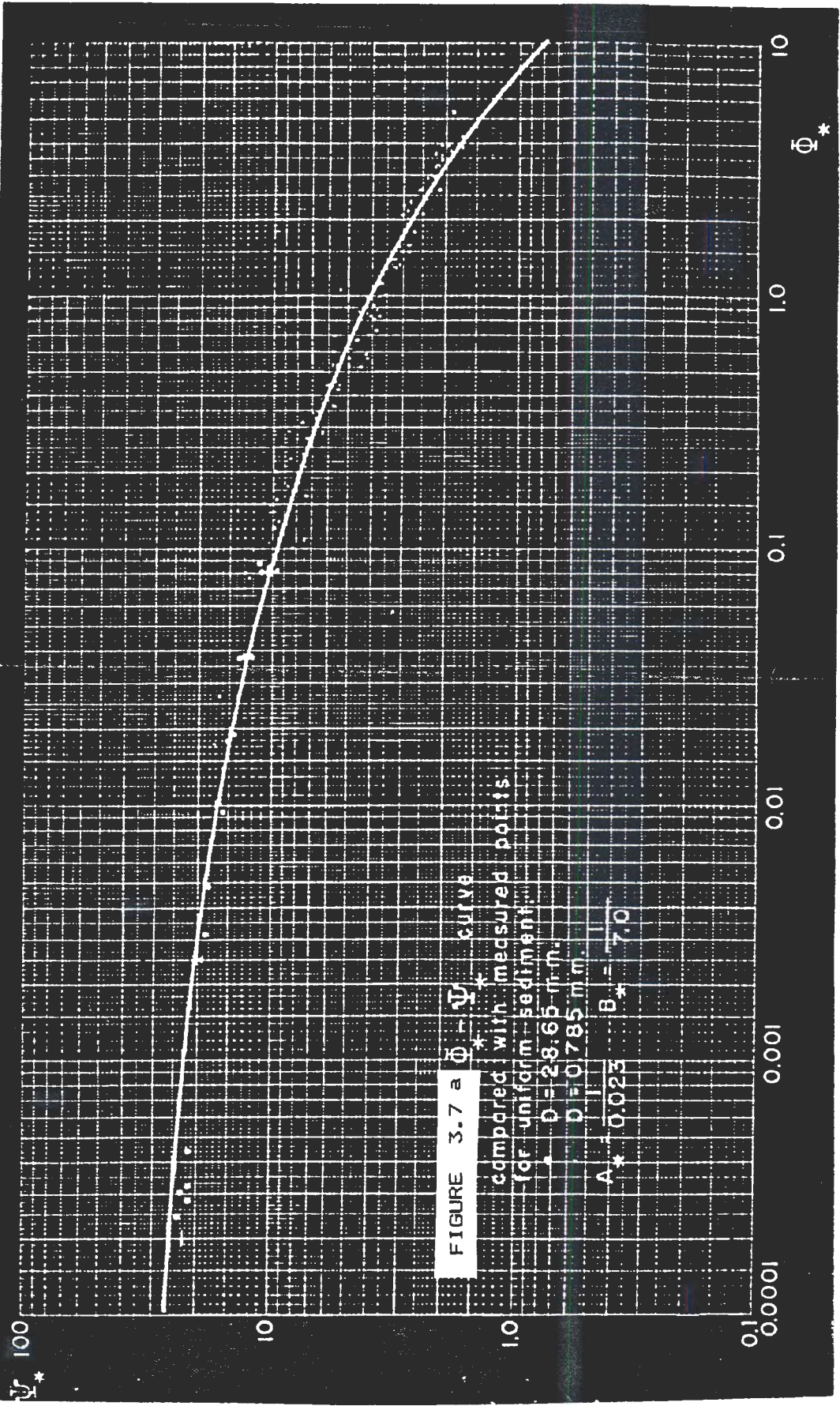


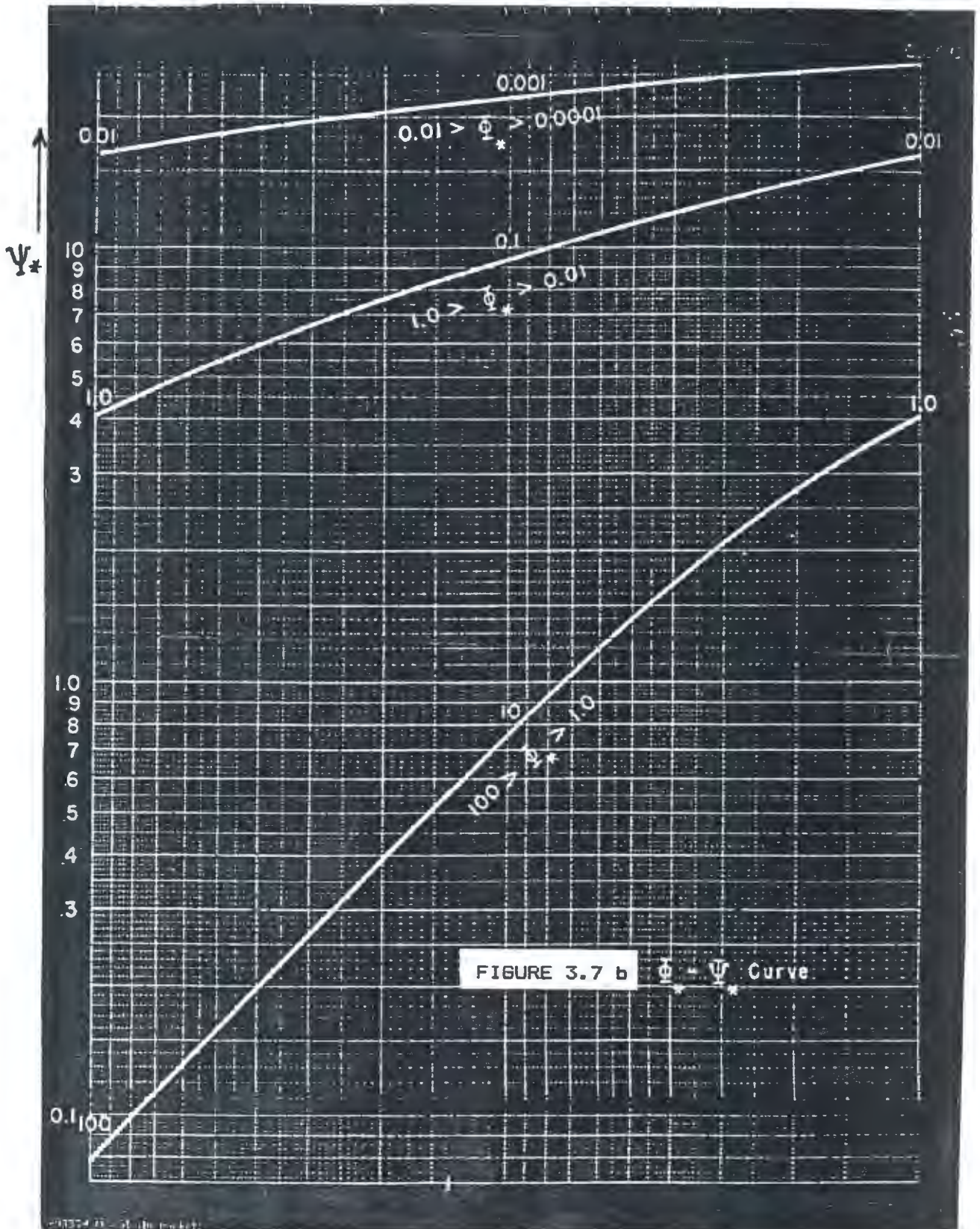
FIGURE 3.4 Settling velocity,  $V_s$ , for various sizes of quartz grains according to Rubey

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### 3.6 SUSPENSION

The fine particles of the sediment load of streams move predominantly as suspended load.

Suspension as a mode of transport is opposite to what Bagnold called "Surface Creep" and to what he defined as the heavy concentration of motion is immediately at the bed.

The characteristic definition of a suspended solid particle is that its weight is supported by the surrounding fluid during its entire motion. While being moved by the fluid, the soil particle, which is heavier than the fluid, tends to settle in the surrounding fluid. If the fluid flow has only horizontal velocities, it is impossible to explain how any sediment particle can be permanently suspended. Only if the irregular motion of the fluid particles, called turbulence, is introduced can one show that sediment may be permanently suspended.

Assume an upward flow of velocity ( $V$ ) in half the area in a channel cross section and a downward flow of the same velocity ( $-V$ ) in the other half. The exchange discharge through the unit area is  $q_v = V/2$ . If the exchange takes place over an average distance of  $l_0$  at elevation  $y$  it can be assumed that the downward-moving fluid originates, as an average, from an elevation  $(y + l_0/2)$  while the upward-moving fluid originates from  $(y - l_0/2)$ . The important assumption is made that the fluid preserves during its exchange the qualities of the fluid at the point of origin. Only after completion of the exchange travel over the distance  $l_0$  will it mix with the surrounding fluid. From

this it is possible to calculate the transport of a given size of suspended particles with a known settling velocity  $W$ , if the concentration of these particles at  $y$  is  $C_y$ , the upward motion of particles per unit area and per unit time is:

$$C_{(y-\frac{1}{2}\Delta y)} \frac{1}{2} (V+W)$$

and the rate of downward is:

$$C_{(y+\frac{1}{2}\Delta y)} \frac{1}{2} (V-W)$$

the net upward motion is therefore:

$$\frac{1}{2} C_{(y-\frac{1}{2}\Delta y)} (V+W) - \frac{1}{2} C_{(y+\frac{1}{2}\Delta y)} (V-W) \dots (3.29)$$

Neglecting all higher terms, the concentration may be expressed as:

$$C_{(y-\frac{1}{2}\Delta y)} = C_y - \frac{1}{2} \Delta y \frac{dC_y}{dy} \dots (3.31)$$

$$C_{(y+\frac{1}{2}\Delta y)} = C_y + \frac{1}{2} \Delta y \frac{dC_y}{dy}$$

Introducing (3.31) in equation (3.29), the net upward motion is:

$$\frac{1}{2} (C_y - \frac{1}{2} \Delta y \frac{dC_y}{dy}) (V+W) - (C_y + \frac{1}{2} \Delta y \frac{dC_y}{dy}) (V-W) = C_y \cdot W - \frac{1}{2} V \cdot \Delta y \frac{dC_y}{dy} \dots (3.32)$$

Most interesting is the equilibrium status at which there exists no net flow in either direction:

$$C_y \cdot W + \frac{1}{2} V \cdot l_c \frac{dC_y}{dy} = 0 \quad \dots (3.33)$$

In equation (3.33) both  $V$  and  $l_c$  are unknown. It is customary to assume that these two values are equal to the corresponding values in a similar equation for the exchange of momentum transport, the depth  $d$  may be introduced.

$$\begin{aligned} \tau_y &= \tau_0 \frac{d-y}{d} = \frac{1}{2} V \cdot \rho [u_{(y-\frac{1}{2}l_c)} - u_{(y+\frac{1}{2}l_c)}] \\ &= \frac{1}{2} V \cdot \rho \left[ u_y - \frac{1}{2} l_c \frac{du}{dy} - \left( u_y + \frac{1}{2} l_c \frac{du}{dy} \right) \right] \\ &= - \frac{1}{2} V \cdot \rho \cdot l_c \frac{du}{dy} \end{aligned}$$

From this we calculate:

$$\frac{1}{2} V \cdot l_c = - \frac{\tau_0}{\rho} \frac{d-y}{d} \frac{1}{du/dy} = - u_*^2 \frac{d-y}{d} \frac{1}{du/dy} \quad \dots (3.34)$$

Using equation (3.3) for the velocity distribution we may calculate  $du/dy$

$$\frac{du_y}{dy} = \frac{5.75}{2.303} \frac{u_*}{y} \quad \dots (3.35)$$

Introducing this value into equation (3.34):

$$\frac{1}{2} V \cdot l_c = -0.40 y \cdot u_* \frac{d-y}{d} \quad \dots (3.36)$$

This value must be used in equation (3.33)

$$C_y \cdot W = 0.40 \cdot y \cdot u_s \frac{d-y}{d} \frac{dC_y}{dy} \quad \dots (3.37)$$

Separating the variables:

$$\frac{dC_y}{C} = \frac{W}{0.40u_s} \frac{d}{y} \frac{dy}{(d-y)} \quad \dots (3.38)$$

And introducing the abbreviation:

$$z = \frac{W}{0.40u_s} \quad \dots (3.39)$$

We can integrate this equation from  $a$  to  $y$

$$\begin{aligned} \int_a^y \frac{dC_y}{C} &= \int_a^y d(\text{Ln} C_y) = \text{Ln}(C_y) - \text{Ln}(C_a) = \text{Ln} \left( \frac{C_y}{C_a} \right) \quad \dots (3.40) \\ &= \int_a^y \frac{z \cdot d \cdot dy}{y(d-y)} = \int_a^y d \left( \text{Ln} \frac{d-y}{y} \frac{a}{d-a} \right) = \text{Ln} \left( \frac{d-y}{y} \frac{a}{d-a} \right) \end{aligned}$$

This may be written in the form:

$$\frac{C_y}{C_a} = \left( \frac{d-y}{y} \frac{a}{d-a} \right)^z \quad \dots (3.41)$$

and be used to calculate the concentration of a given grain size with the settling velocity  $W$  at the distance  $y$  from the bed, if the concentration  $C_a$  of the same particle at distance  $a$  is known.

### 3.7 INTEGRATION OF THE SUSPENDED LOAD

The integral of suspended load moving through the unit width of a cross section may be obtained by combining equations (3.42) and (3.3).

$$\int_y^d C_y \cdot \bar{u}_y \cdot dy = \int_y^d C_a \left( \frac{d-y}{y} \right)^{\frac{z}{d-a}} 5.75 u_* \text{Ln} \left( 30.2 \frac{y}{\Delta} \right) dy \quad \dots (3.42)$$

Taking some of the constant factors out the integral, and referring the concentration to that at the lower limit of integration  $a$ , and replacing  $a$  by its dimensionless value  $A_E = a/d$ , and using  $d$  as the unit for  $y$ , we obtain:

$$\begin{aligned} q_s &= \int_a^d C_y \cdot \bar{u}_y \cdot dy = \int_{A_E}^1 d \cdot C_y \cdot \bar{u}_y \cdot dy \\ &= d \cdot u_* \cdot C_a \left( \frac{A_E}{1-A_E} \right)^{\frac{z}{d-a}} 5.75 \int_{A_E}^1 \left( \frac{1-y}{y} \right)^{\frac{z}{d-a}} \text{Log} \left( \frac{30.2y}{\Delta/d} \right) dy \\ &= 5.75 C_a \cdot d \cdot u_* \left( \frac{A_E}{1-A_E} \right)^{\frac{z}{d-a}} \left( \text{Log} \left( \frac{30.2d}{\Delta} \right) \int_{A_E}^1 \left( \frac{1-y}{y} \right)^{\frac{z}{d-a}} dy + \right. \\ &\quad \left. \int_{A_E}^1 \left( \frac{1-y}{y} \right)^{\frac{z}{d-a}} \text{Log} y dy \right) \quad \dots (3.43) \end{aligned}$$

In order to reduce the two integrals in equation (3.43) into a basic form, the  $\text{Log}(y)$  is changed to  $\text{Ln}(y)$ .

$$\text{Log}(y) = \text{Ln}(y) \cdot \text{Log}(e) \quad \dots (3.44)$$

Where:  $\text{Log}(e) = 0.43429$

Now we may write equation (3.44) in the form:

$$q_s = 5.75 u_* \cdot d \cdot C_{s0} \left[ \frac{R_0}{1-R_0} \right]^2 \log \left[ \frac{30.2d}{\Delta} \right] + \int_{R_0}^1 \left( \frac{1-y}{y} \right) dy + 0.434 \int_{R_0}^1 \left( \frac{1-y}{y} \right)^2 \ln \left( \frac{1-y}{y} \right) dy \quad (2.45)$$

With

$$u_* = W / (0.40 u_{*c})$$

$y$  measured with  $d$  as unit

$$R_0 = a/d$$

$$u_{*c} = \sqrt{3.0 \tau_{*c}} = \sqrt{8 R_0 g}$$

Herein are:

$q_s$  The sediment load in suspension (per unit of width) measured in weight, moving per unit of time between the water surface and the reference level  $y = a$  ( $C_{s0}$  is measured in weight per unit volume of mixture).

$R_0$  The dimensionless distance of this lower limit of integration from the bed,  $R_0 = a/d$

$u$  Defined as the settling velocity  $W$  of the particles divided by the Karman constant 0.40 and the shear velocity  $u_*$ .

$y$  The variable of integration, the dimensionless distance of any point in the vertical from the bed, measured in water depths  $d$ .

It is convenient to transform equation (2.45) in order to make the numerical integration of suspended load:

$$q_s = 11.6 u_* \cdot C_{s0} \cdot a + 2.503 \log \left[ \frac{30.2d}{\Delta} \right] \left( I_1 + I_2 \right) \quad (2.46)$$

With:

$$I_1 = 0.216 \frac{A_s^{2.1}}{(1-A_s)^2} \int_0^1 \frac{1-y}{y} I^2 dy$$

... (3.47)

$$I_2 = 0.216 \frac{A_s^{2.1}}{(1-A_s)^2} \int_0^1 \frac{1-y}{y} I \ln(y) dy$$

Herein  $(11.6uy)$  is the flow velocity at the outer edge of the laminar sublayer in case of a hydraulically smooth bed, or the velocity in a distance of 3.28 roughness diameter from the wall in case of a rough wall. The symbol  $C_a$  stands for the sediment concentration at a distance 'y' from the bed. The integral values  $I_1$  and  $I_2$  are plotted in figures 3.8 and 3.9, respectively.

### 3.8 TRANSITION BETWEEN BED LOAD AND SUSPENDED LOAD

The total rate of transport,  $q_b i_b$  of a given grain size in the bed and the thickness of the layer of 2D within this transport occurs have been determined and assumed respectively. From these values the average concentration in the bed layer may be found. The concentration is defined as the weight of solids per unit volume of water-sediment mixture. First, the weight of bed-load material in motion may be calculated for the unit of bed area. Let  $u_b$  be the average velocity with which bed-load material moves in the bed layer while in motion, not including the rest periods. Then the weight of particles of a given size per unit area is

$$\frac{I_2 \cdot q_b}{u_b}$$

The volume of the unit area of bed layer is 2D and the



average concentration in the layer is:

$$\frac{i_b \cdot Q_b}{u_b \cdot 2D}$$

We may assume that the concentration in the entire bed layer is constant, since the layer is only two diameter thick. The following equation may be set up:

$$C_a = \frac{A_b \cdot i_b \cdot Q_b}{2D \cdot u_b} \quad \dots (3.48)$$

The velocity  $u_b$  is not known. Both the flow velocity and the transport near the bed are functions of  $u_b^*$ . The two must determine  $u_b$ . This makes it very probable that  $u_b$  is proportional to  $u_b^*$  because it has the dimension of a velocity, too. Therefore,  $C_a$  may be expressed in the form:

$$C_a = A_b \frac{i_b \cdot Q_b}{2D \cdot u_b^*} \quad \dots (3.49)$$

The value  $A_b$  must be determined experimentally. It includes both the distribution of concentrations in the bed layer and the velocity of the bed layer. It can therefore, best be determined from flume experiments which compare the suspended load with the hydraulics of the flow. "Flume tests with Sediment Mixtures" suggests that  $A_b = 1/11.4$ , and the equation (3.49) may thus be written as:

$$i_b \cdot Q_b = 11.4 C_a \cdot u_b^* \cdot a \quad \dots (3.50)$$

and the total suspended load per unit width  $i_b \cdot Q_b$  may be calculated from equations (3.46) and (3.50):

$$\begin{aligned}
 i_s \cdot q_s &= i_b \cdot q_b \left( 2.303 \log \left( \frac{30.2 \pi}{Ks/d} \right) + i_1 + i_2 \right) \quad \dots (3.51) \\
 &= i_b \cdot q_b (PE \cdot i_1 + i_2)
 \end{aligned}$$

$$\text{Whereby } PE = 2.303 \log \left( \frac{30.2 \pi}{Ks/d} \right) = 2.303 \log \left( \frac{30.2 \pi}{\Delta} \right) \quad \dots (3.52)$$

This relationship relates transportation as bed load to that in suspension of all particle sizes for which a bed load function exists. PE is called transport parameter.

### 3.9 THE TOTAL SEDIMENT TRANSPORT

The total load rate  $(i_s \cdot q_s)$  is obtained by addition of the bed load rate  $(i_b \cdot q_b)$  and the suspended load rate  $(i_s \cdot q_s)$ .

$$i_s \cdot q_s = i_b \cdot q_b + i_s \cdot q_s = i_b \cdot q_b (PE \cdot i_1 + i_2 + 1) \quad \dots (3.53)$$

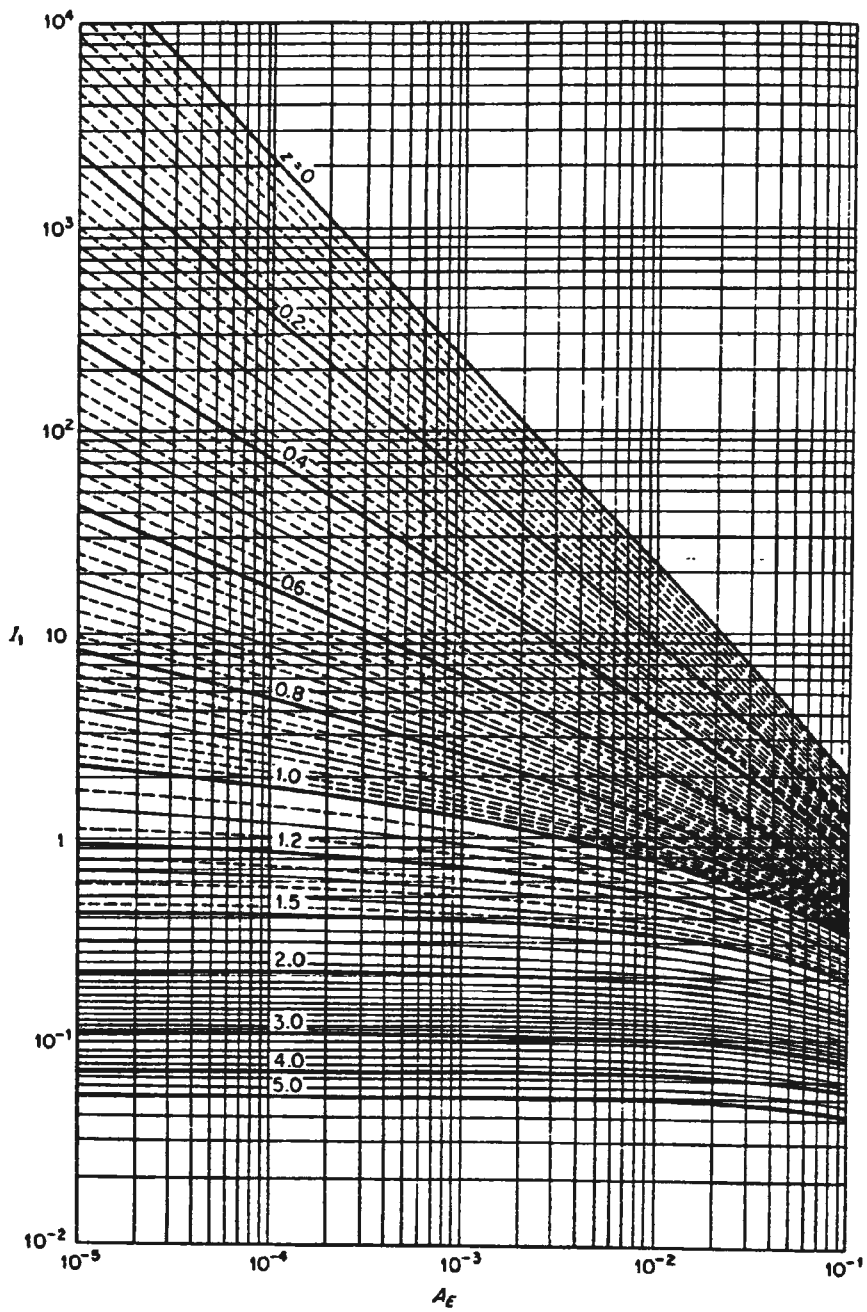


FIGURE 3.8 Function  $I_1$  in terms of  $A_E$  for values of  $z$ . [After EINSTEIN (1950).]

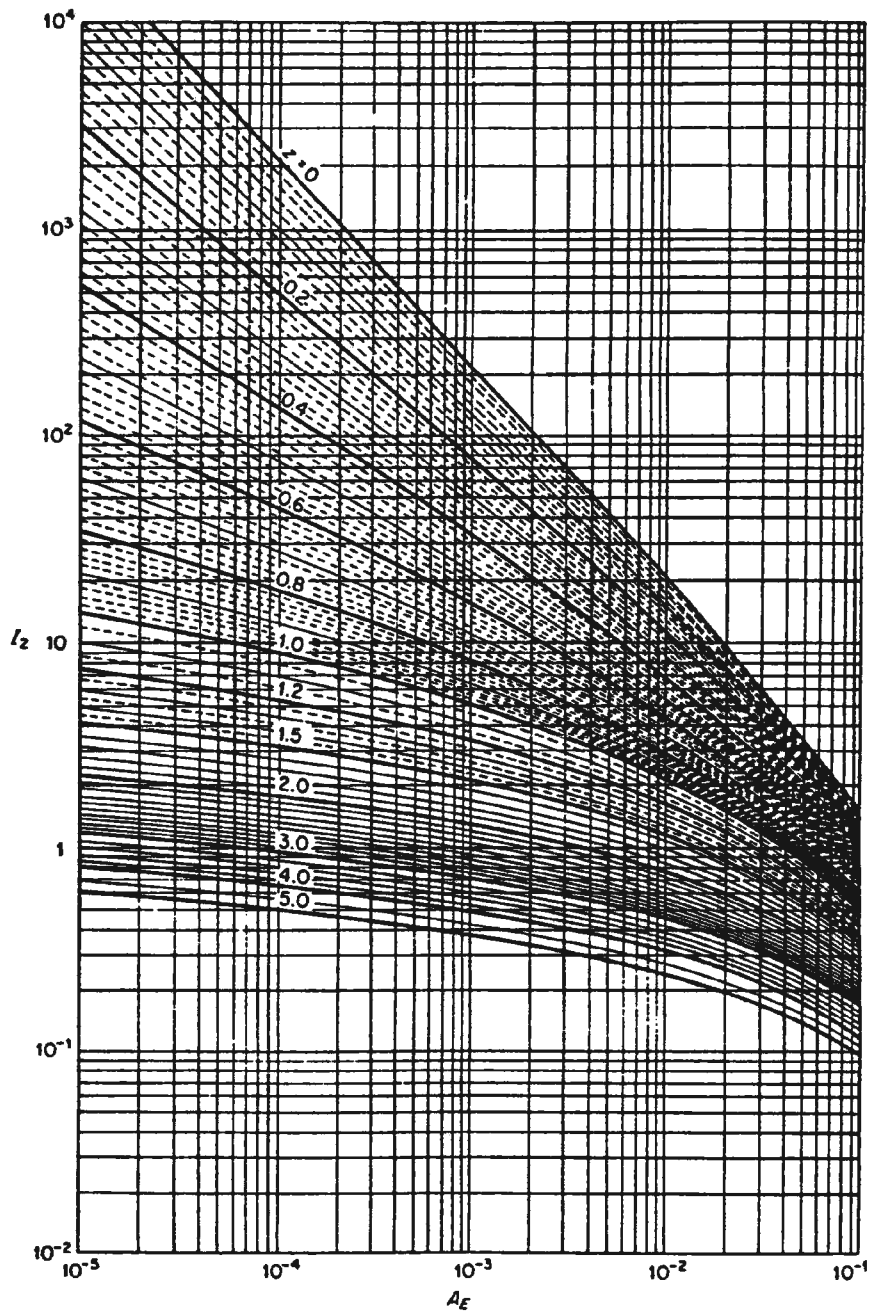


FIGURE 3.9 Function  $I_2$  in terms of  $A_E$  for values of  $z$ . [After EINSTEIN (1950).]

#### 4. GENERAL DESCRIPTION OF THE SITES AND DATA

##### 4.1 GENERAL DESCRIPTION OF THE SITES

The Zaire river is located in the central part of Africa. It is the second largest river in the world in terms of the annual fresh water inflow to the oceans.

The head of the estuary is called the Matadi and the braided area, 60 Km. long, begins at the harbour of Boma, 60 Km. downstream from the harbour of Matadi. The study area is situated about 15 Km. downstream of Boma.


The measurements had been made at Mateba by three campaigns, one during the period August-September 1978, the second during October 1978 and the last in 1981.

These measurements were carried out for the following cross sections :

AMONT OISEAUX	1978
OISEAUX NORD	1978
BARRAJE	1978
MATEBA AMONT SUD	1978
MATEBA SUD MANDUD	1979
MATEBA SUD KAPITA	1979
MATEBA AMONT	1981
MATEBA CENTRAL	1981
MATEBA AVAL	1981

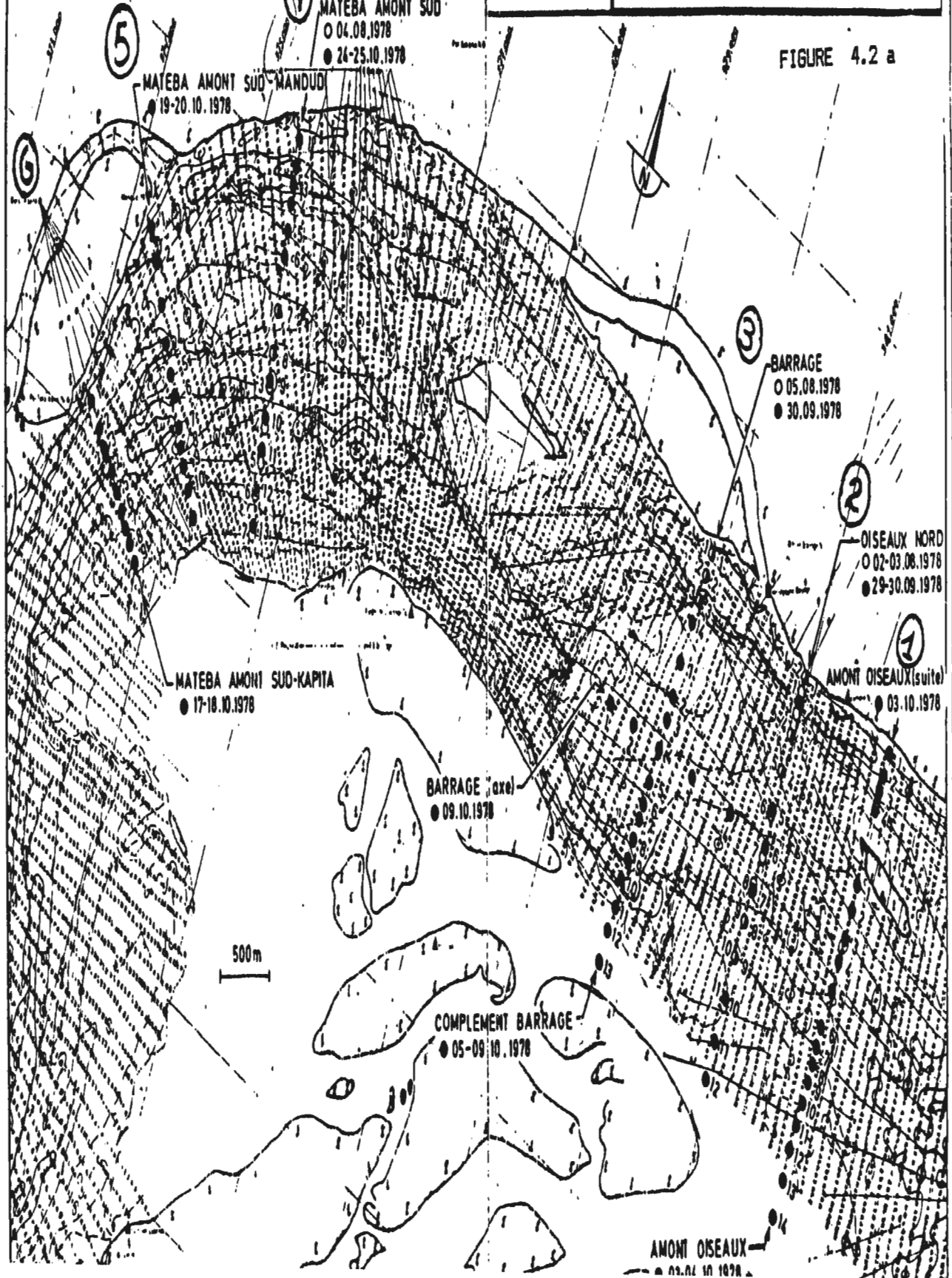
The location of these cross sections are shown on Figures 4.2.



 LOCALISATION DE STATIONS DE MESURES	MOD. 255-16 BIEF MARITIME DU FLEUVE ZAIRE
	LOCALISATION DES STATIONS DE MESURES MATEBA AMONT - AOUT-OCTOBRE 1978

RM: Plan R.XM. 249. 211.

FIGURE 4.2 a



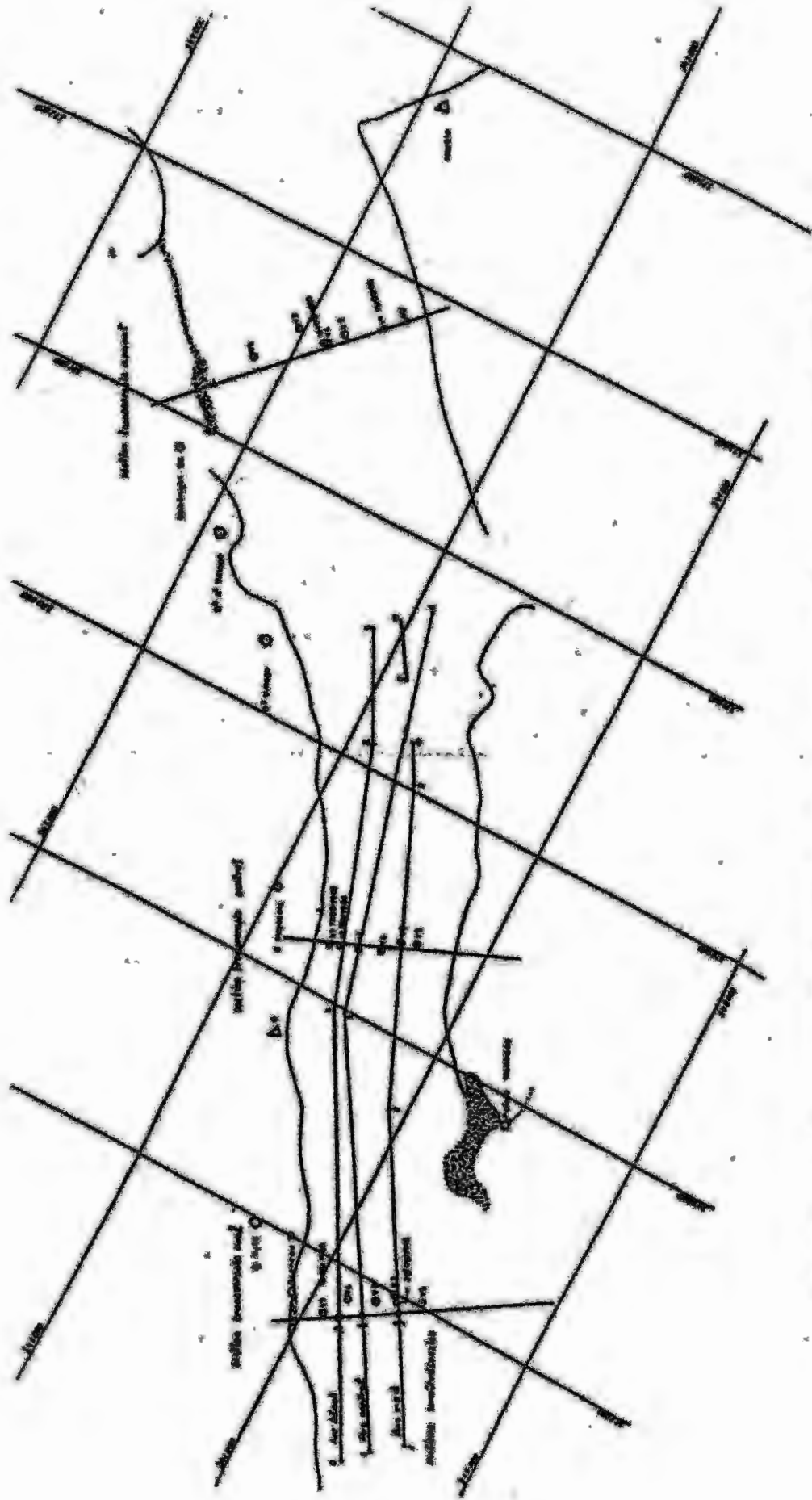


FIGURE 4.2.6 MAP OF MATÉBA WITH CROSS SECTIONS: ARIENT, CENTRAL AND AVIAL.



## 4.2 GENERAL DESCRIPTION OF DATA

The data were obtained from measurements consisting of values of  $Z$  and  $V$  whereby  $Z$  is the distance in meters from the water surface to the point where the velocity ( $V$ ) is measured in a considered vertical. The other data correspond to the coordinates of the verticals for each cross section.

The data mentioned above were provided from data files VELOCIT and COORD, created by Amarakoon for six cross sections and from data file AVAL1 and COORDXY, created by Dyer for the last three cross sections described in 4.1.

These data files have been used to feed the computer program CHANGE which transforms the information for easy handling and use it to generate the requirement data through the program RATING.

For hydraulic considerations some cross sections were divided into two parts (1), and (2) and each of these was considered as independent.

- #1. - DATA FILE : VELOCIT and AVAL1
- column 1 : vertical number
  - column 2 : velocity of the water surface for each vertical (m/sec).
  - column 3 : total water depth for each vertical (m).
  - column 4 : number of velocity measurement in each vertical.
  - column 5 to 17 : measurements of  $Z$  and  $V$  wherein,  $Z$  is the distance in meters from the water surface

to the point where the velocity  
(V) is measured.

b).- DATA FILE : COORD and CORDXY

- column 1 : vertical number
- column 2 : coordinate X of the vertical.
- column 3 : coordinate Y of the vertical.

c).- DATA FILE : ZADATA1 (\*) and ZADATA2 (\*\*)

- column 1 : vertical number
- column 2 : total water depth (m), for each vertical (the right and left banks are considered as vertical too).
- column 4 to 17 : values of Z and V wherein, Z is the distance in meters from the water surface to the point where the velocity (V) is measured.

(\*) data file which contains information for the cross sections : Mateba Awant, Mateba Central, and Mateba Aval.

(\*\*) contains the information for all cross sections considered in the present study.

d).- DATA FILE : RATINGS1

Generated by the computer program RATINGS

- column 1 : vertical number
- column 2 : total water depth for each vertical.
- column 3 : distance in meters from right bank to each vertical.

- column 4 to 17 : values of  $Z_1$  and  $Q$  wherein,  $Z_1$  is the distance from the bottom of each vertical to half of two consecutive points of velocities and  $Q$  is the discharge ( $m^3/sec$ ) to the corresponding  $Z_1$ .

3).- DATA FILE : RATINGS

obtained from computer program RATINGS

- column 1 : vertical number
- column 2 : height calculated from the reference to the bottom of each vertical. The maximum depth of the cross section is considered as the reference.
- column 3 : distance from right bank to each vertical.
- column 4 to 17 : values of  $H_1$  and  $Q$  wherein,  $H_1$  is the distance from datum to half of the two consecutive points of velocities and  $Q$  is the discharge ( $m^3/sec$ ) to the corresponding  $H_1$ .

4).- DATA FILE : RATINGS

obtained from computer program RATINGS

- column 1 :  $H$  water elevation from the bottom (ft).
- column 2 : total discharge corresponding to  $H$  ( $m^3/sec$ ).
- column 3 : hydraulic area corresponding to  $H$  in  $1$ .

- column 4 : wetted perimeter (m).
- column 5 : hydraulic radius (m).

g).- DATA FILE : RATING4

Obtained from program RATING

- column 1 : number of vertical.
- column 2 : water depth (m), for each vertical.
- column 3 : discharge for each vertical ( $m^3/sec$ ).
- column 4 : hydraulic area ( $m^2$ ) for each vertical.
- column 5 : mean velocity (m/sec) for each vertical
- column 6 : shear velocity (m/sec) for each vertical.
- column 7 : wetted perimeter (m) for each vertical.
- column 8 : hydraulic radius (m) for each vertical.

In the data file ZADATA2, RATING1, RATING2 and RATING4 we can identify :

ROW 1 to 6 : CROSS SECTION : AMONT OISEAUX 1  
 ROW 7 to 22 : CROSS SECTION : AMONT OISEAUX 2  
 ROW 23 to 28 : CROSS SECTION : OISEAUX NORD 1  
 ROW 29 to 40 : CROSS SECTION : OISEAUX NORD 2  
 ROW 41 to 47 : CROSS SECTION : BARRAJE 1  
 ROW 48 to 63 : CROSS SECTION : BARRAJE 2  
 ROW 64 to 72 : CROSS SECTION : MATEBA AMONT SUD 1  
 ROW 73 to 82 : CROSS SECTION : MATEBA AMONT SUD 2  
 ROW 83 to 95 : CROSS SECTION : MATEBA SUD MANDUDI  
 ROW 96 to 106 : CROSS SECTION : MATEBA SUD KAPITA  
 ROW 107 to 113 : CROSS SECTION : MATEBA AMONT  
 ROW 114 to 120 : CROSS SECTION : MATEBA CENTRAL

ROW 121 to 127 : CROSS SECTION : MATEBA AVAL

In data file RATING 3 we identify :

ROW 1 to 13 : CROSS SECTION : AMONT OISEAUX 1  
 ROW 14 to 23 : CROSS SECTION : AMONT OISEAUX 2  
 ROW 24 to 41 : CROSS SECTION : OISEAUX NORD 1  
 ROW 42 to 51 : CROSS SECTION : OISEAUX NORD 2  
 ROW 52 to 66 : CROSS SECTION : BARRAJE 1  
 ROW 67 to 81 : CROSS SECTION : BARRAJE 2  
 ROW 82 to 92 : CROSS SECTION : MATEBA AMONT SUD 1  
 ROW 93 to 101 : CROSS SECTION : MATEBA AMONT SUD 2  
 ROW 102 to 111 : CROSS SECTION : MATEBA SUD MANDUDI  
 ROW 112 to 130 : CROSS SECTION : MATEBA SUD KAPITA  
 ROW 131 to 140 : CROSS SECTION : MATEBA AMONT  
 ROW 141 to 153 : CROSS SECTION : MATEBA CANTRAL  
 ROW 154 to 166 : CROSS SECTION : MATEBA AVAL

#### h).- ADDITIONAL DATA

- SLOPE : see table 4.1
- D<sub>35</sub> : 35 percent of the mixture is finer  
= 0.1567 mm.
- D<sub>65</sub> : 65 percent of the mixture is finer  
= 0.325 mm.
- D<sub>50</sub> : 50 percent of the mixture is finer  
= 0.240 mm.
- Temperature of water: 28.5 °C
- Kinematic viscosity of water:  $0.841 \times 10^{-6} \text{ m}^2/\text{sec}$
- Gravity:  $9.81 \text{ m}/\text{sec}^2$
- Water density:  $1000 \text{ Kg}/\text{m}^3$

- Sediment density:  $2650 \text{ Kg m}^{-3}$

The sediment transport will be calculated for the grain sizes between 0.2 and 0.4 mm. This will cover 100% of the bed material. The calculation will be performed for individual sieve fractions representing equal proportions of average grain sizes of 0.212, 0.254, 0.312 and 0.370 millimeters.

TABLE 4.1 DETERMINATION OF ENERGY SLOPE AND MANNING COEFFICIENT

Cross Section	Discharge Q	Area A	Mean Velocity V	Hydraulic Radius R	Chey Coefficient C <sup>†</sup>	Manning Coefficient n <sup>**</sup>	Slope S <sup>**</sup>
	m <sup>3</sup> /sec	m <sup>2</sup>	m/sec	m			
Amont Oiseaux 1	3100.97	3121.04	0.993	5.89	51	0.026	0.000064
Amont Oiseaux 2	14091.99	12930.53	1.089	5.80	53	0.025	0.000071
Oiseaux Nord 1	5819.51	5146.10	1.131	11.44	54	0.028	0.0000386
Oiseaux Nord 2	10911.55	10155.60	1.035	5.10	51	0.026	0.000080
Barrage 1	4931.51	5889.00	0.837	7.85	41	0.034	0.000053
Barrage 2	10609.51	11670.4	0.909	6.27	44	0.031	0.000068
Mataba Amont Sud 1	3345.94	5206.70	0.643	5.13	32	0.042	0.000066
Mataba Amont Sud 2	8717.39	8738.15	0.998	6.03	50	0.027	0.000066
Mataba Sud Mandudl	12816.51	13126.4	0.976	8.02	48.6	0.029	0.0000503
Mataba Sud Kapita	11400.10	9881.68	1.154	8.52	55.7	0.026	0.0000503
Mataba Amont	4832.02	5028.7	0.961	5.06	50.0	0.026	0.000073
Mataba Central	6051.44	4913.7	1.231	7.70	60.9	0.023	0.000053
Mataba Aval	7416.61	5761.5	1.287	7.04	64.8	0.021	0.000056

<sup>†</sup> Obtained from J. Amarakoon's Thesis, IUPHY/VUB, July 1986

<sup>\*\*</sup> Calculated from these equations:  $n = \frac{R^{1/4}}{C}$  ;  $V = C \sqrt{RS}$

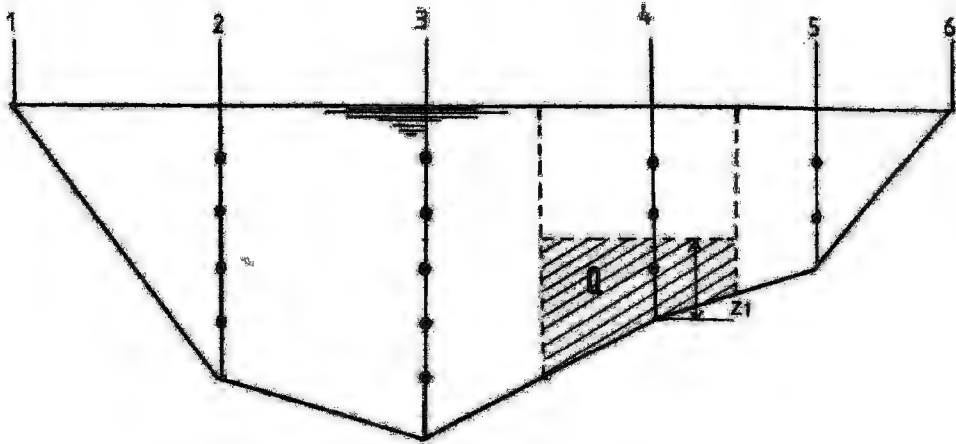


FIGURE 4.3 a SCHEMATIC REPRESENTATION OF DATA FILE: RATING1

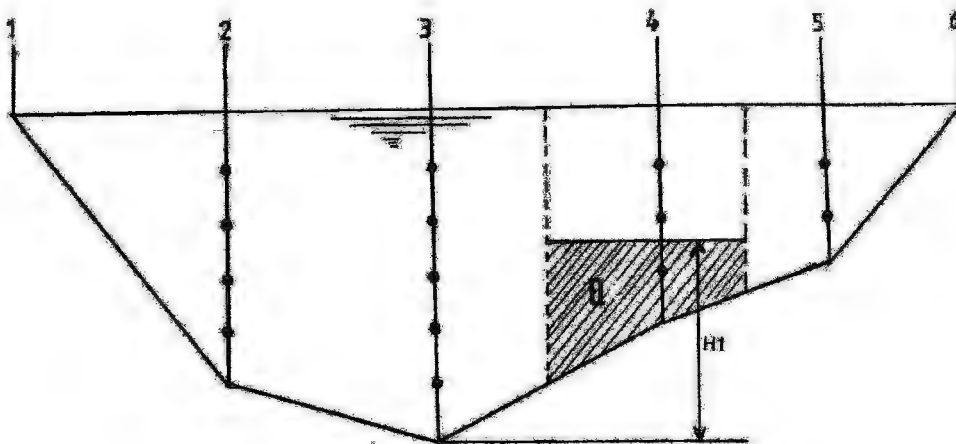


FIGURE 4.3 b SCHEMATIC REPRESENTATION OF DATA FILE: RATING2



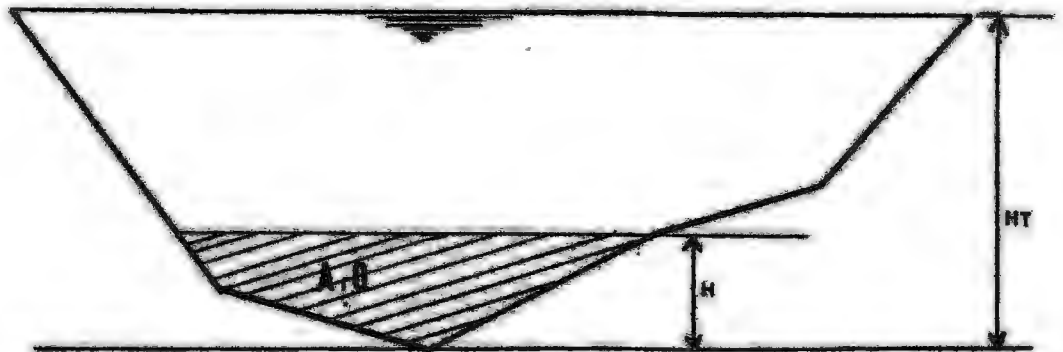


FIGURE 4.3 c SCHEMATIC REPRESENTATION OF DATA FILE: RATING3

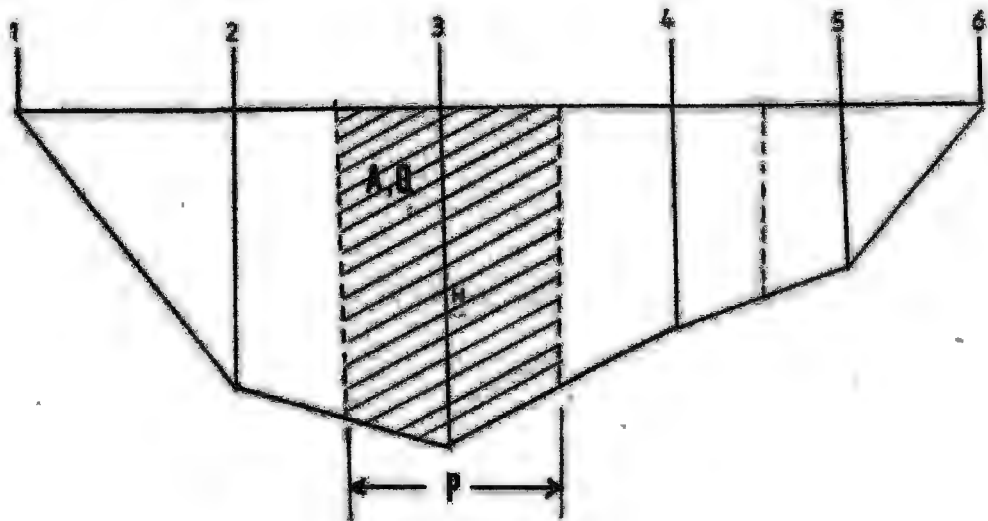


FIGURE 4.3 b SCHEMATIC REPRESENTATION OF DATA FILE: RATING4

FIGURE 4.4 a  
CROSS-SECTION GEOMETRY  
Section... Ament Dlseaux

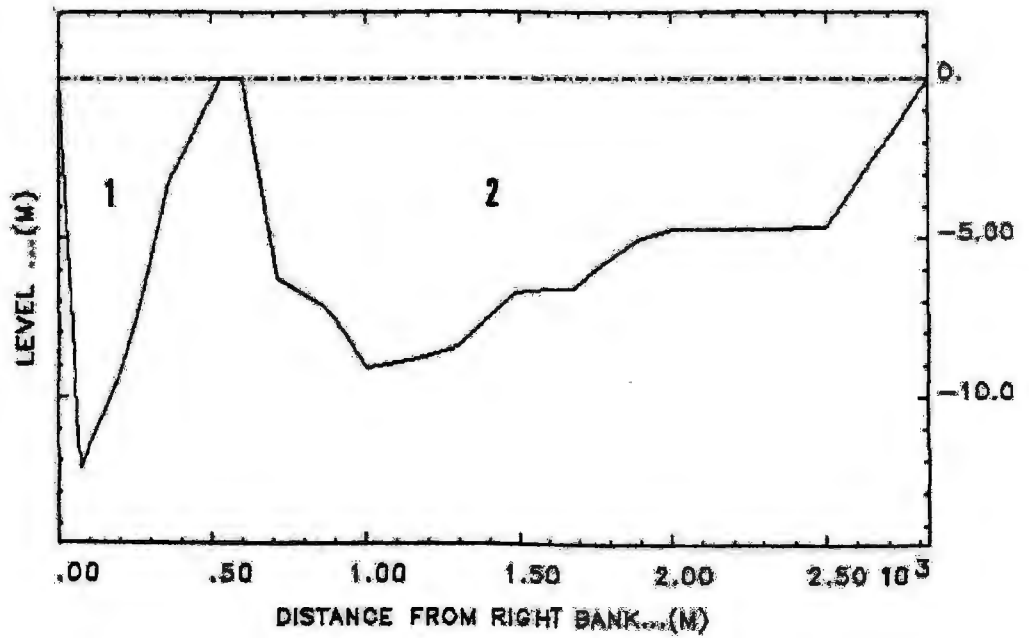


FIGURE 4.4 b  
CROSS-SECTION GEOMETRY  
Section... Dlseaux Nord

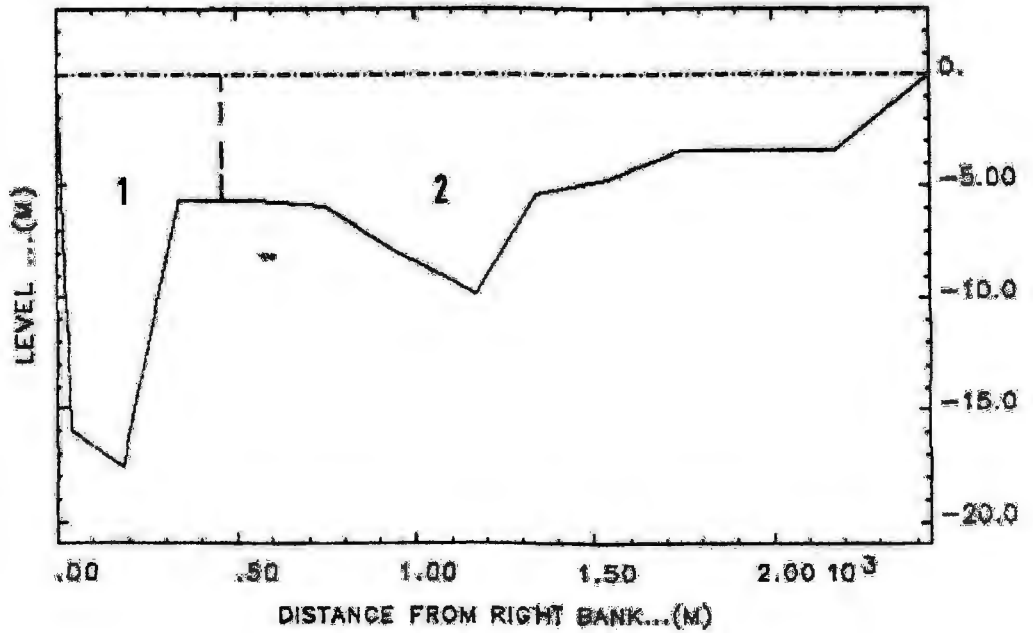


FIGURE 4.4 c

CROSS-SECTION GEOMETRY  
Section... Mateba Amont Sud

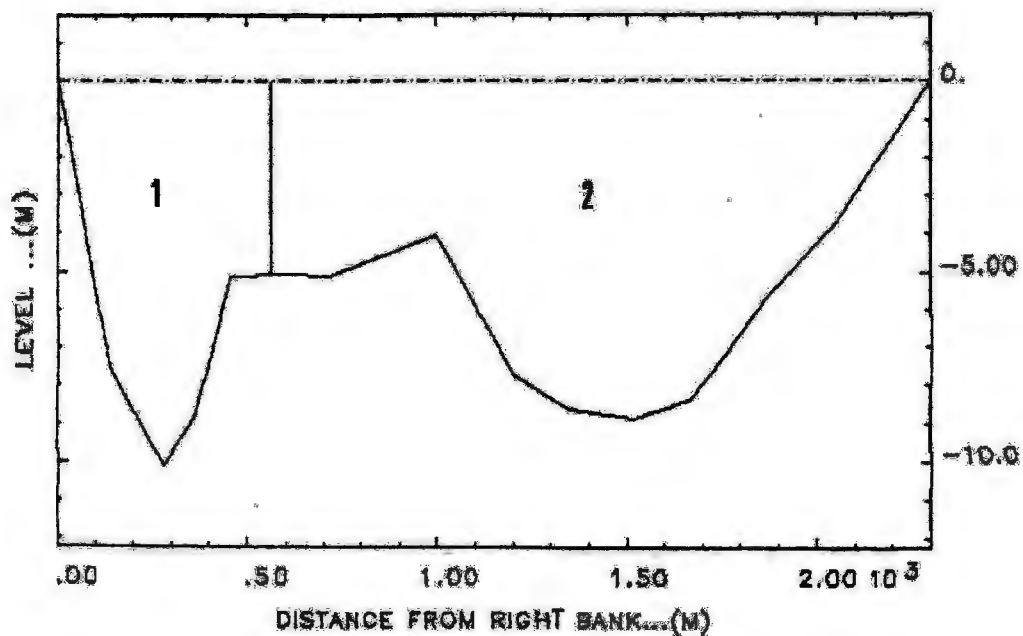


FIGURE 4.4 d

CROSS-SECTION GEOMETRY  
Section... Barraja

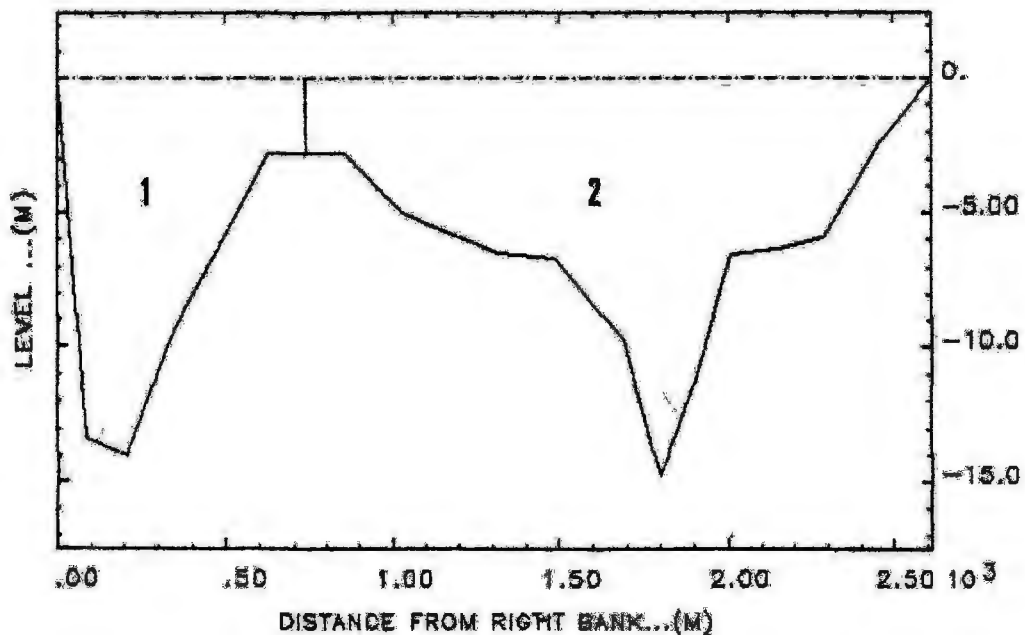


FIGURE 4.4 e

CROSS-SECTION GEOMETRY  
Section... Mateba Sud Mandud

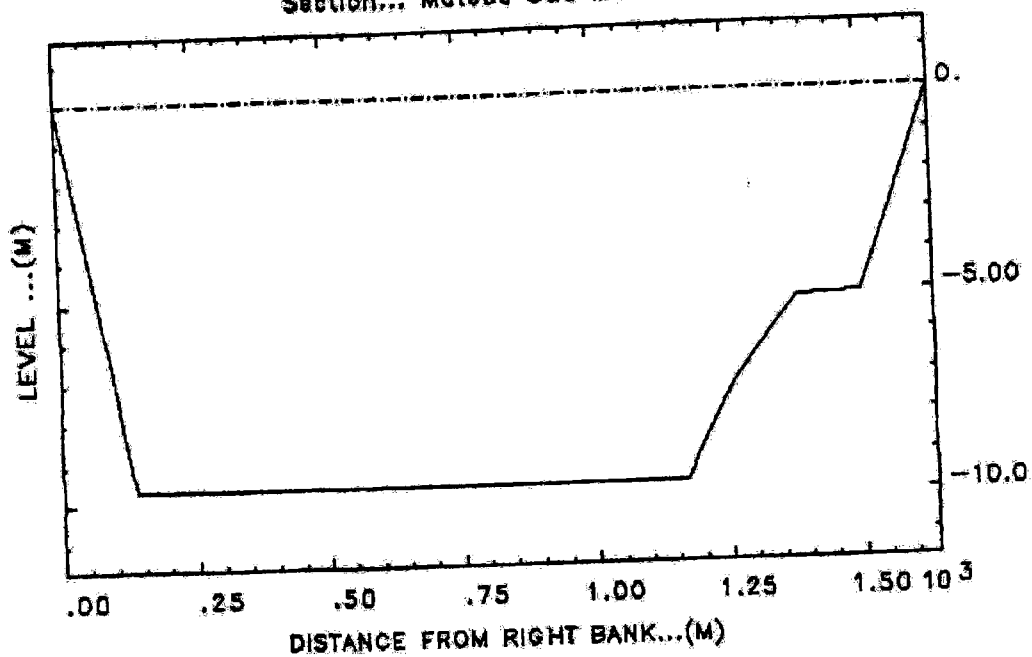


FIGURE 4.4 f

CROSS-SECTION GEOMETRY  
Section... Mateba Sud Kapita

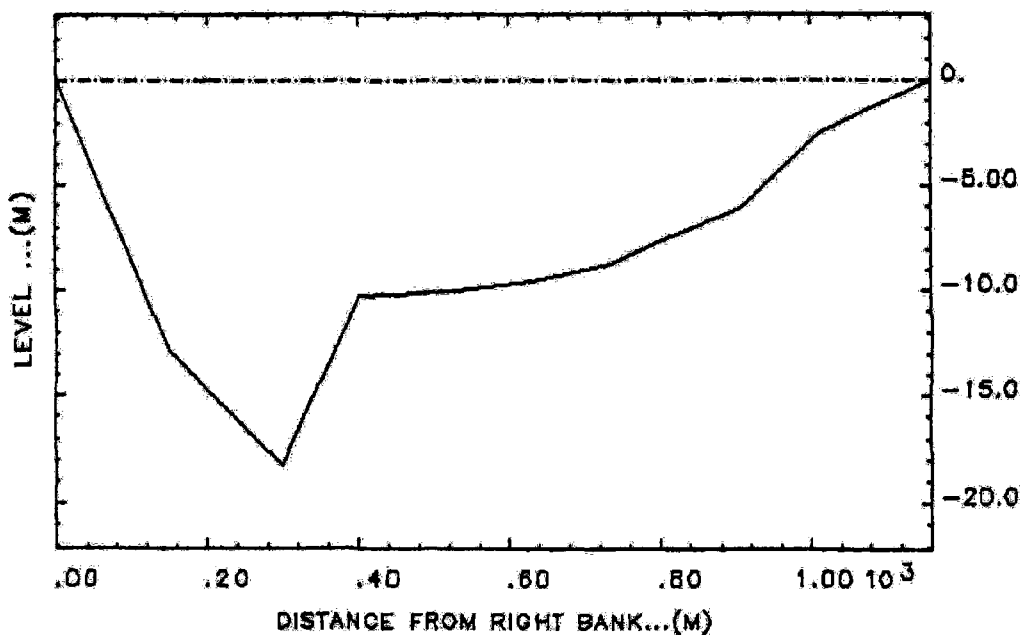


FIGURE 4.4 g  
 CROSS-SECTION GEOMETRY  
 Section... Mataba Amont

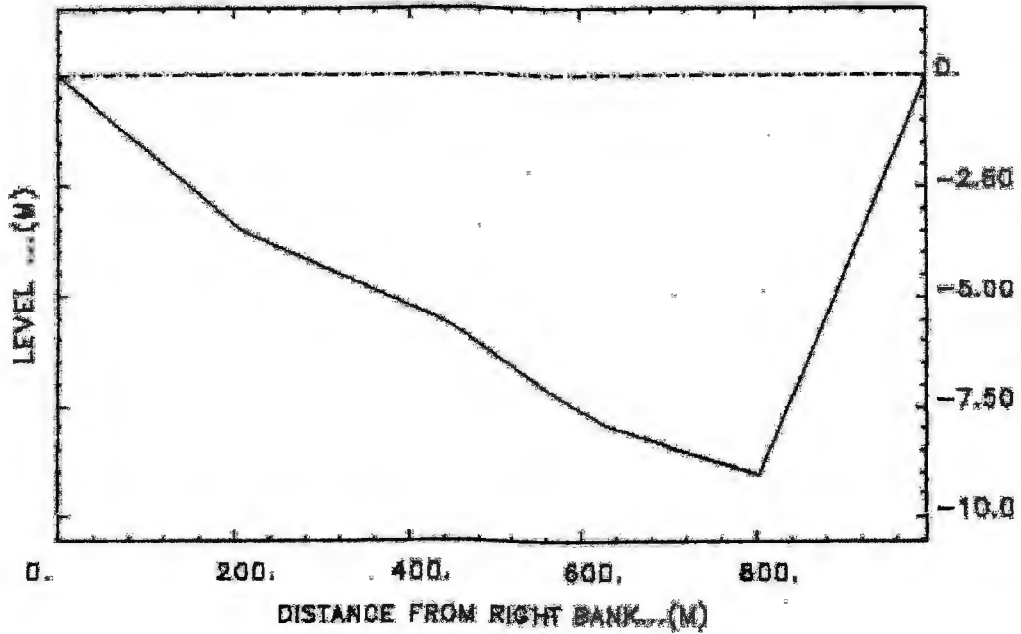


FIGURE 4.4 h  
 CROSS-SECTION GEOMETRY  
 Section... Mataba Central

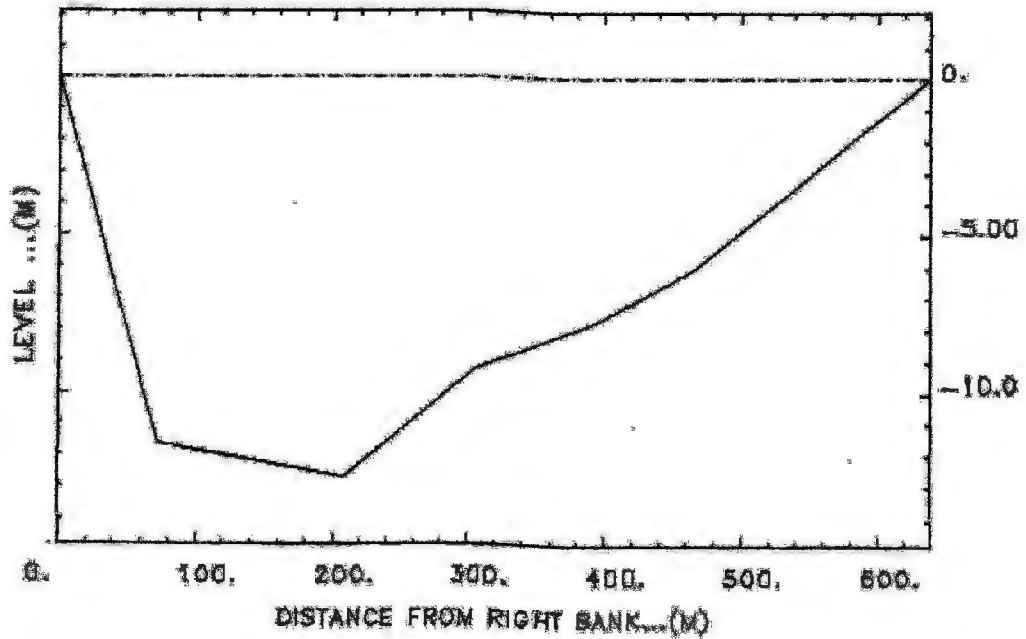


FIGURE 4.4 1 CROSS-SECTION GEOMETRY  
Section... Mateba Aval

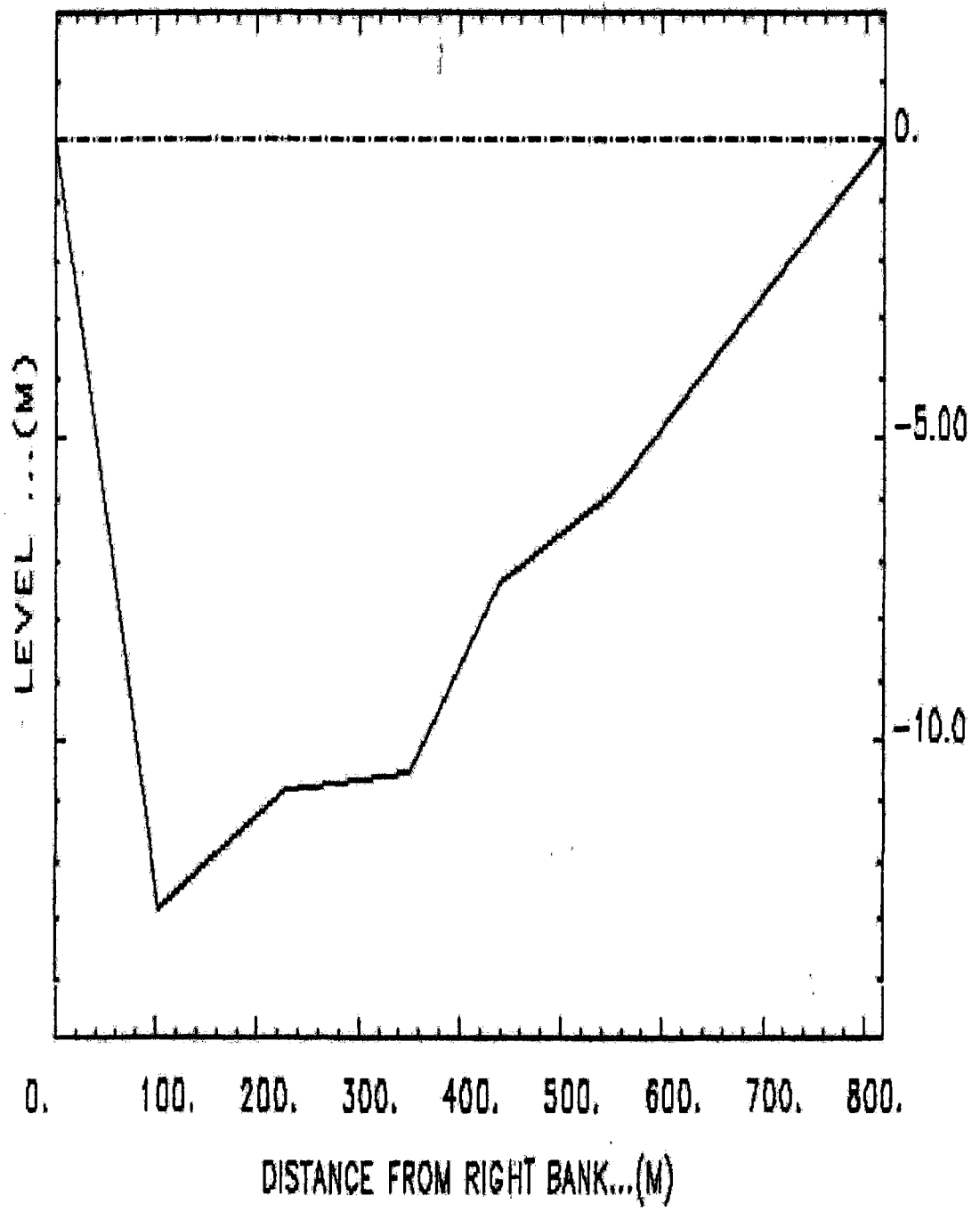


FIGURE 4.5 a RATING CURVE AND GEOMETRIC CHARAC.  
Section..Amont Oiseaux 1

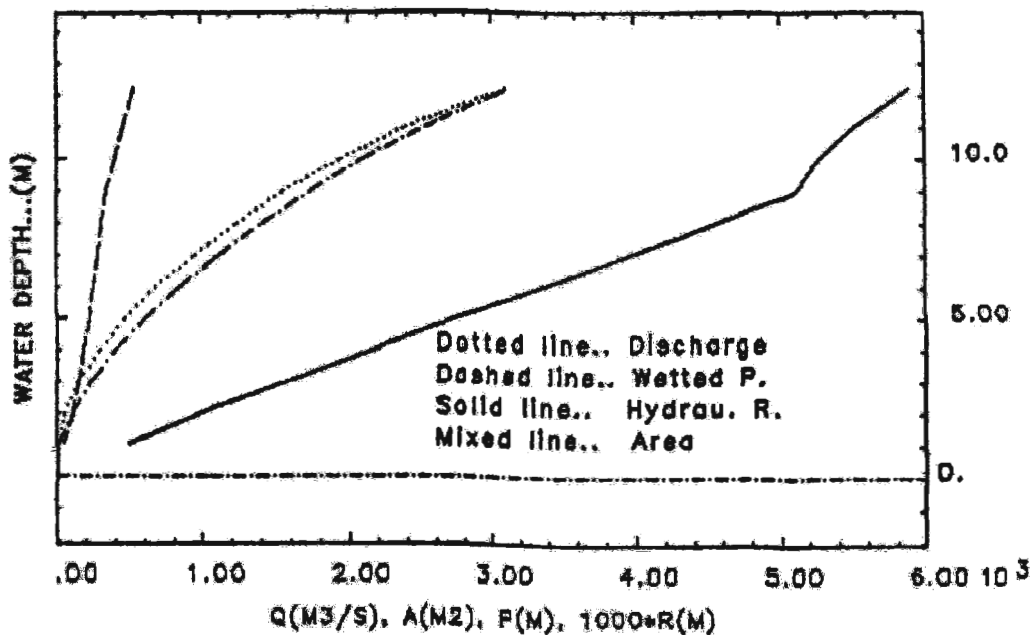


FIGURE 4.5 b RATING CURVE AND GEOMETRIC CHARAC.  
Section..Amont Oiseaux 2

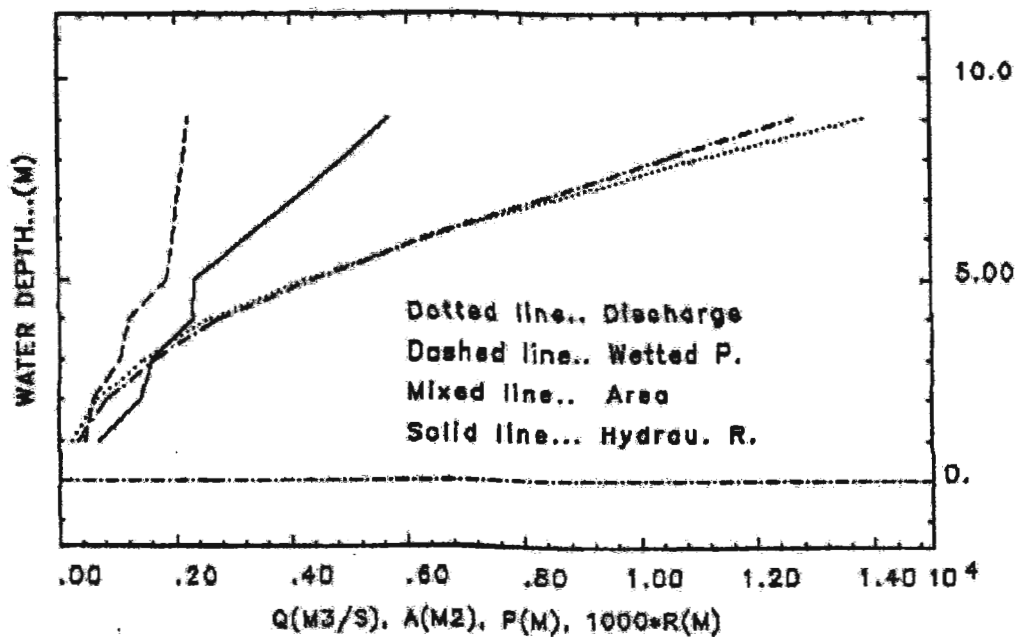


FIGURE 4.5 c RATING CURVE AND GEOMETRIC CHARAC.  
Section..Oiseaux Nord 1

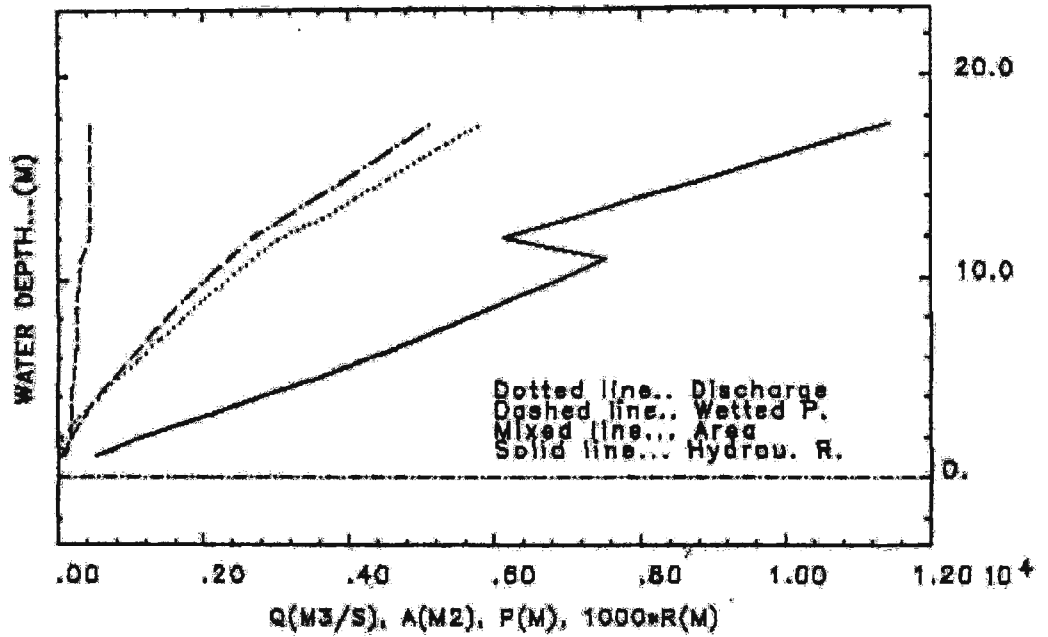


FIGURE 4.5 d RATING CURVE AND GEOMETRIC CHARAC.  
Section..Oiseaux Nord 2

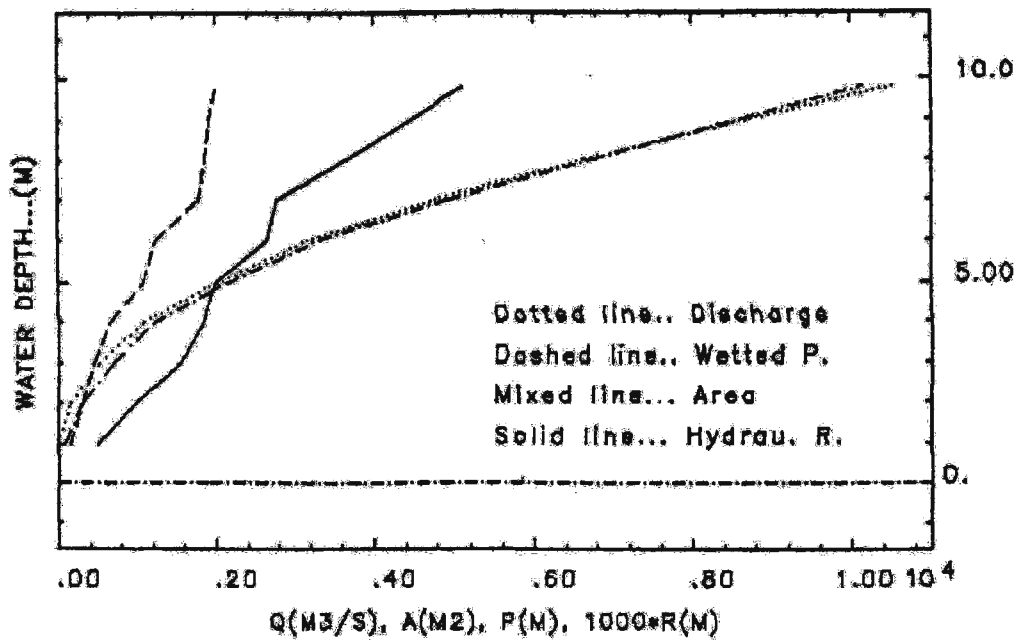




FIGURE 4.5 e RATING CURVE AND GEOMETRIC CHARAC.  
Section..Barraje 1

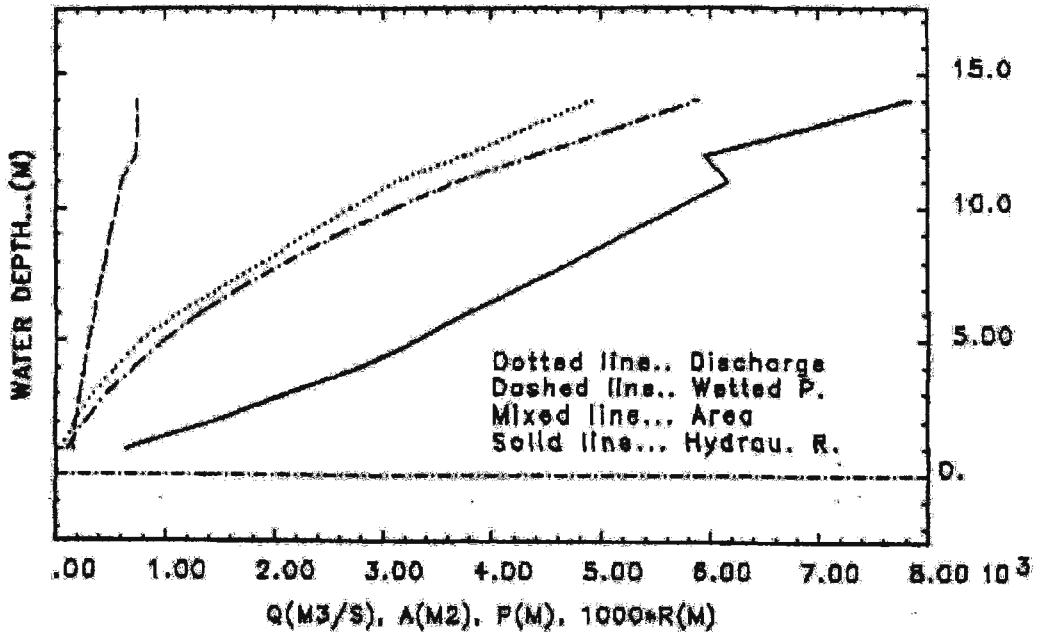


FIGURE 4.5 f RATING CURVE AND GEOMETRIC CHARAC.  
Section..Barraje 2

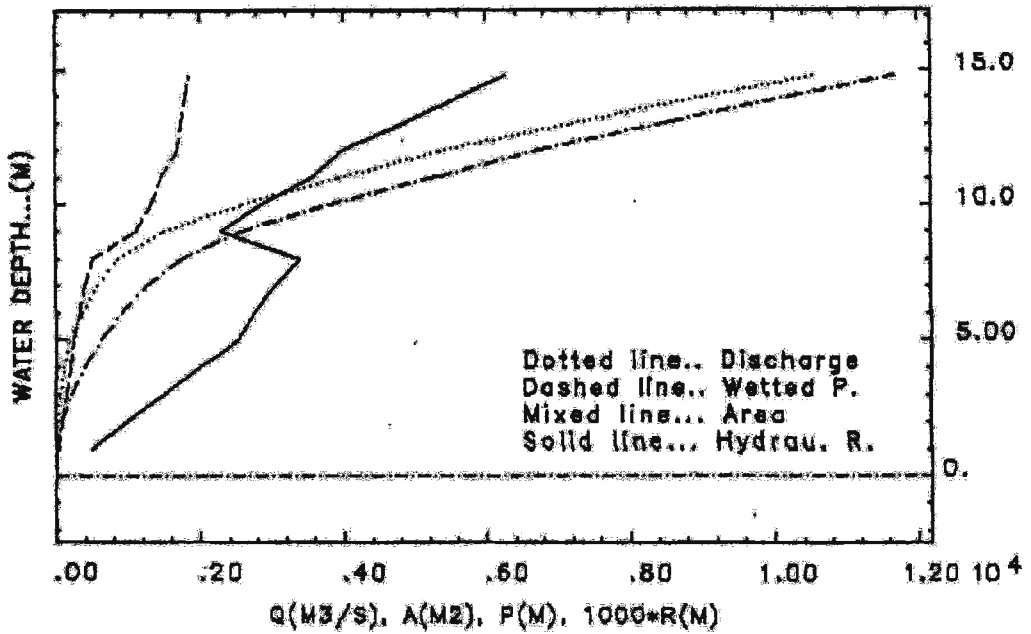


FIGURE 4.5 g RATING CURVE AND GEOMETRIC CHARAC.  
Section..Mateba Ament Sud 1

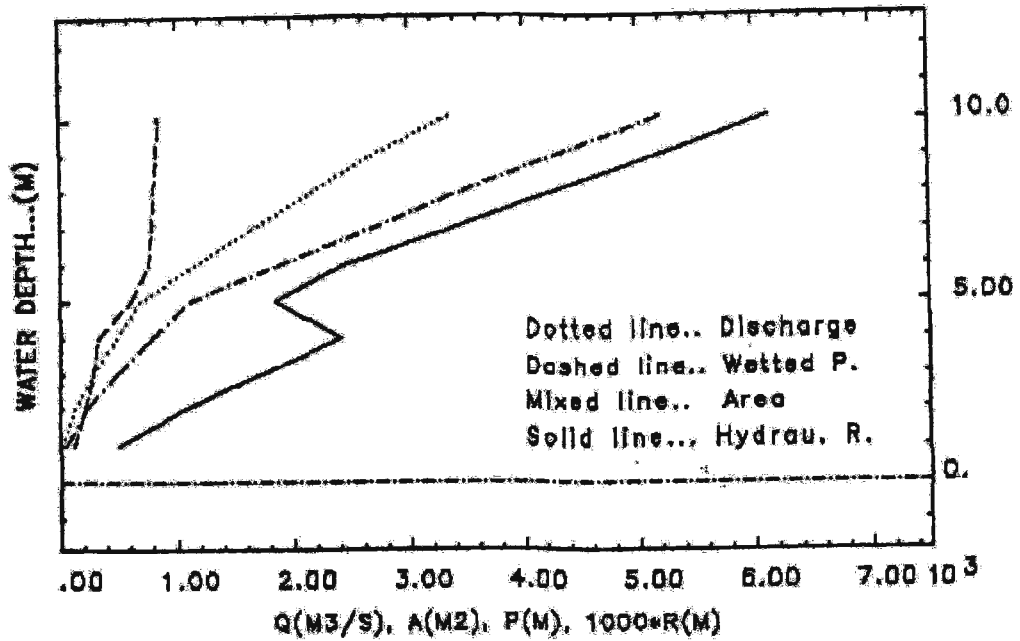


FIGURE 4.5 h RATING CURVE AND GEOMETRIC CHARAC.  
Section..Mateba Ament Sud 2

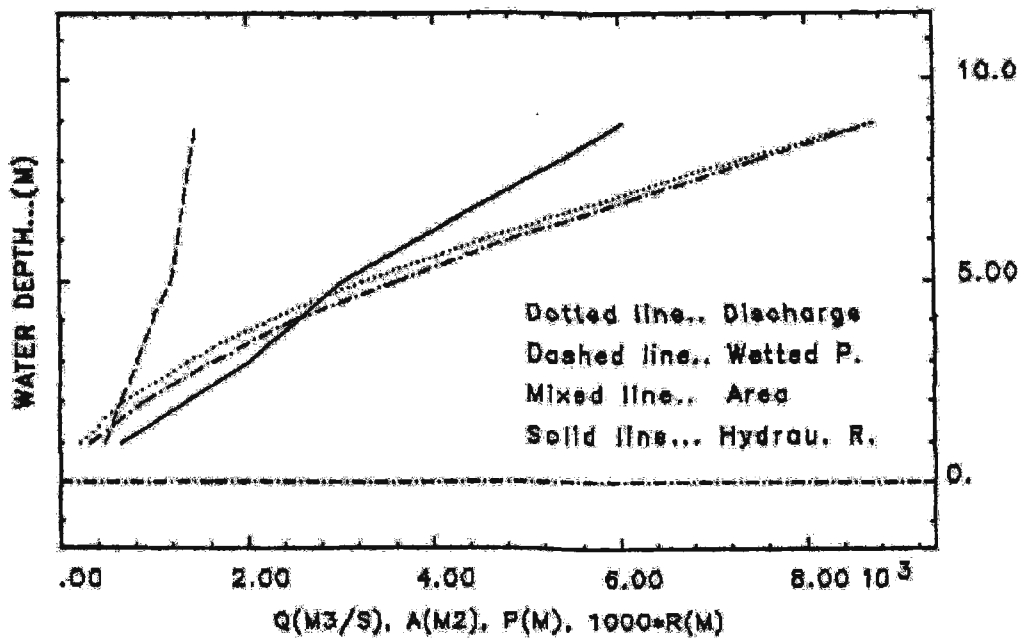


FIGURE 4.5 1 RATING CURVE AND GEOMETRIC CHARAC.  
Section..Matoba Sud Mandud

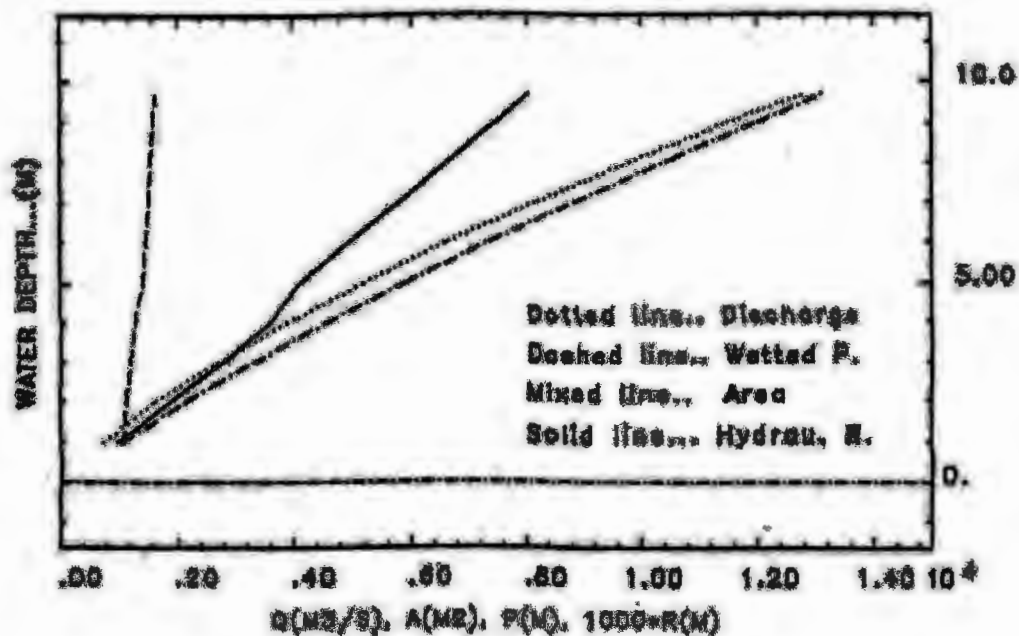


FIGURE 4.5 2 RATING CURVE AND GEOMETRIC CHARAC.  
Section..Matoba Sud Kapita

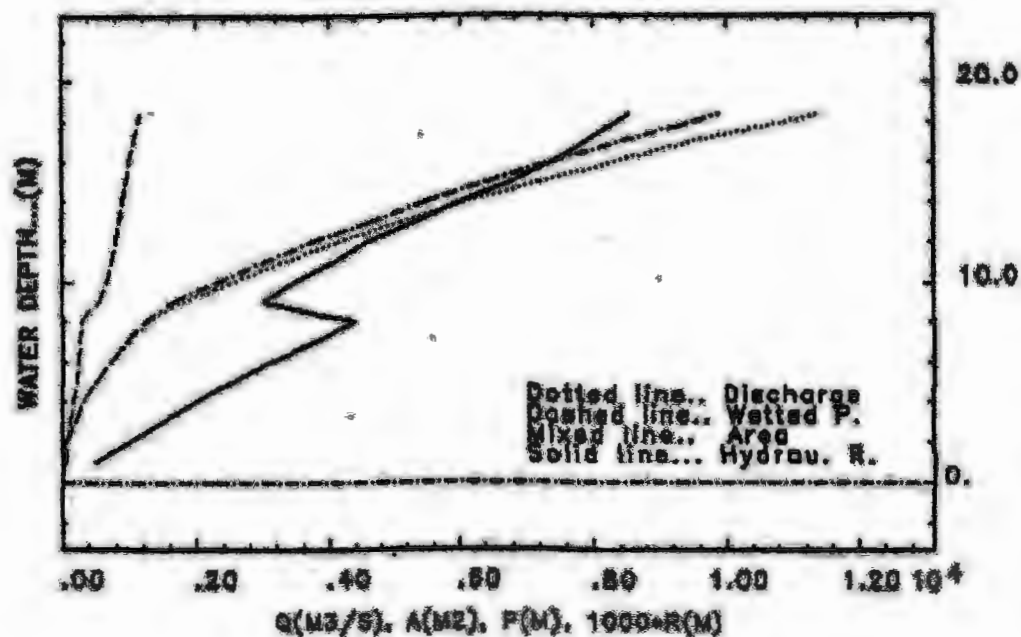


FIGURE 4.5 k

RATING CURVE AND GEOMETRIC CHARAC.  
Section...Matoba Ament

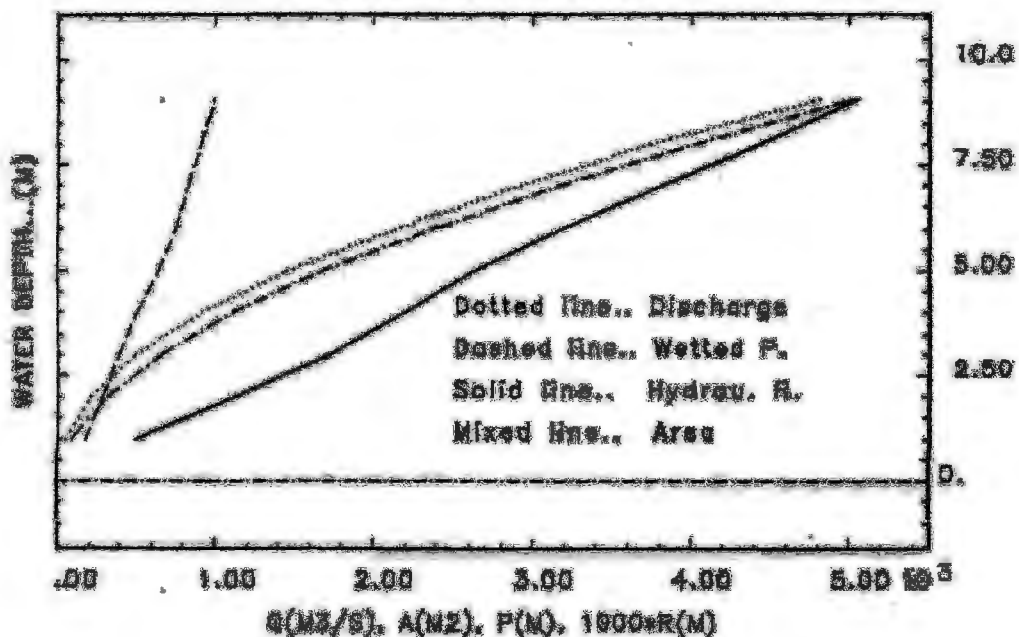
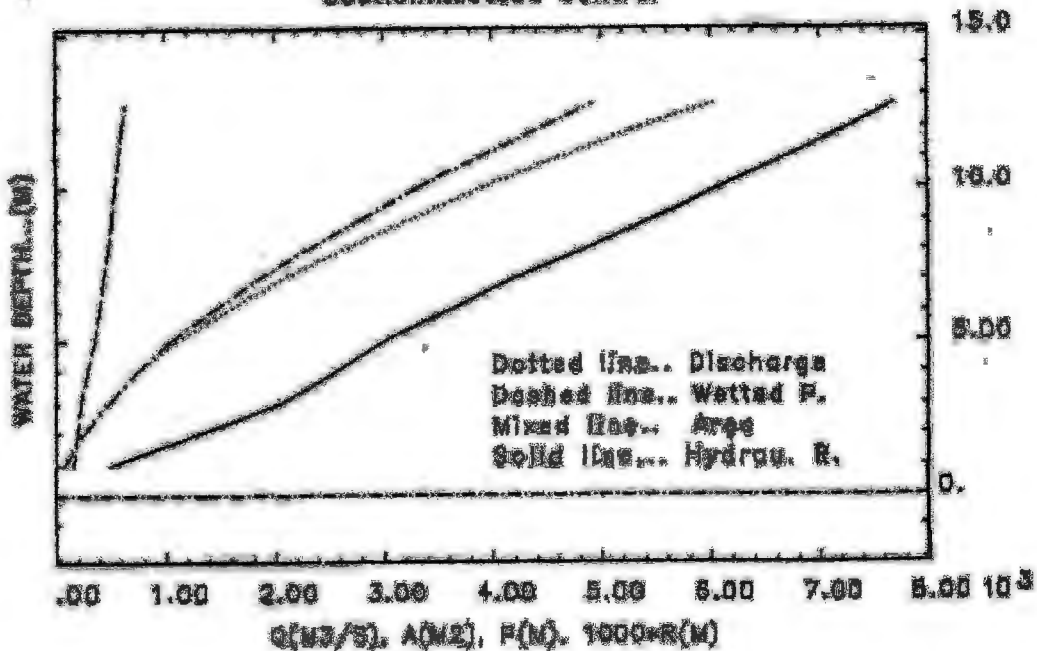


FIGURE 4.5 l

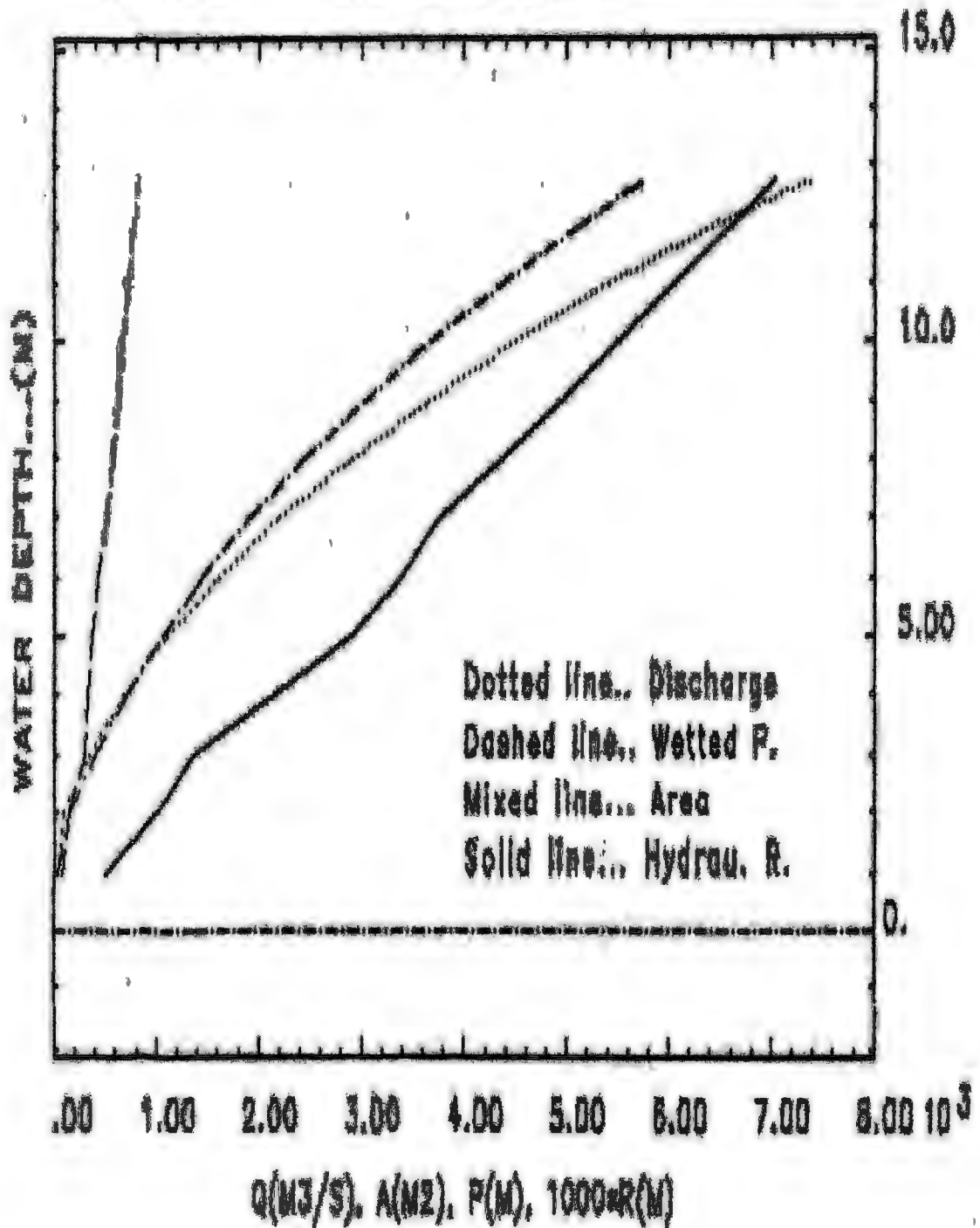
RATING CURVE AND GEOMETRIC CHARAC.  
Section...Matoba Central



# RATING CURVE AND GEOMETRIC CHARAC.

FIGURE 4.8 a

Section..Mateba Aval



## 9. APPLICATION OF EINSTEIN MODEL TO THE DATA OF ZAIRE RIVER

### 9.1 TRADITIONAL EINSTEIN PROCEDURE

The procedure calculates sediment transport for different water stages (H) in the cross section as shown in Figure 4.3c. The calculation is ended when the stage corresponding to the total water depth or total discharge is reached.

In order to simplify the application of the model, the procedure of calculations is divided into two parts: (1) the hydraulic calculations and (2) the sediment transport calculations.

#### 9.1.1 HYDRAULIC CALCULATIONS

The necessary data are provided from data file RATINGS (represented in figures 4.5a to 4.5m). Additional data such as diameter of particles, kinematic viscosity of water, gravity, water density, sediment density and slope are included into the main program EINSTEN. The hydraulic calculations were carried out in tabulated form consisting of 23 steps as follows:

STEP 1: An analysis of the equations shows that the most direct approach is obtained if values of the hydraulic radius with respect to the grain,  $R^*$ , are assumed. Various values are assumed to cover the entire discharge range desired.

- STEP 2 : From  $R'$  the corresponding friction velocity  $u_*'$  is calculated using equation 3.8 .
- STEP 3 : The thickness of the laminar sublayer  $\delta$  is obtained from equation  $\delta = 11.6 \nu / u_*'$
- STEP 4 : With  $K_s = D\delta$  the values of  $K_s / \delta$  are calculated.
- STEP 5 : The correction  $\alpha$  for the transition from smooth to rough boundaries may be read from graph of figure 3.2 .
- STEP 6 : The values of the apparent roughness  $\Delta = K_s / \alpha$  are calculated.
- STEP 7 : The average flow velocity  $\bar{U}$  is calculated from equation 3.9
- STEP 8 : Next, for the determination of the frictional contribution of the channel irregularities, the parameter  $\psi'$  is calculated according to equation

$$\psi' = \frac{K_s \bar{U}}{R' u_*'}$$

The parameter  $\psi'$  is dimensionless.

- STEP 9 : From figure 3.3, the values of  $\bar{U} / u_*'$  are read for the  $\psi'$  values.
- STEP 10: The velocity  $u_*''$  due to channel irregularities,  $u_*''$  is calculated.
- STEP 11: The hydraulic radius with respect to channel irregularities, ( $R''$ ) is calculated from equation (3.4).
- STEP 12: The two components  $R'$  and  $R''$  are usually the only components of the hydraulic radius  $R$  and

It is calculated from equation (3.4) .

- STEP 13: The total shear velocity is calculated from equation  $u_* = \sqrt{g.R.S}$
- STEP 14: The water elevation ( d ) is calculated as function of R.  $d = a + b.R$  ; (a, b are constants).
- STEP 15: The cross section area is obtained from description of the section for a given d ( see figure 4.5 ).
- STEP 16: The wetted perimeter is obtained from description of the cross section for a given d ( see figure 4.5 ).
- STEP 17: The flow discharge ( Q ) is obtained from the rating curve of figure 4.5, for a given d .
- STEP 18: The calculated discharge ( Q<sub>1</sub> ) is obtained from equation:  $Q_1 = \bar{u}.A$  .
- STEP 19: The characteristic distance X is calculated from equation .
- STEP 20: The pressure correction-term Y is calculated from figure 3.6
- STEP 21:  $\beta_x$  (logarithmic function) is equal to :  
 $\beta_x = \text{Log}( 10.6 X/\Delta )$ . See equation 3.23
- STEP 22: The value of  $(\beta / \beta_x)^2$  is calculated with  
 $\beta = \text{Log}(10.6)$
- STEP 23: The Einstein's Transport parameter PE is calculated from equation 3.52 .



### 3.1.2 SEDIMENT TRANSPORT CALCULATION

The sediment transport is calculated for the individual grain size fractions of the bed and for the entire range of discharges. In this respect, it is advantageous to distinguish in a separate table the steps which are common to all grain sizes and those which must be performed separately for each grain size. As the calculation of transport rate is performed for the flow rates given in Hydraulic Calculations it is convenient to summarize the calculation as follows:

- STEP 1 : The representative grain sizes are chosen at the geometric mean of the two size limits of each sieve fraction (  $D$  ) .
- STEP 2 : The fraction  $i_p$  of the bed material for the same sieve fraction is determined as an average of all bed samples available for the river.
- STEP 3 : The same  $R'$  values are used as in hydraulic calculations.
- STEP 4 : Next,  $\psi$  is calculated using equation 3.23 .
- STEP 5 : The ratio  $D/X$  is calculated for  $X$  given in step 19 of hydraulic calculations.
- STEP 6 : The  $f$  values are read from figure 3.2 in terms of  $D/X$  ( $f$  is called hiding factor).
- STEP 7 : The parameter  $\psi_0$ , intensity of shear for individual grain size, is calculated according to equation 3.26 .
- STEP 8 : The intensity of transport for individual

grain size  $\phi_*$  is read in function of  $\psi_*$  from figure 3.7

STEP 9: The bed-load rate  $i_B \cdot q_B$  is calculated from  $\phi_*$  using equations (3.15) and (3.16) in the form

$$i_B \cdot q_B = \phi_* \cdot i_B \cdot \rho_s \cdot g^{3/2} \cdot D^{3/2} \left( \frac{\rho_s - \rho}{\rho} \right)^{1/2} \text{ in } \left( \frac{\text{Newton}}{\text{m} \cdot \text{sec.}} \right)$$

to convert in (Ton/m.day), we multiply by 86.4/9.81 .

STEP 10:  $i_B \cdot Q_B$ , bed-load rate in weight per unit time for entire cross section  $i_B \cdot Q_B = i_B \cdot q_B \cdot P$

STEP 11:  $\sum i_B \cdot Q_B$  Bed load rate for all size fractions.

STEP 12:  $A_B$  is calculated as  $A_B = 2D/d$

STEP 13:  $W_s$  the settling velocity is read from figure 3.4 .

STEP 14: The exponent  $z$  is calculated according to equation  $z = W_s / 0.4u_*'$

STEP 15: The integral  $I_1$  is calculated from equation 3.47 as function of  $A_B$  and  $z$ . Also may be read from figure 3.8 .

STEP 16: The integral  $I_2$  is calculated from equation 3.47 as function of  $A_B$  and  $z$ . Also may be read from figure 3.9 .

STEP 17: The expression  $(PE \cdot I_1 + I_2)$ , is calculated.

STEP 18: The suspended sediment transport per unit width and time for a size fraction is calculated according the equation:

$$i_s \cdot q_s = i_B \cdot q_B (PE \cdot I_1 + I_2)$$

STEP 19: The suspended sediment transport per unit time

for a size fraction is calculated :

$$i_s \cdot Q_s = i_s \cdot Q_s \cdot P \quad (P = \text{wetted perimeter})$$

STEP 20: The total suspended sediment transport per unit time for all size fractions is calculated

$$\sum i_s \cdot Q_s$$

STEP 21: The total sediment rate (bed-load plus suspended load) is calculated by summation of the values calculated in step 11 and step 20.

$$\sum i_T \cdot Q_T = \sum i_b \cdot Q_b + \sum i_s \cdot Q_s$$

TABLE 5.1 RELATIONSHIP BETWEEN THE SYMBOLS USED IN ANALYTICAL FORMULAS AND COMPUTER PROGRAM

HYDRAULIC CALCULATIONS

STEP	FORMULAS	PROGRAM	UNITS	REMARK
1	$R^1$	R1	m	Assumed
2	$u_b$	U1	m/s	$u_b = \sqrt{g \cdot R^1 \cdot S}$
3	$\delta$	DELTA1	m	$\delta = 11.6 \sqrt{u_b}$
4	$K_b/g$	M	-	with $K_b = D \delta^5$
5	$x$	X1	-	figure 3.2
6	$\Delta$	DELTA2	m	$\Delta = K_b/x$
7	$\bar{u}$	U	m/s	equation 3.7
8	$\psi$	C	-	$\psi = (g - f) \cdot x \cdot \bar{u} / u_b^3$
9	$u/u_b$	M2	-	figure 3.3
10	$u_b^2$	U2	m/s	$u_b^2 = \bar{u} / f(\psi)$
11	$R^2$	R2	m	$R^2 = (u_b^2)^2 / gS$
12	$R$	R	m	$R = R^1 + R^2$
13	$u_g$	UC	m/s	$u_g = \sqrt{g \cdot R \cdot S}$
14	$d$	H	m	$d = a + b \cdot R$
15	$a$	A	$m^2$	figure 4.5
16	$b$	P	m	figure 4.5
17	$Q$	Q	$m^3/s$	figure 4.5
18	$Q1$	Q1	$m^3/s$	$Q1 = \bar{u} \cdot A$
19	$X =$	X2	m	equation 3.17
20	$Y$	Y	-	figure 3.6
21	$\beta_x$	BX	-	$\beta_x = \text{Log}(18.6X/A)$
22	$(\theta/R_x)^2$	BB	-	with $\beta^2 = \text{Log}18.6$
23	$PE$	PE	-	equation 3.52

TABLE 5.2 RELATIONSHIP BETWEEN THE SYMBOLS, USED IN ANALYTICAL FORMULAS AND COMPUTER PROGRAM

SEDIMENT TRANSPORT CALCULATION

STEP	FORMULAS	PROGRAM	UNITS	REMARK
1	D	D	m	data
2	$i_b$	FBI	-	data
3	$R^*$	R1	m	assumed
4	$\psi$	ISRPZ	-	equation 3.23
5	$D/X$	DD	-	-
6	$\xi$	FF	-	read from fig. 3.5
7	$\psi_s$	ISIG	-	equation 3.24
8	$\phi_s$	ISIIG	-	read from fig. 3.7
9	$i_b \cdot Q_b$	BLSF	Ton/m.day	see step 9
10	$i_b \cdot Q_b$	BLEC	Ton/day	$i_b \cdot Q_b = i_b \cdot Q_b \cdot P$
11	$\sum i_b \cdot Q_b$	TBLEC	Ton/day	-
12	$A_s$	AE	-	$A_s = 20/d$
13	$W$	W	cm/sec	read from fig. 3.4
14	$z$	Z	-	$z = W/D \cdot 480 i_b$
15	$I_1$	FI1	-	equation 3.47
16	$I_2$	FI2	-	equation 3.47
17	$PI \cdot I_1 + I_2$	IST	-	-
18	$i_b \cdot Q_b$	BMLSF	Ton/m.day	$i_b \cdot Q_b = i_b \cdot Q_b (PI \cdot I_1 + I_2)$
19	$i_b \cdot Q_b$	BMREC	Ton/day	$i_b \cdot Q_b = i_b \cdot Q_b \cdot P$
20	$\sum i_b \cdot Q_b$	BMRAS	Ton/day	-
21	$\sum i_b \cdot Q_b$	TBLSL	Ton/day	$\sum i_b \cdot Q_b = \sum i_b \cdot Q_b + \sum i_b \cdot Q_b$

## 5.2 TWO-DIMENSIONAL APPROACH

### 5.2.1 CALCULATION OF MEAN VELOCITY

The mean velocity at each vertical for a given cross section was calculated considering a panel as shown in figure 4.3d. For each panel the discharge and the area were calculated through the computer program RATING and the mean velocity was obtained according to the following relation:

$$U = Q / A$$

in each vertical.

### 5.2.2 CALCULATION OF SHEAR VELOCITY

The velocity distribution in a uniform channel flow will become stable when the turbulent boundary layer is fully developed. With the assumption that the mixing length is proportional to water depth and that the shearing stress is constant, Prandtl arrived at the following equation:

$$u = 5.75 u_* \text{Log}(y) + C$$

Where:  $u$  = velocity at any point of the vertical

$u_*$  = shear velocity

$y$  = water depth to the point in consideration

$C$  = constant

When velocity  $u$  is plotted against the logarithm of the corresponding depth, the gradient of the graph will be  $5.75u_*$  and  $u_* = \text{gradient}/5.75$

These calculations were made through the computer program RATING and the results are presented on data file

RATING4 (Appendix 10).

### 5.2.3 CALCULATION OF SEDIMENT TRANSPORT RATE

The necessary data are provided from data file RATING4. The same procedure described in 3.1 was used, as for hydraulic calculations and sediment transport calculations.

The additional data mentioned in 4.2 are included in the computer program EINSTE1, which performs the calculations for 127 verticals (13 cross sections). The results are presented in appendix 14.

## 5.3 GENERAL DESCRIPTION OF COMPUTER PROGRAMS

### a.- PROGRAM CHANGE1

This program transforms the data obtained from original data base and feed it to the computer program RATING.

The most important change made to the data is the transformation of the coordinates in distance from right bank to each vertical as shown in data file ZADATA1 and ZADATA2 (appendix 3 and 5).

### b.- PROGRAM RATING:

This is the most important program in transforming the data on file ZADATA2 into different ways. At the first time the values of  $Z$  and  $V$  are transformed into  $Z1$  and  $Q$ , then in values of  $H1$  and  $Q$  and finally in  $H$  and  $Q$  for the entire cross section. The Figures 4.3a to 4.3d show the sequential procedure on how the program RATING transforms the data on file ZADATA2 in the useful way which are represented in the file RATING3. This transformation uses the computer program

## EINSTEN.

Finally, the program calculates the requirement data for the program EINSTEN which was developed to calculate the sediment transport in two-dimensional approach.

The meaning of  $Z$ ,  $Z_1$ ,  $M_1$ ,  $H$ ,  $V$  and  $Q$  were discussed in 4.2 and the output files RATINB1, RATINB2, RATINB3 and RATINB4 are shown in appendices 7, 8, 9 and 10 respectively.

### 6.- PROGRAM EINSTEN:

This computer program is used to calculate the sediment transport rate in the traditional way. It has the following structure:

#### - INPUT FILE: RATINB3

This file contains the hydraulic and geometric characteristic data of 13 cross sections as described in 4.2(g). As the program runs interactively some additional informations are given during the time of running. These informations are tabulated in table 5.3.

#### - OUTPUT FILE:

SEDIM1 : Results of the application of Einstein model

SEDIM2 : Data file for plott program.

perform: /Save, SEDIM2 = SEDPL1, in case of cross section Amont Oiseaux 1.



TABLE 5.3 ADDITIONAL INFORMATION REQUIRED BY THE  
COMPUTER PROGRAM EINSTEIN

cross section	water depth HT	slope S	increment of R1 DIF
Amont Oiseaux 1	12.20	0.000064	0.15
Amont Oiseaux 2	9.04	0.000071	0.15
Oiseaux Nord 1	17.58	0.0000386	0.25
Oiseaux Nord 2	14.04	0.000053	0.20
Barraje 1	14.04	0.000053	0.20
Barraje 2	14.77	0.000068	0.15
Mateba Amont Sud 1	10.09	0.000066	0.15
Mateba Amont Sud 2	8.89	0.000066	0.15
Mateba Sud Mandud	9.69	0.0000503	0.20
Mateba Sud Kapita	18.31	0.0000503	0.20
Mateba Amont	9.05	0.000073	0.10
Mateba Central	12.70	0.000053	0.15
Mateba Aval	12.80	0.000056	0.15

d.- PROGRAM EINSTEIN:

This program is used to calculate the sediment transport rate into two-dimensional approach. The calculations are performed for each vertical along the cross sections .

The program was runned considering 4 different energy slopes and 4 different particle diameters (D50) and the result corresponding to the cross sections Mateba Amont, Mateba Central and Mateba Aval are shown in Figures 6.3 and 6.4.

g. - PROGRAM PLSEDI:

This program is used for plotting the bed-load and the suspended-load sediment transport for a given cross section.

h. - PROGRAM PLSLOPE:

This program is used for plotting the variation of sediment transport along the cross section for different slopes and different particle diameters.

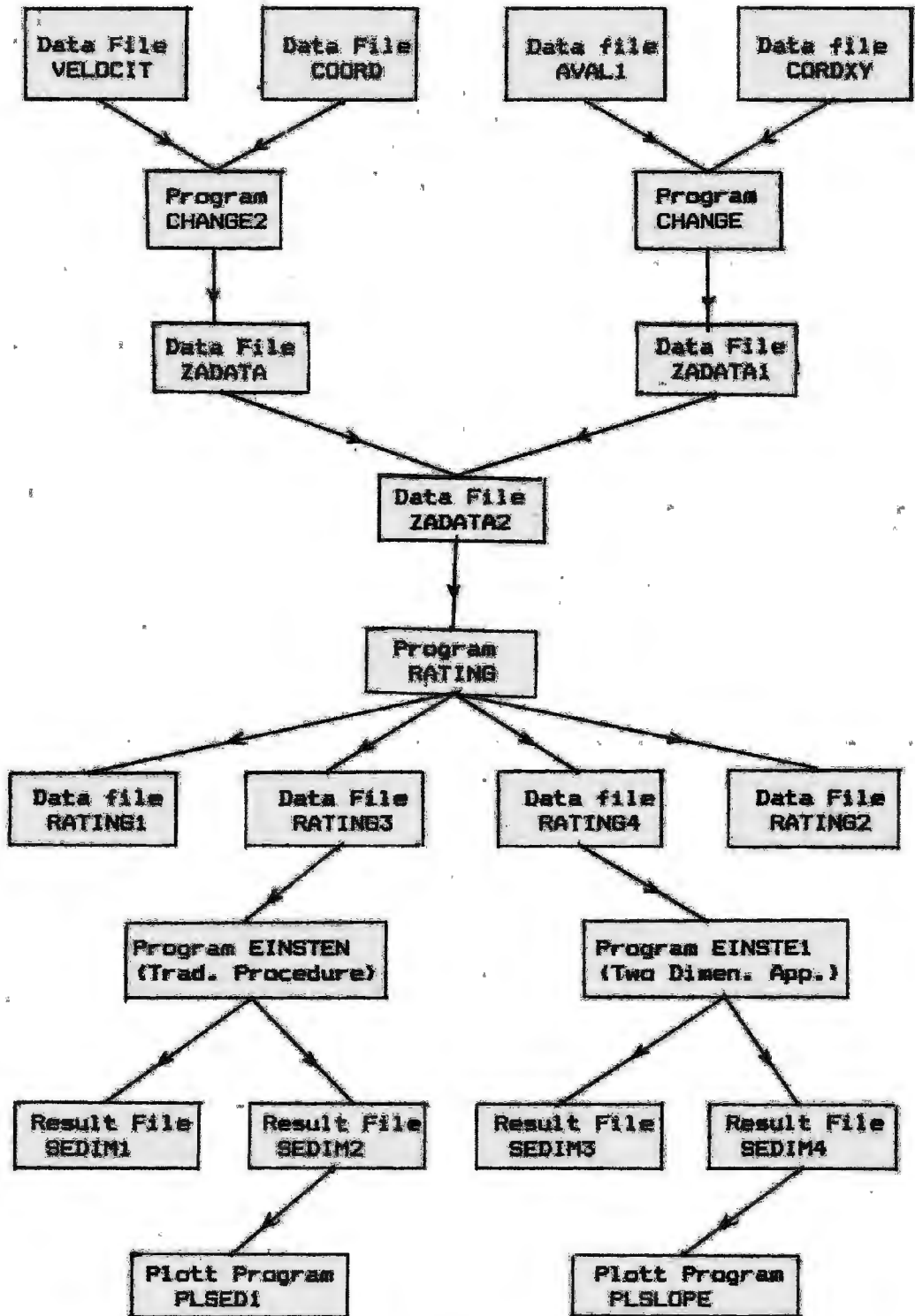


FIGURE 5.1 : FLOWCHART OF EINSTEIN MODEL

## 6. RESULTS

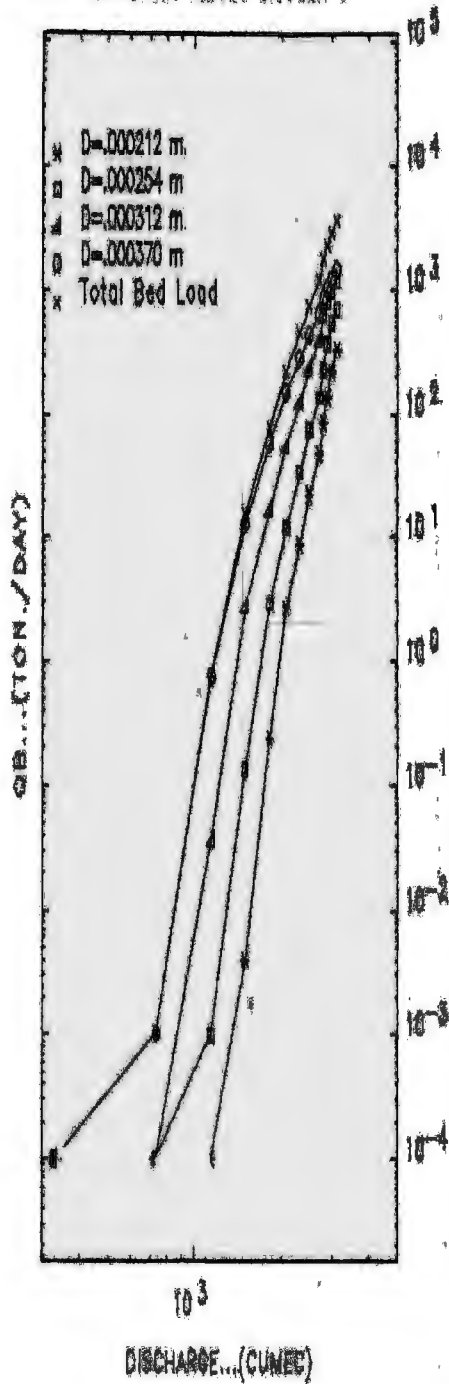
TABLE : 6.1. RESULTS OF EINSTEIN MODEL

( TRADITIONAL PROCEDURE )

CROSS SECTION	BED-LOAD (Ton/day)	SUSPENDED LOAD (Ton/day)	TOTAL LOAD (Ton/day)
AMONT OISEAUX 1	3,620.047	2,463.456	6,083.504
AMONT OISEAUX 2	13,108.429	8,189.790	21,298.219
OISEAUX NORD 1	3,110.902	2,133.106	5,244.008
OISEAUX NORD 2	9,519.284	5,350.864	14,869.859
BARRAJE 1	5,366.090	3,752.013	9,118.103
BARRAJE 2	9,836.514	5,812.749	15,649.263
MATEBA AMONT SUD 1	3,250.593	1,640.664	4,891.257
MATEBA AMONT SUD 2	7,077.607	4,020.999	11,098.606
MATEBA SUD MANDUD	8,334.358	4,835.455	13,169.813
MATEBA SUD KAPITA	8,840.458	6,407.343	15,247,801
MATEBA AMONT	1,904.103	771.759	2,675.862
MATEBA CENTRAL	3,904.787	2,500.900	6,409.687
MATEBA AVAL	4,051.222	2,310.197	6,361.419

### BED LOAD SEDIMENT TRANSPORT

Section... Amont Quesaux 1



### SUSPENDED LOAD SEDIMENT TRANSPORT

Section... Amont Quesaux 1

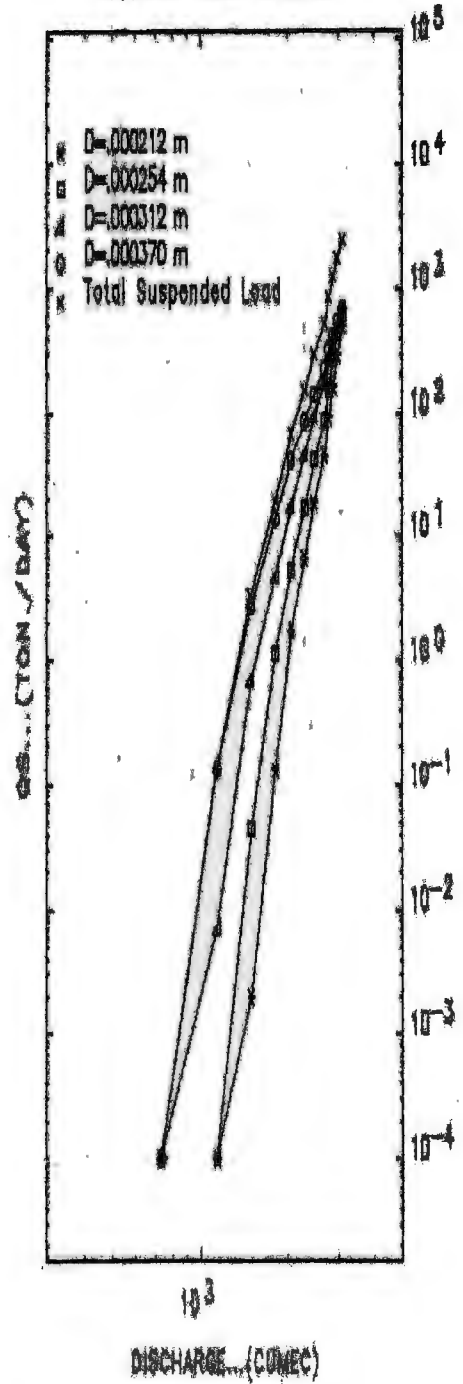
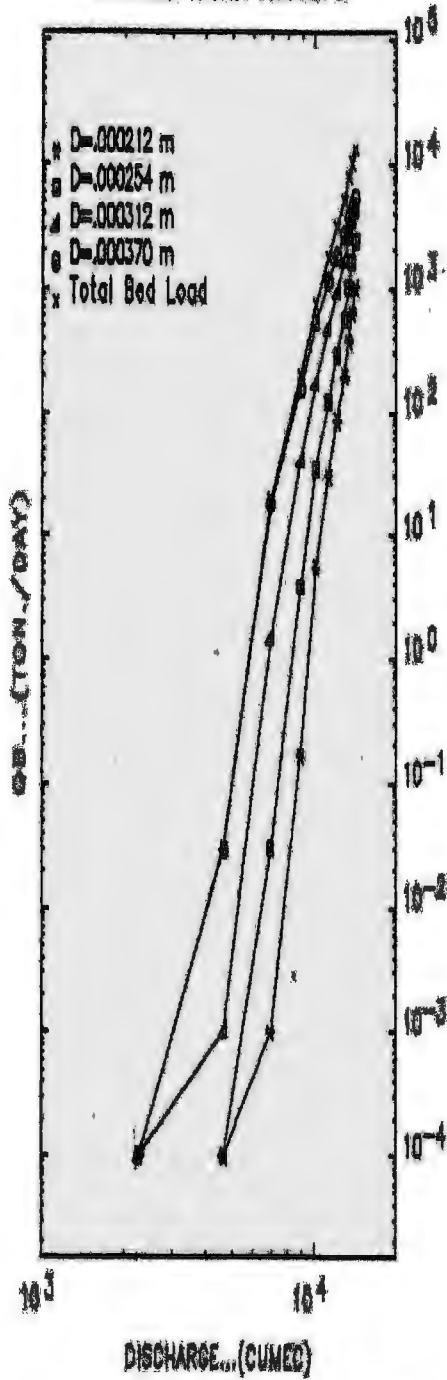


FIGURE A.1 a

BED LOAD SEDIMENT TRANSPORT

Section... Amont Oiseaux 2



SUSPENDED LOAD SEDIMENT TRANSPORT

Section... Amont Oiseaux 2

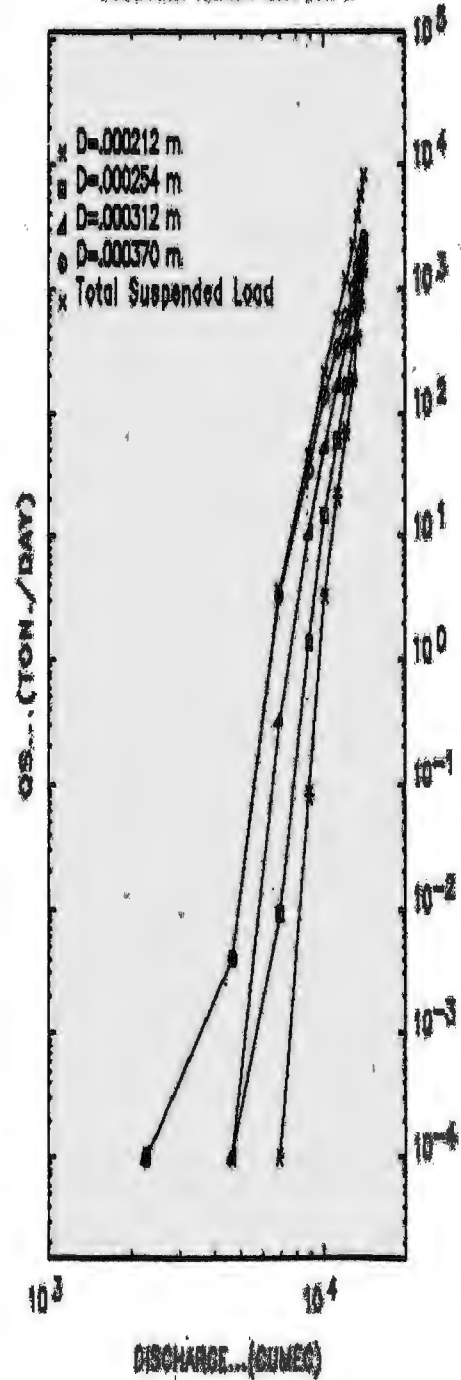
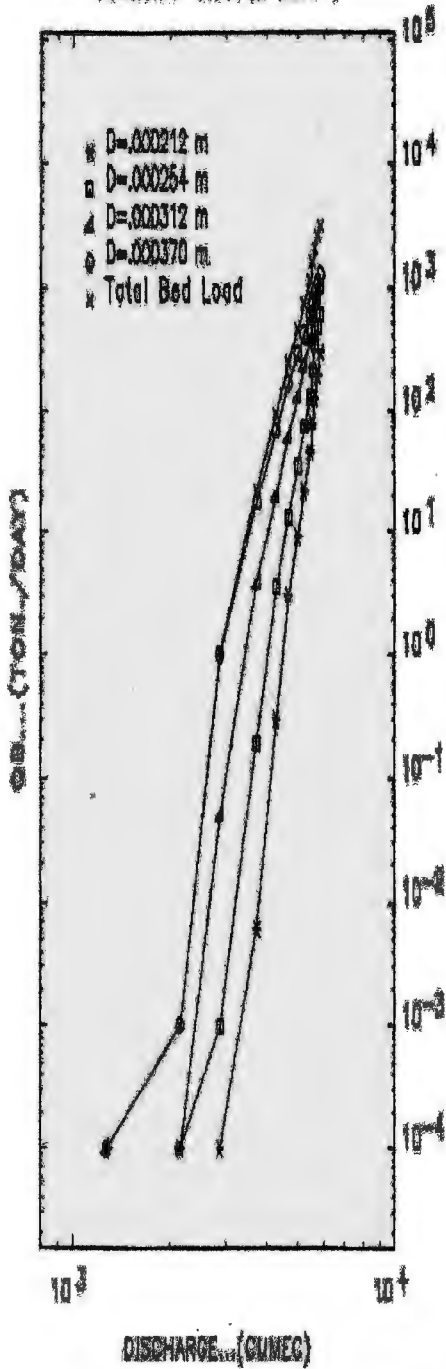


FIGURE 6.1 b

BED LOAD SEDIMENT TRANSPORT  
Section... Oiseaux Nord 1



SUSPENDED LOAD SEDIMENT TRANSPORT  
Section... Oiseaux Nord 1

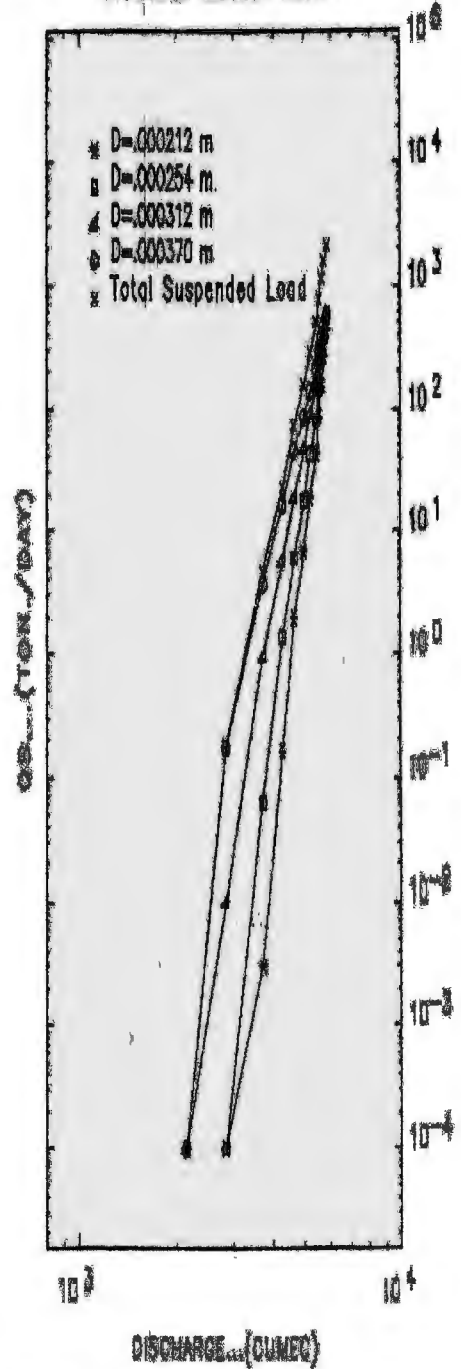
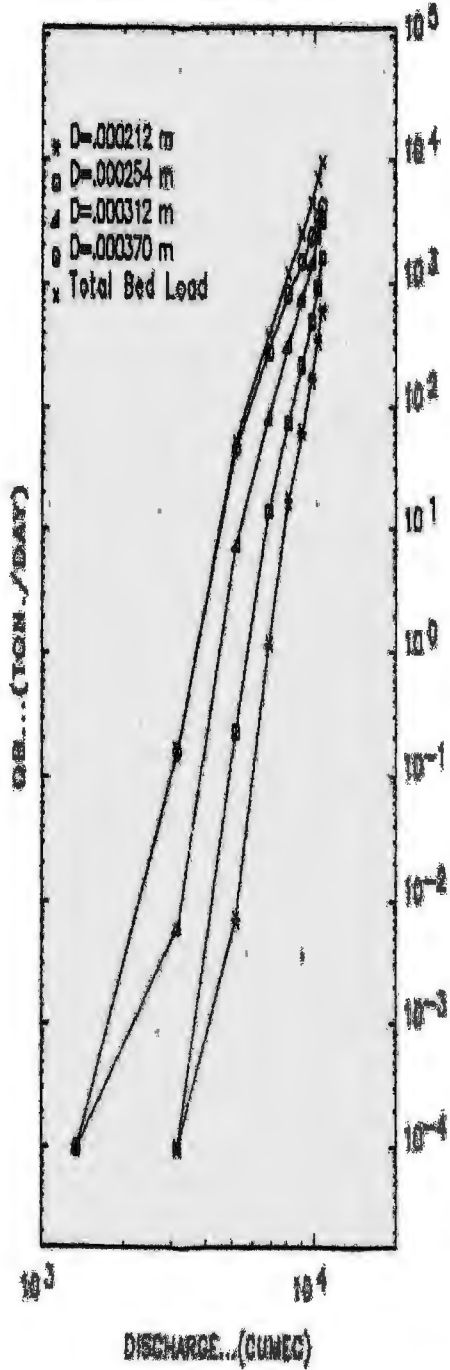


FIGURE A.1 c

BED LOAD SEDIMENT TRANSPORT

Section... Oiseaux Nord 2



SUSPENDED LOAD SEDIMENT TRANSPORT

Section... Oiseaux Nord 2

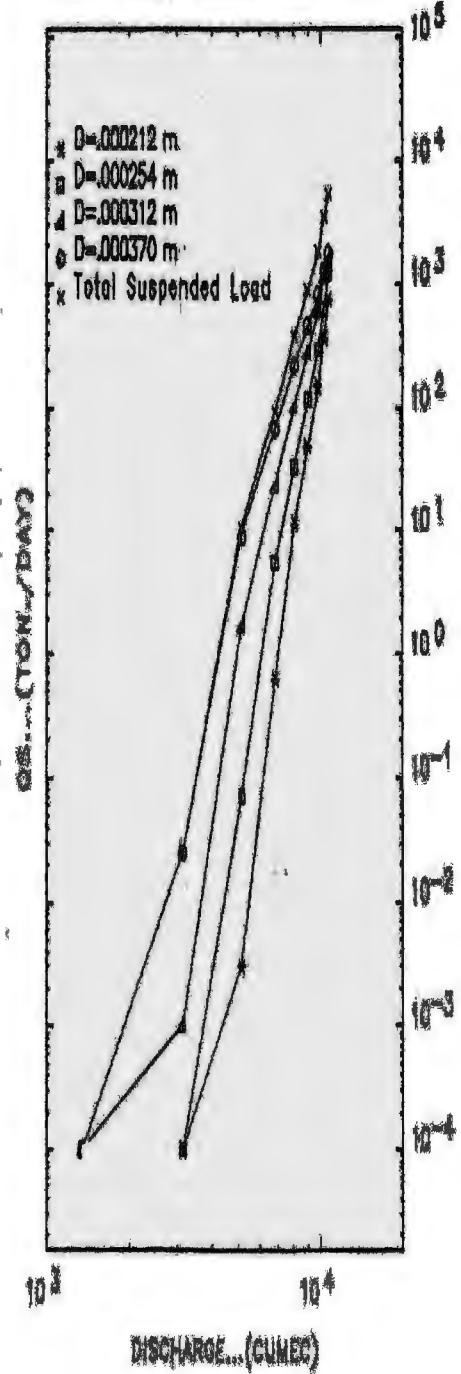
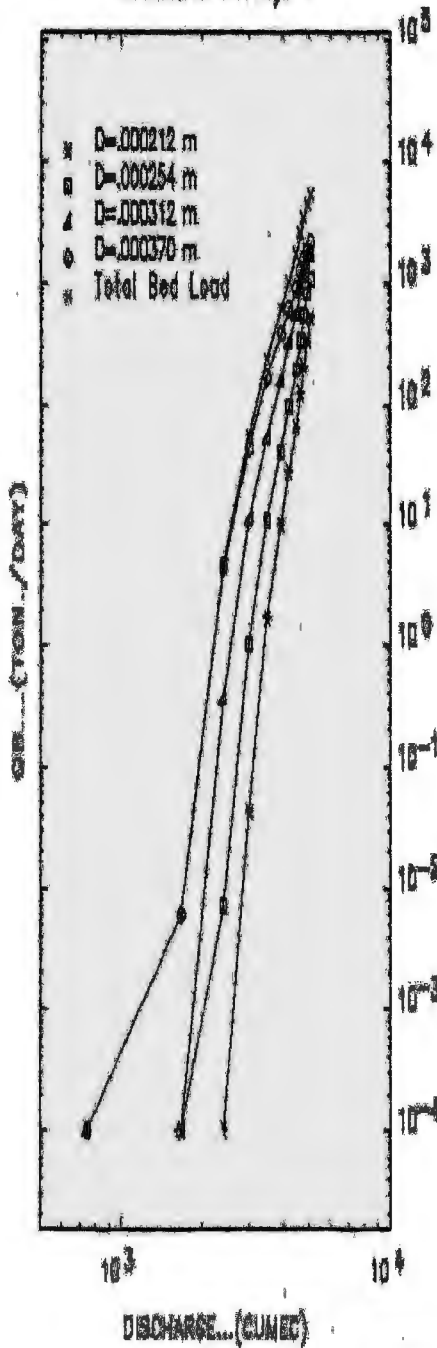


FIGURE 6.3 d



BED LOAD SEDIMENT TRANSPORT

Section... Barraje 1



SUSPENDED LOAD SEDIMENT TRANSPORT

Section... Barraje 1

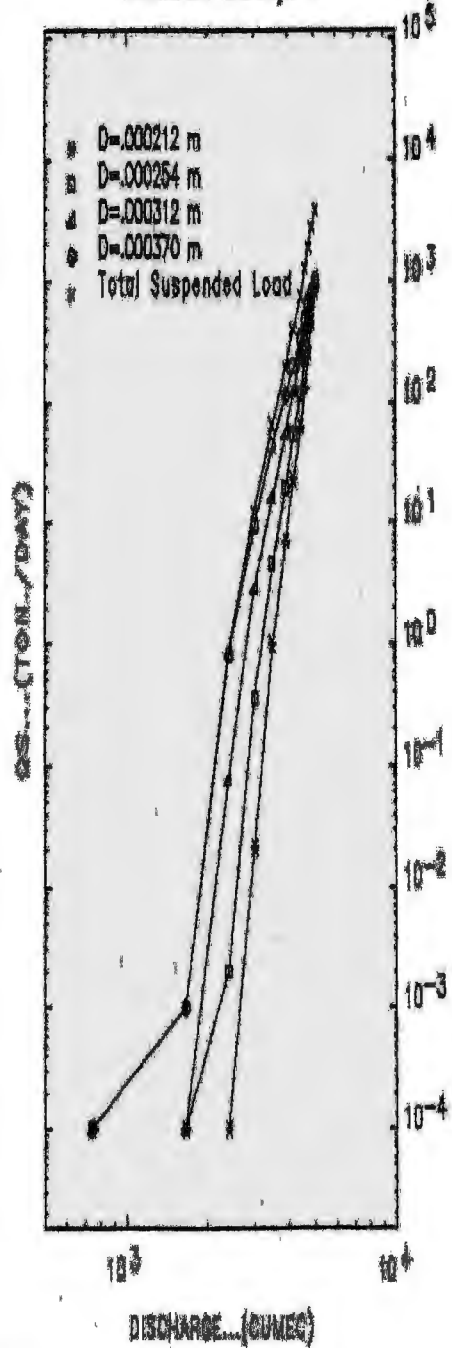
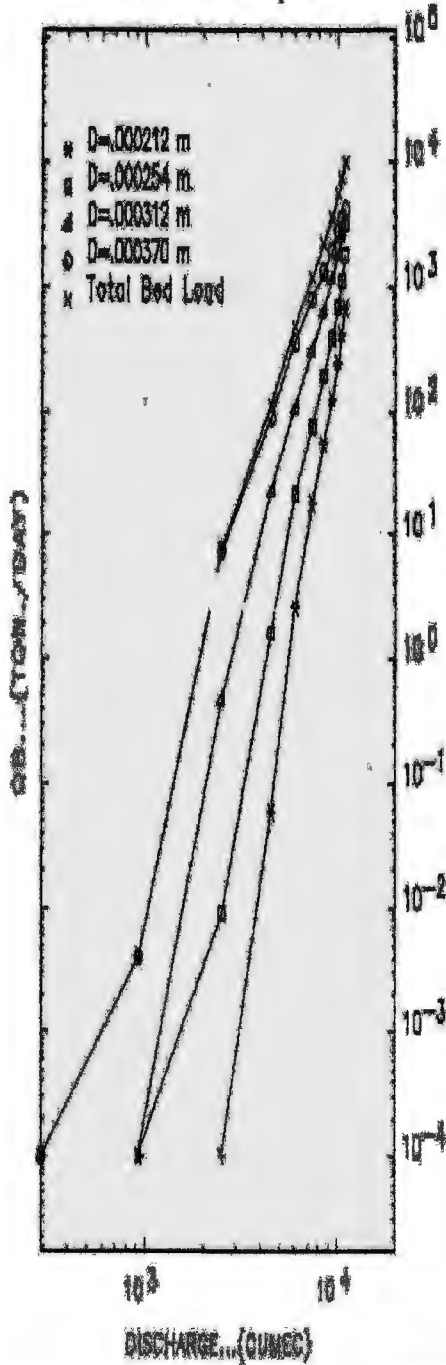


FIGURE 2.1 e

BED LOAD SEDIMENT TRANSPORT

Station... Barraje 2



SUSPENDED LOAD SEDIMENT TRANSPORT

Station... Barraje 2

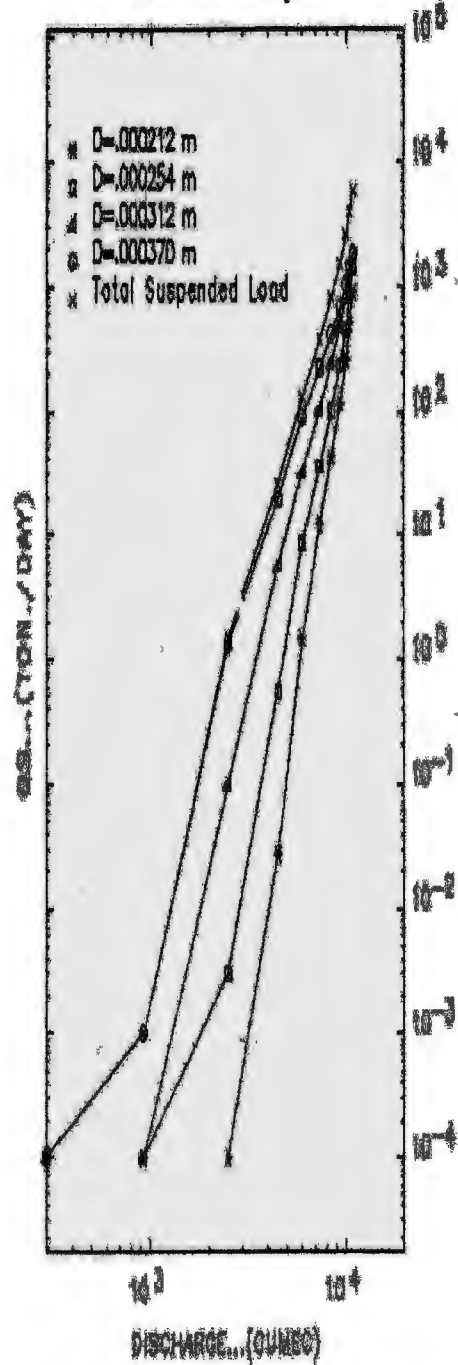
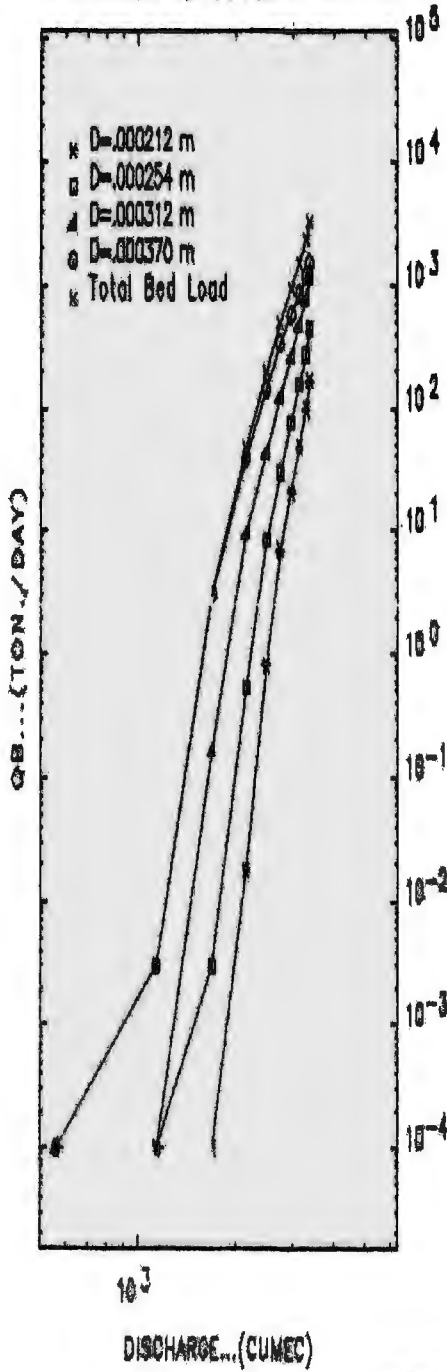


FIGURE 6.14

BED LOAD SEDIMENT TRANSPORT

Section... Mateba Amont Sud 1



SUSPENDED LOAD SEDIMENT TRANSPORT

Section... Mateba Amont Su 1

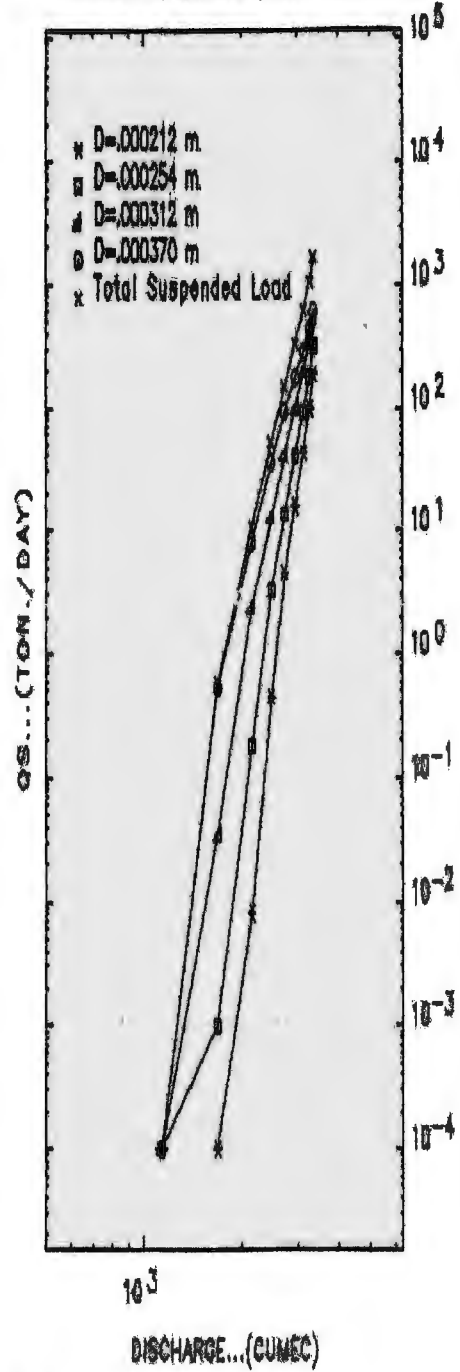
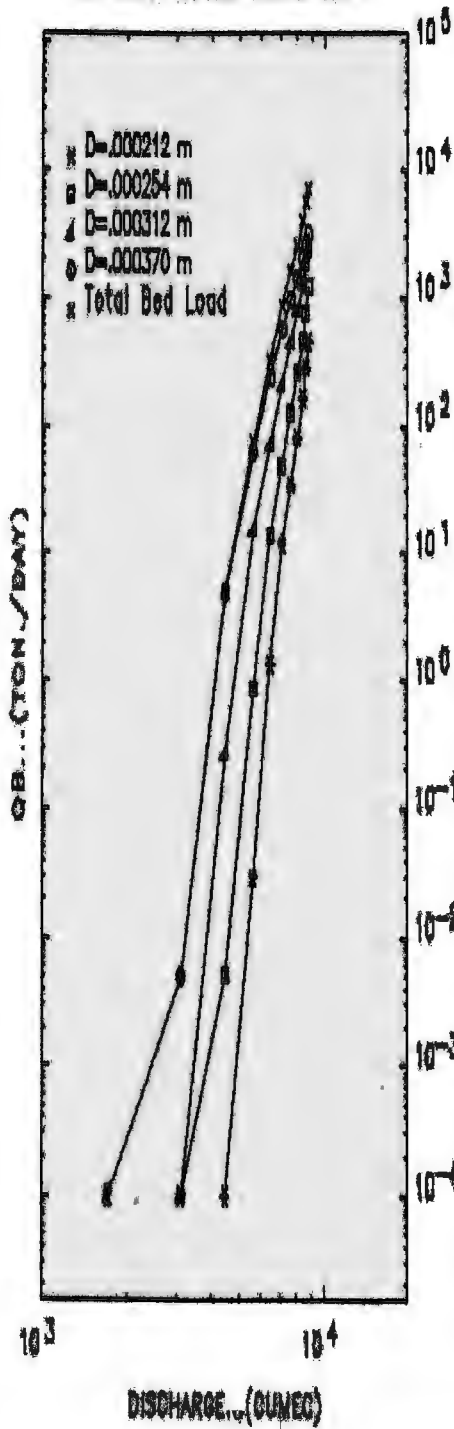


FIGURE 6.1 g

BED LOAD SEDIMENT TRANSPORT

Section... Mateba Ament Sud 2



SUSPENDED LOAD SEDIMENT TRANSPORT

Section... Mateba Ament Sud 2

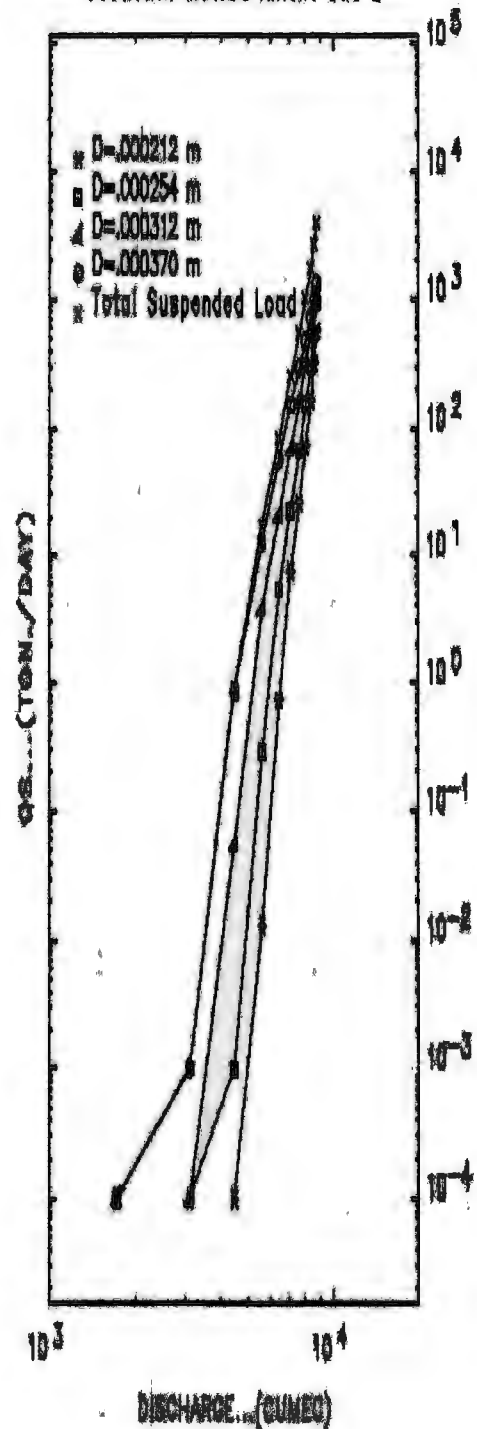
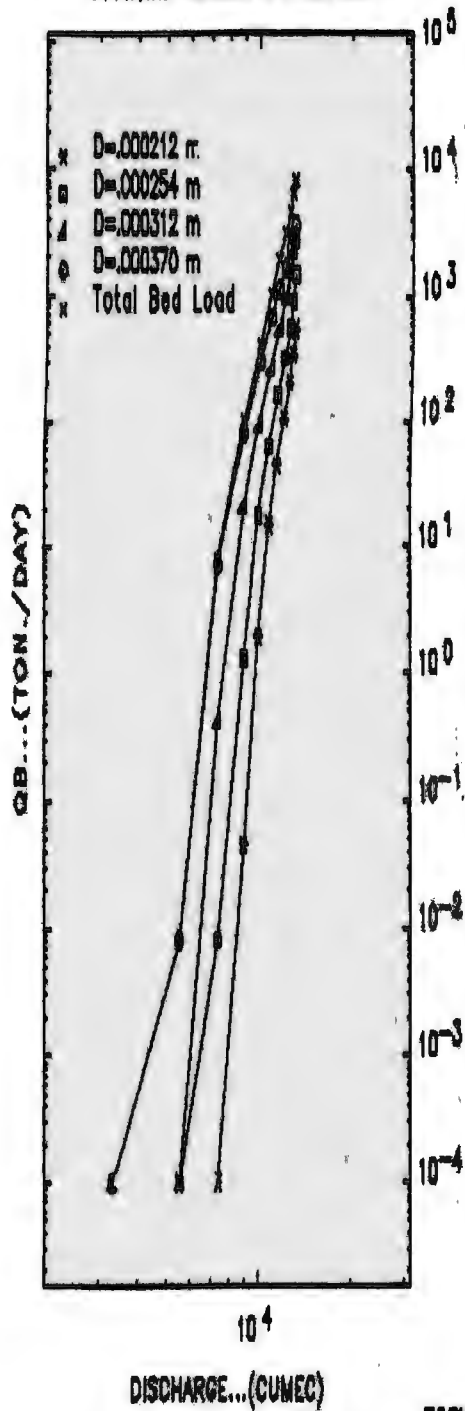


FIGURE 6.1 h

**BED LOAD SEDIMENT TRANSPORT**  
 Section... Mateba Sud Mandud



**SUSPENDED LOAD SEDIMENT TRANSPORT**  
 Section... Mateba Sud Mandud

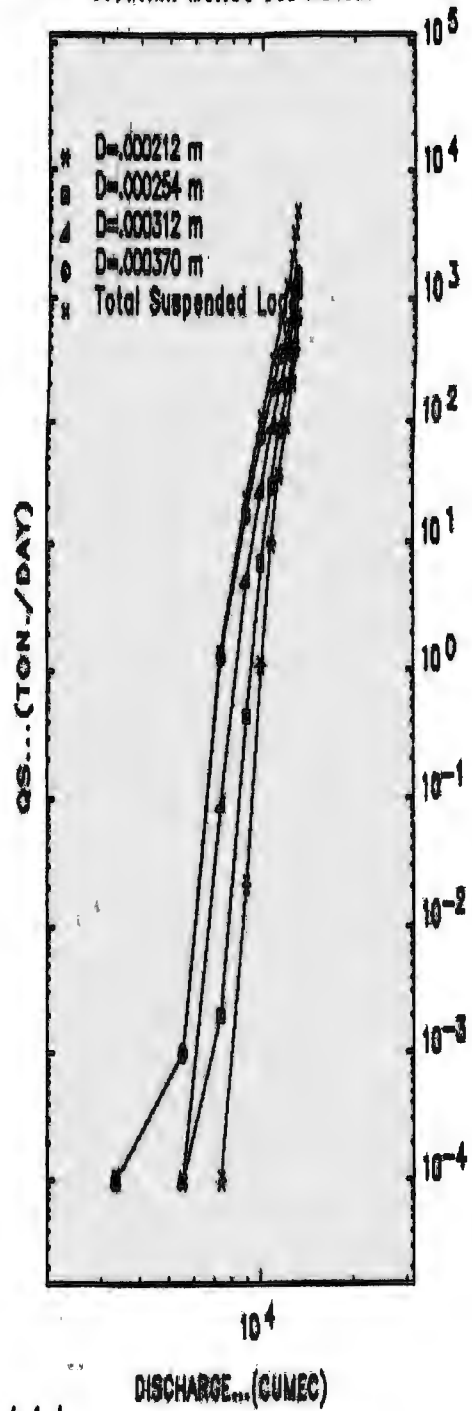
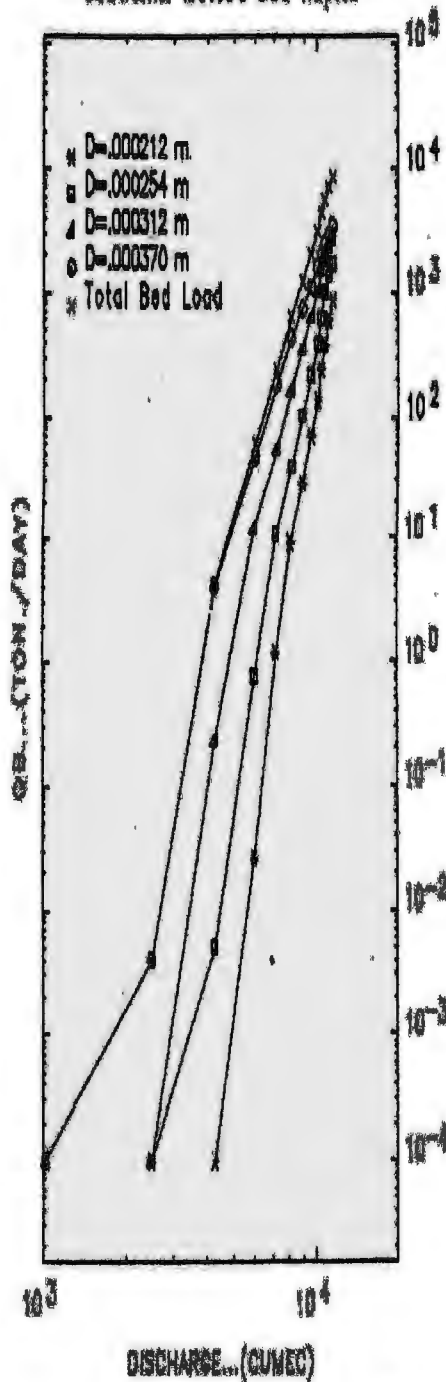


FIGURE 6.1 1

**BED LOAD SEDIMENT TRANSPORT**  
 Section... Motoba Sud Kapita



**SUSPENDED LOAD SEDIMENT TRANSPORT**  
 Section... Motoba Sud Kapita

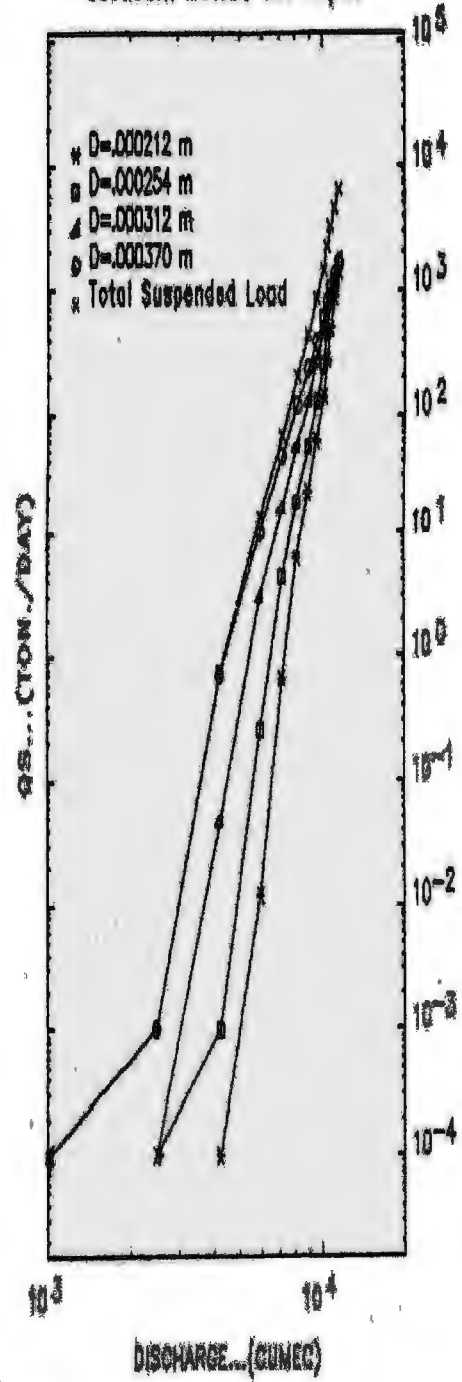
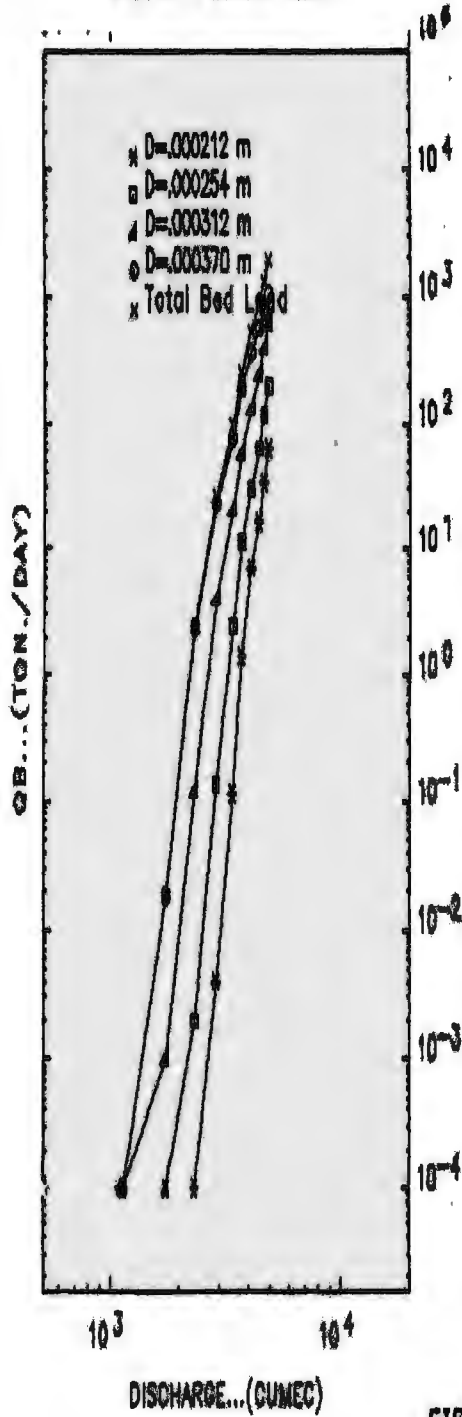


FIGURE 6.1 J

BED LOAD SEDIMENT TRANSPORT

Section... Mateba Ament



SUSPENDED LOAD SEDIMENT TRANSPORT

Section... Mateba Ament

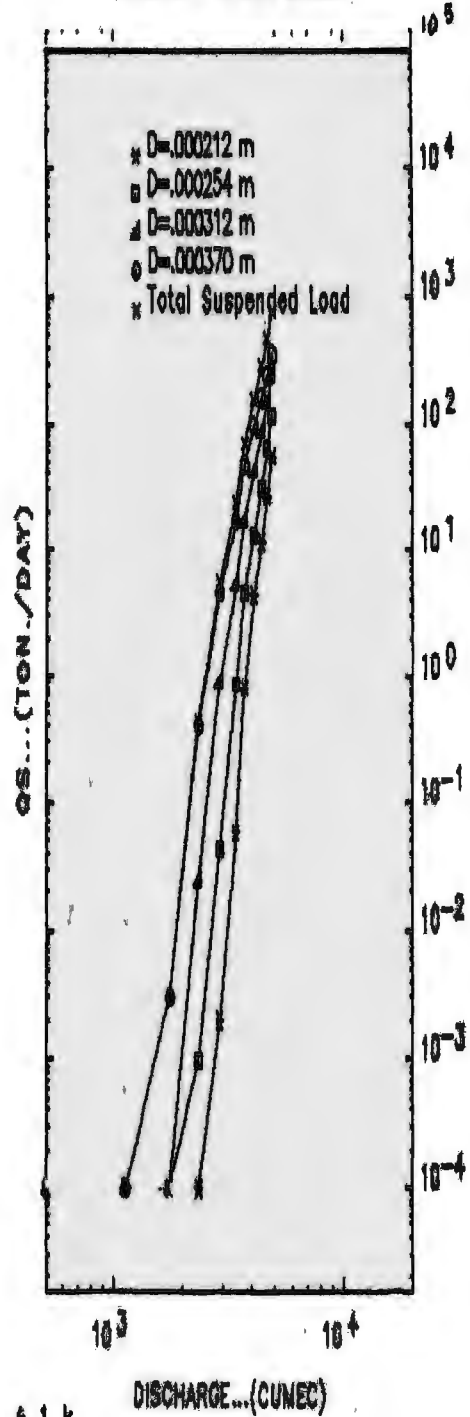
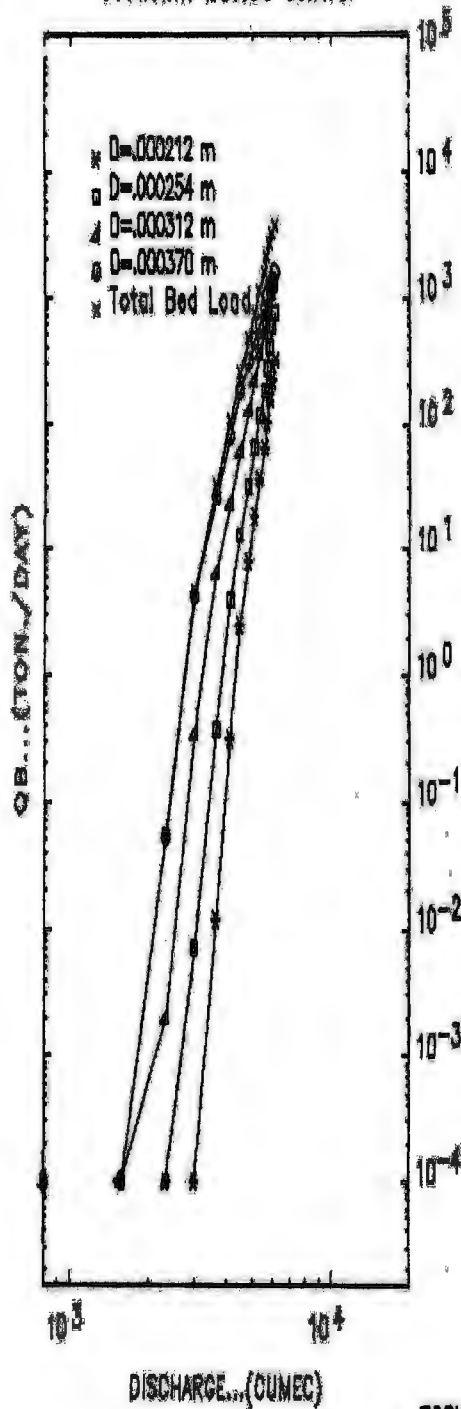


FIGURE 4.1 k

BED LOAD SEDIMENT TRANSPORT

Section... Natoba Central



SUSPENDED LOAD SEDIMENT TRANSPORT

Section... Natoba Central

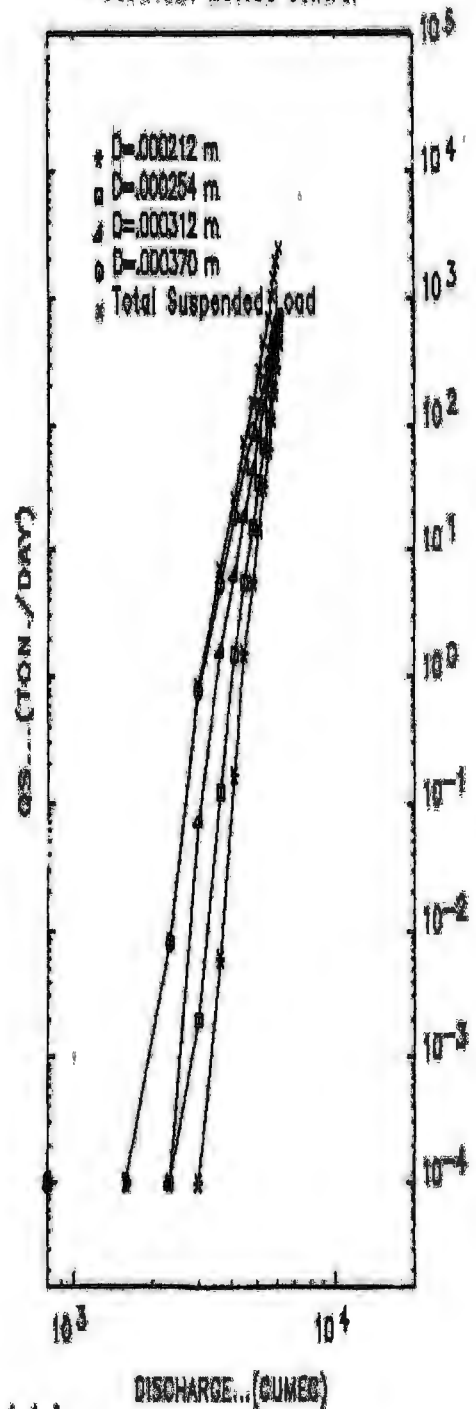
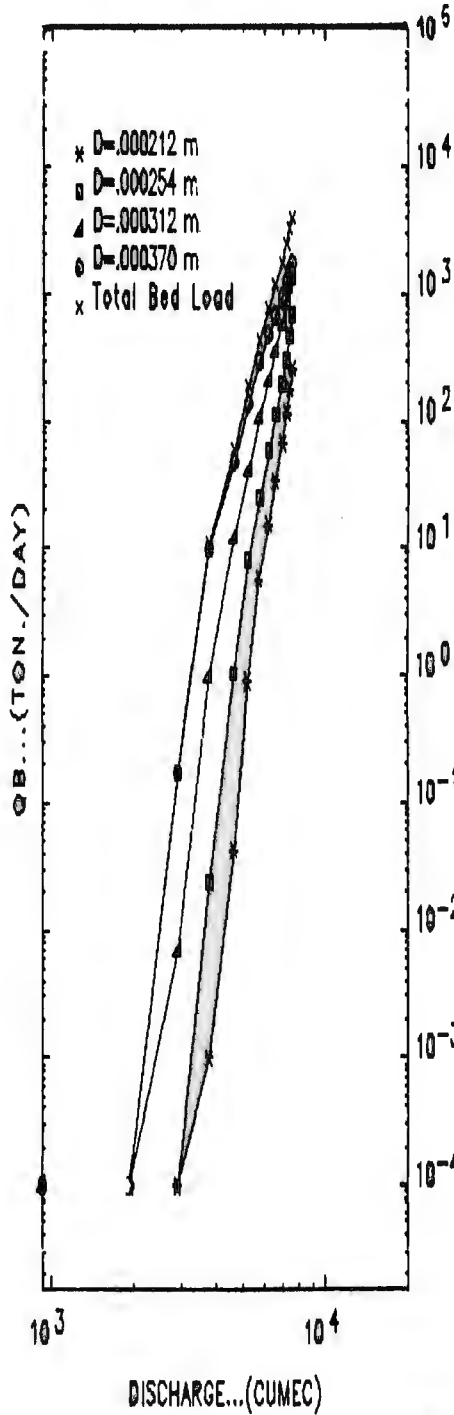


FIGURE 6.1.1



BED LOAD SEDIMENT TRANSPORT

Section... Mateba Aval



SUSPENDED LOAD SEDIMENT TRANSPORT

Section... Mateba Aval

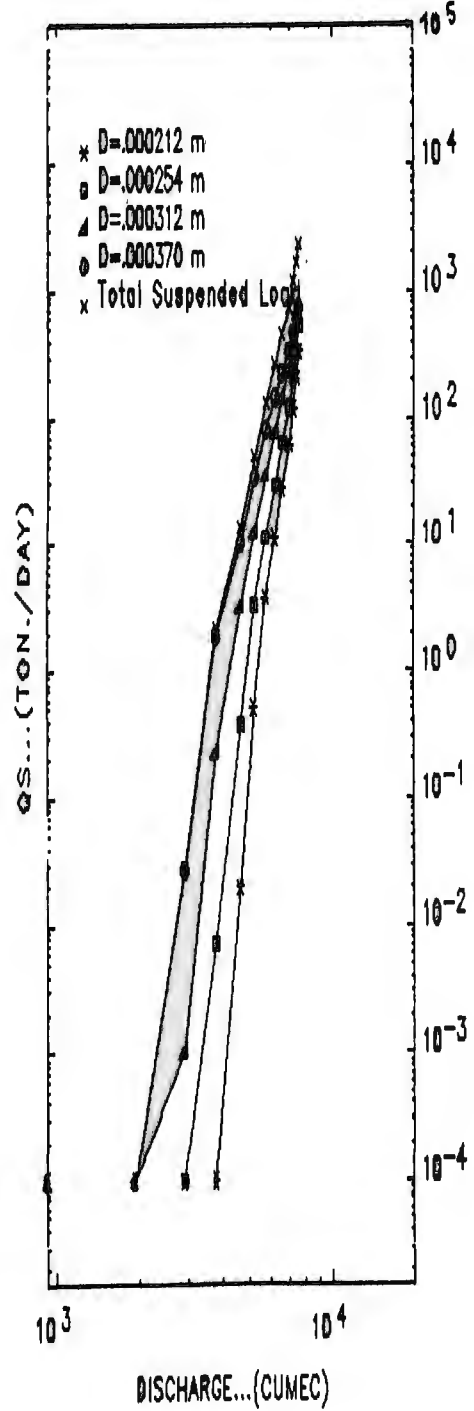


FIGURE 6.1 m

FIGURE 6.2 a VELOCITY VARIATION ALONG THE CROSS SECT.  
Section... Mateba Ament

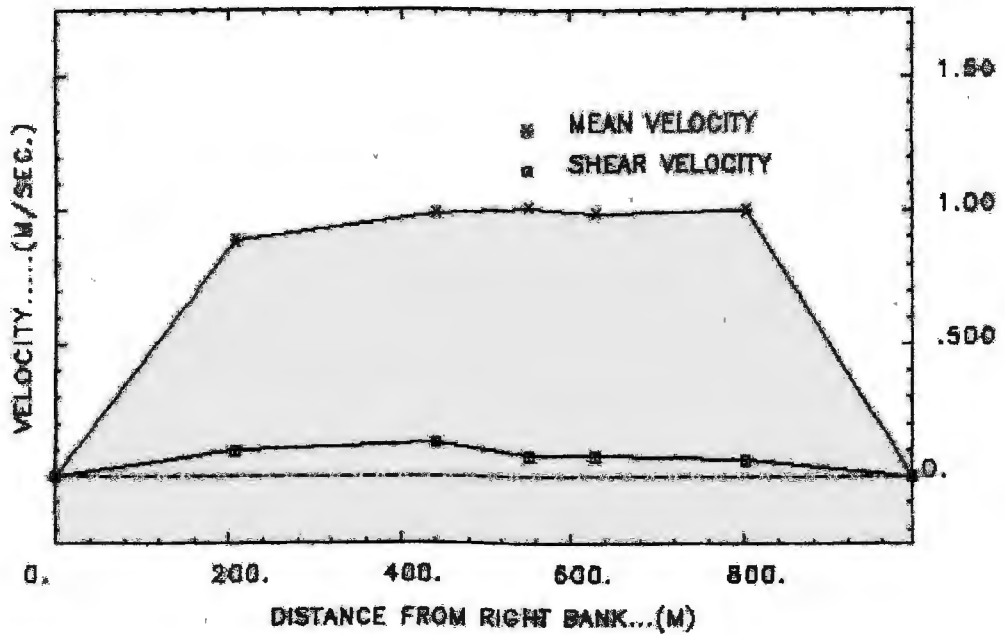
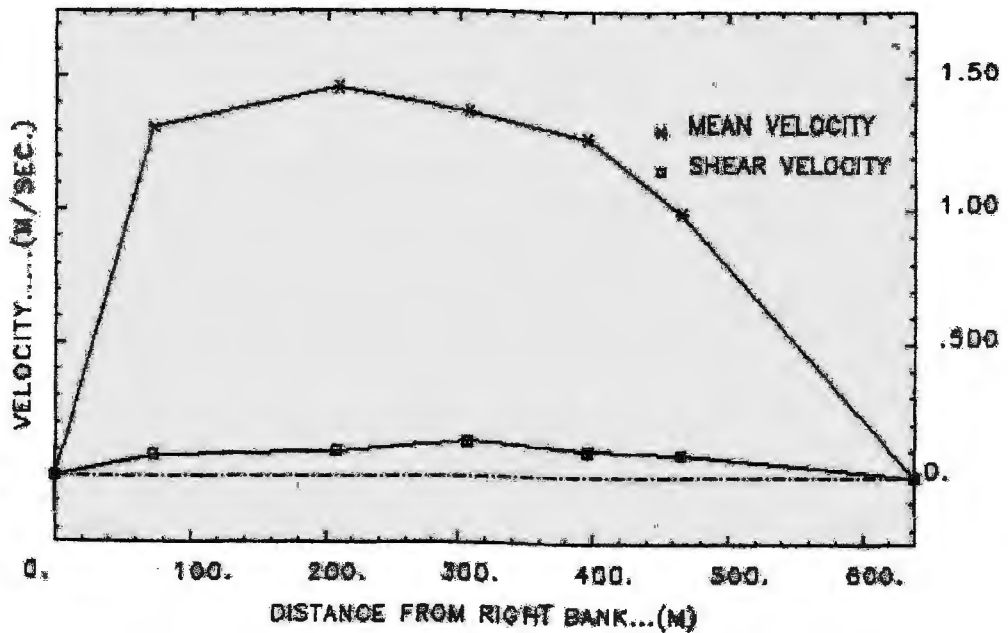


FIGURE 6.2 b VELOCITY VARIATION ALONG THE CROSS SECT.  
Section... Mateba Central



# VELOCITY VARIATION ALONG THE CROSS SECT.

FIGURE 6.2 c

Section... Mateba Ayal

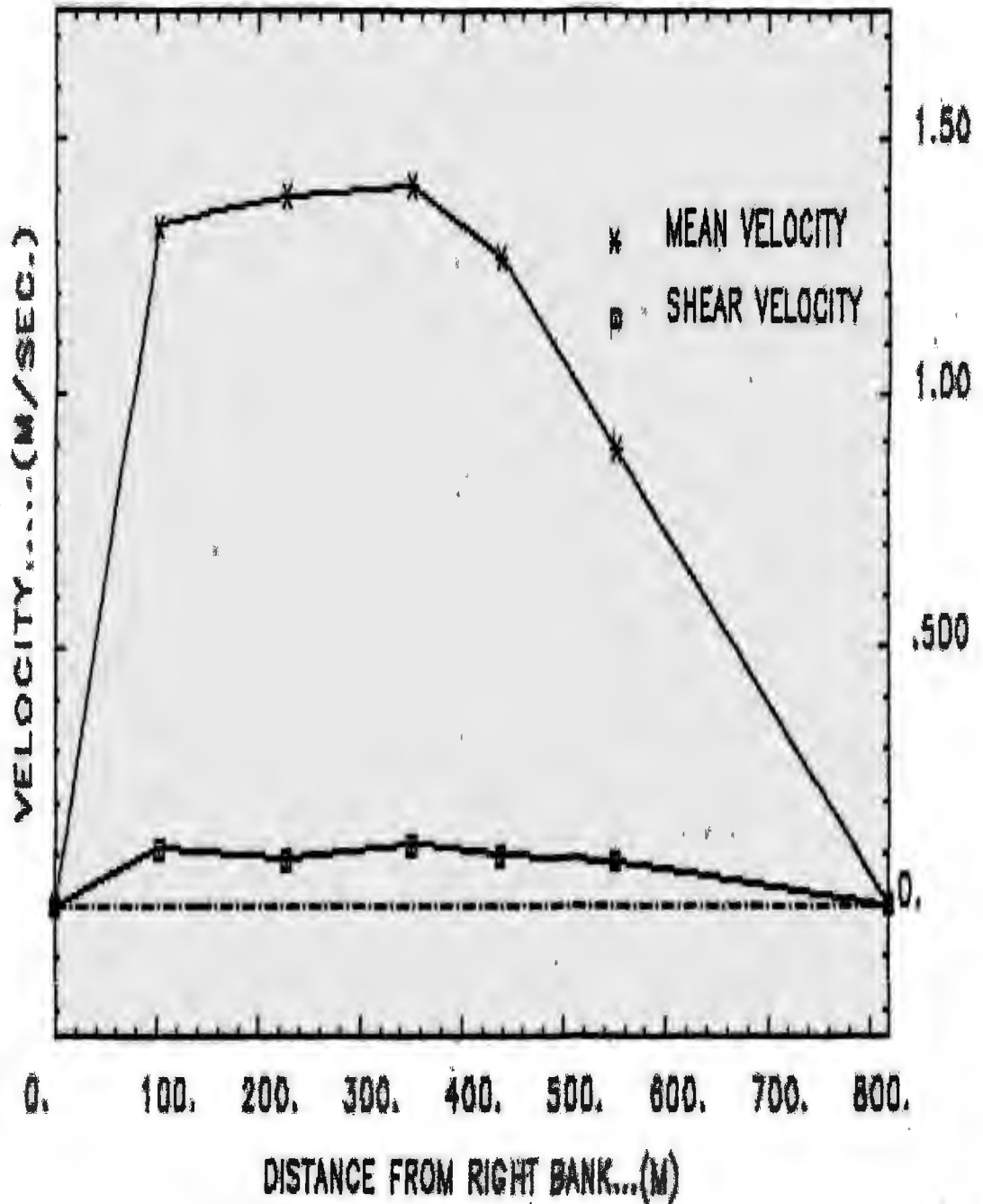


FIGURE 6.3 a SEDIMENT TRANSPORT ALONG THE CROSS SECT.  
Section... Mateba Ament

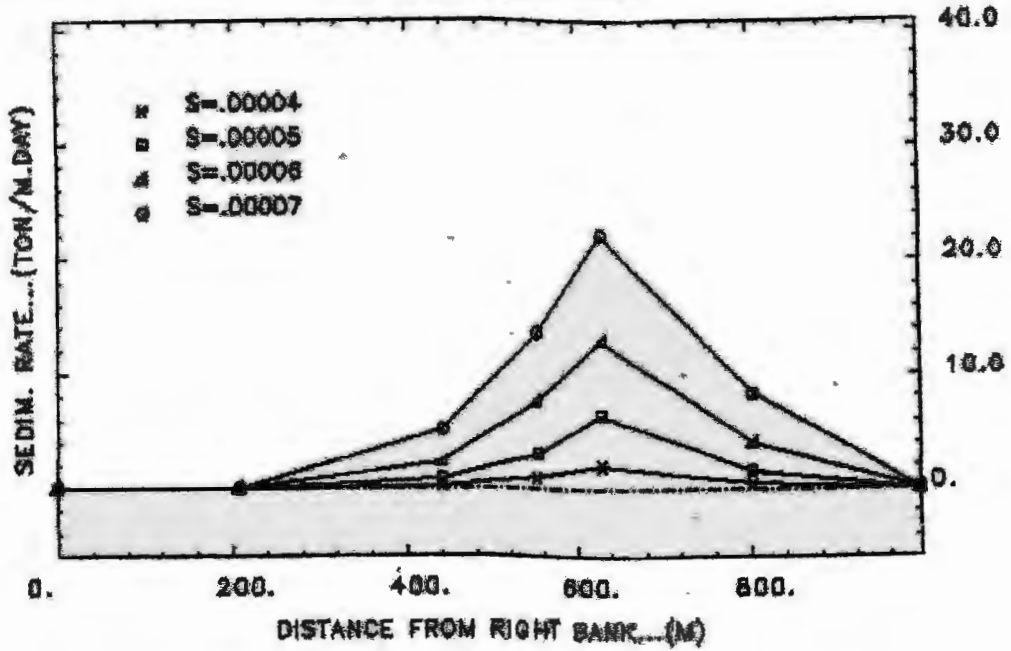


FIGURE 6.3 b SEDIMENT TRANSPORT ALONG THE CROSS SECT.  
Section... Mateba Central

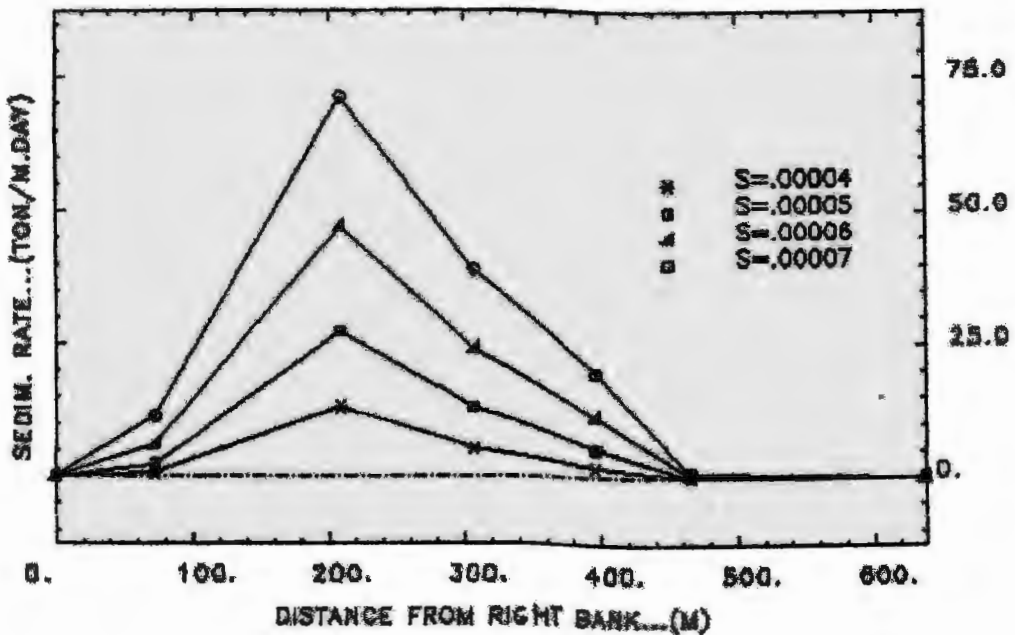


FIGURE 6.3 c SEDIMENT TRANSPORT ALONG THE CROSS SECT,  
 Section... Mateba Aval

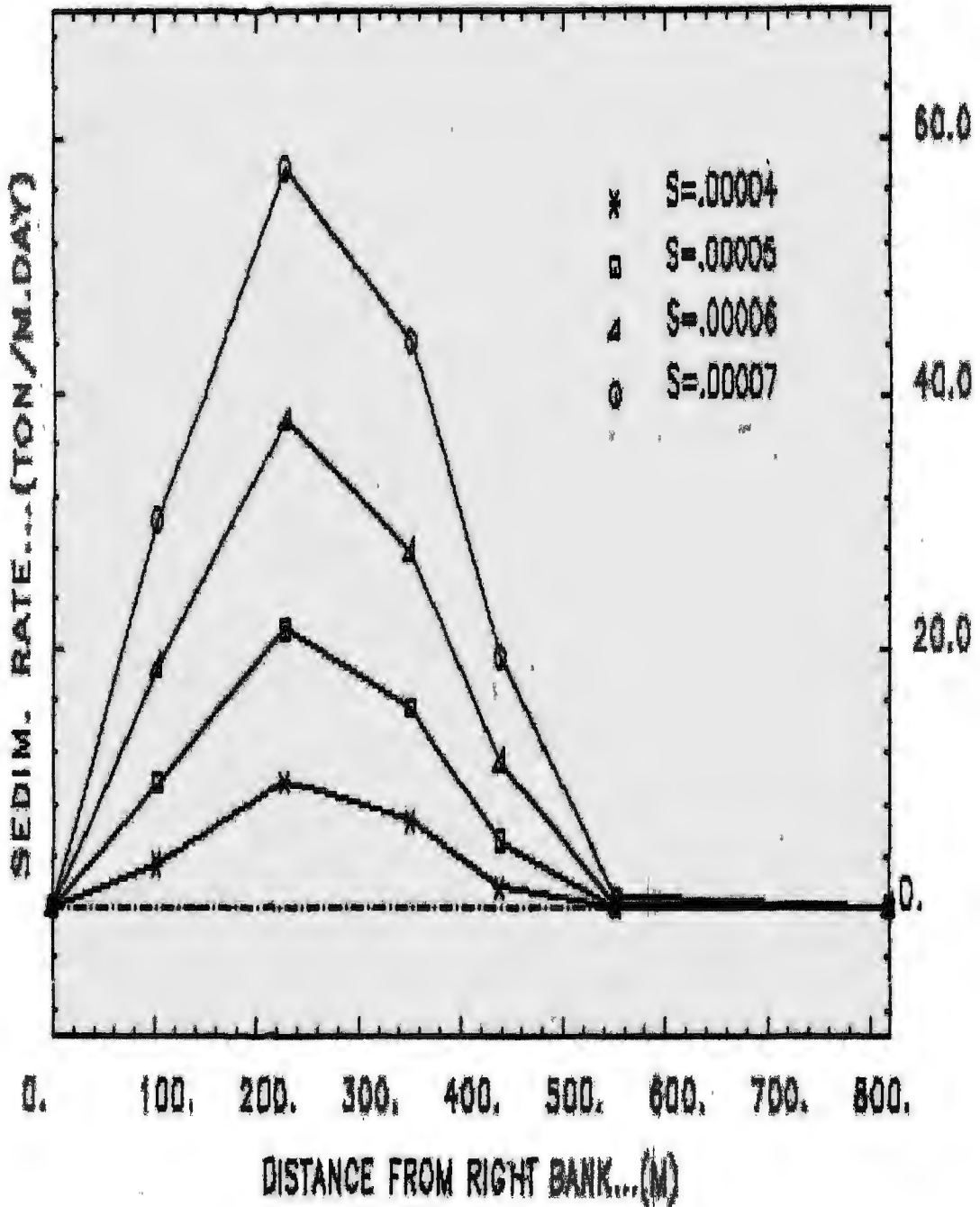


FIGURE 6.4 a SEDIMENT TRANSPORT ALONG THE CROSS SECTI  
Section... Mateba Ament

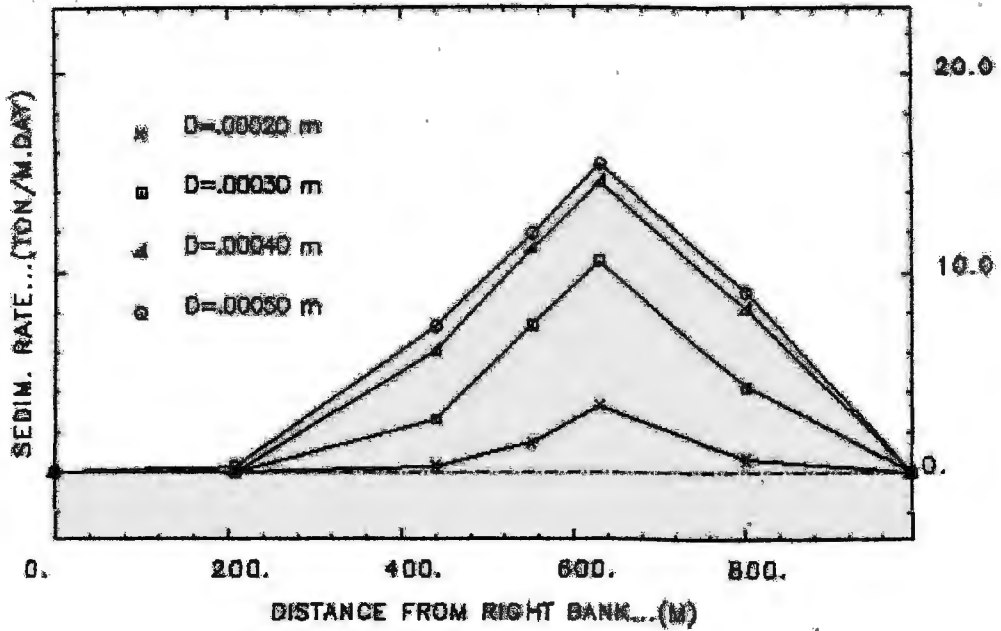


FIGURE 6.4 b SEDIMENT TRANSPORT ALONG THE CROSS SECT.  
Section... Mateba Central

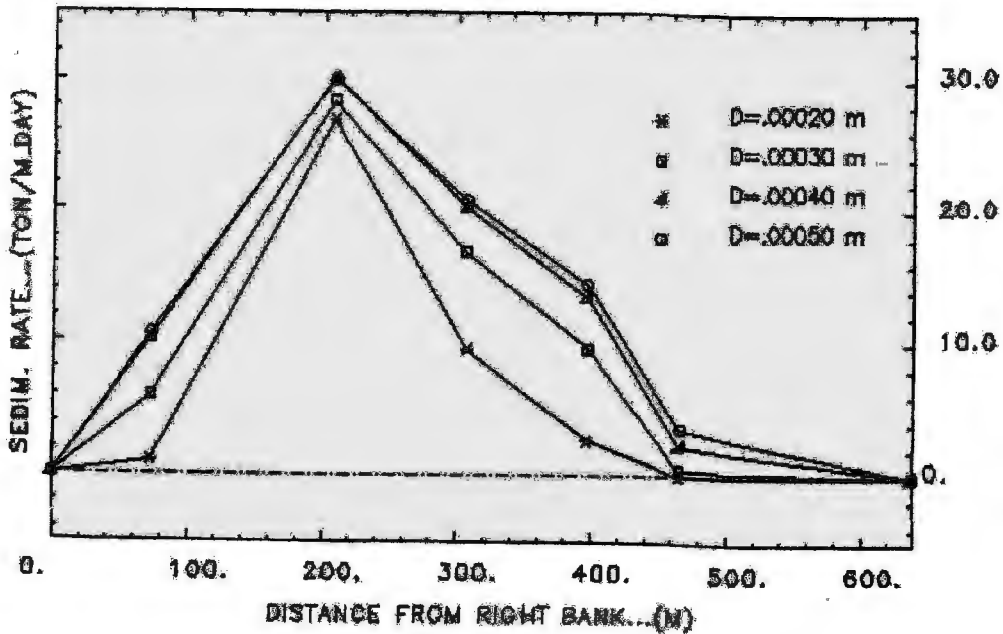


FIGURE 6.4 c SEDIMENT TRANSPORT ALONG THE CROSS SECT.

Section... Mataba Aval

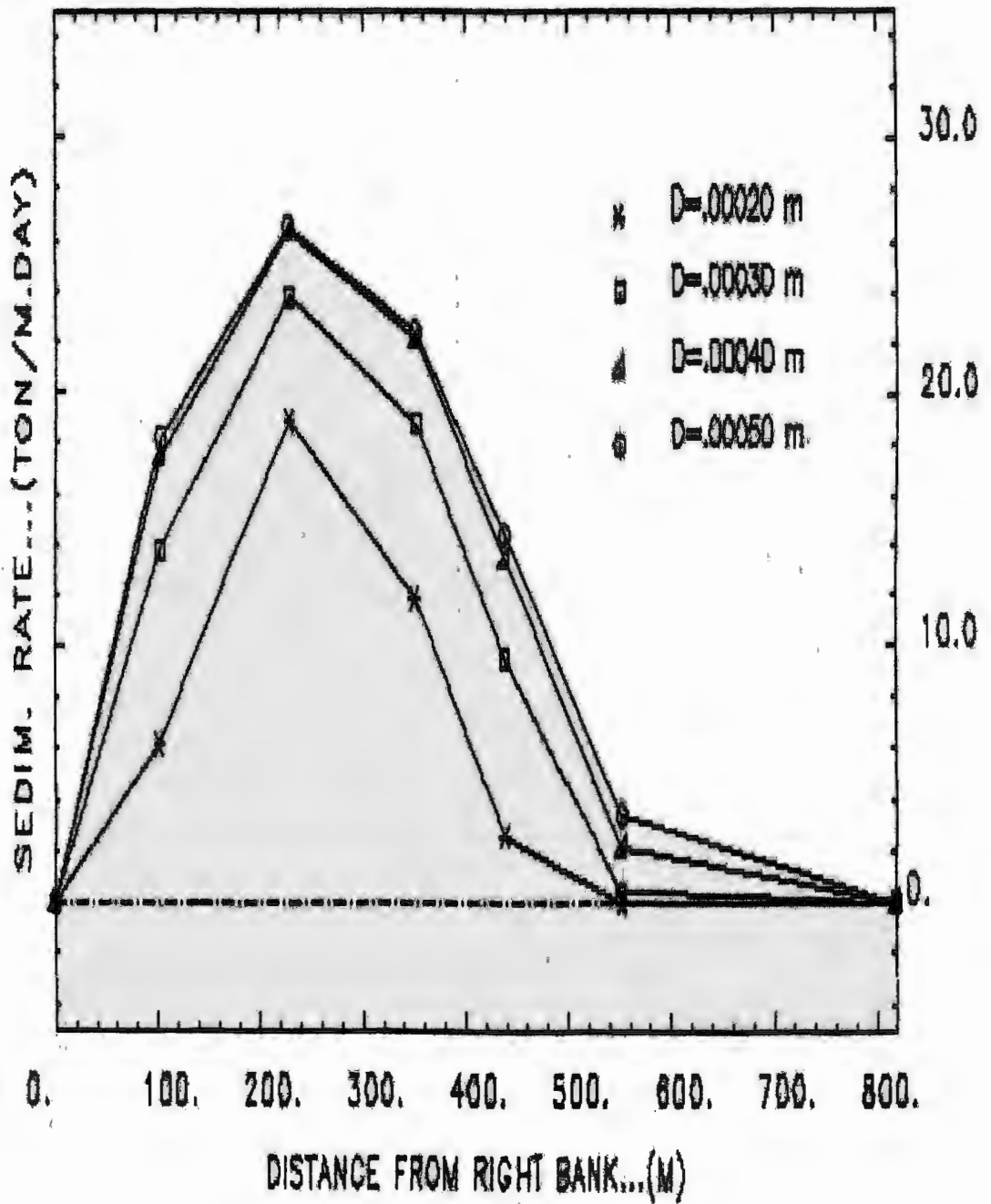


FIGURE 6.5a SEDIMENT TRANSPORT ALONG THE CROSS SECT.  
Section... Mateba Ament

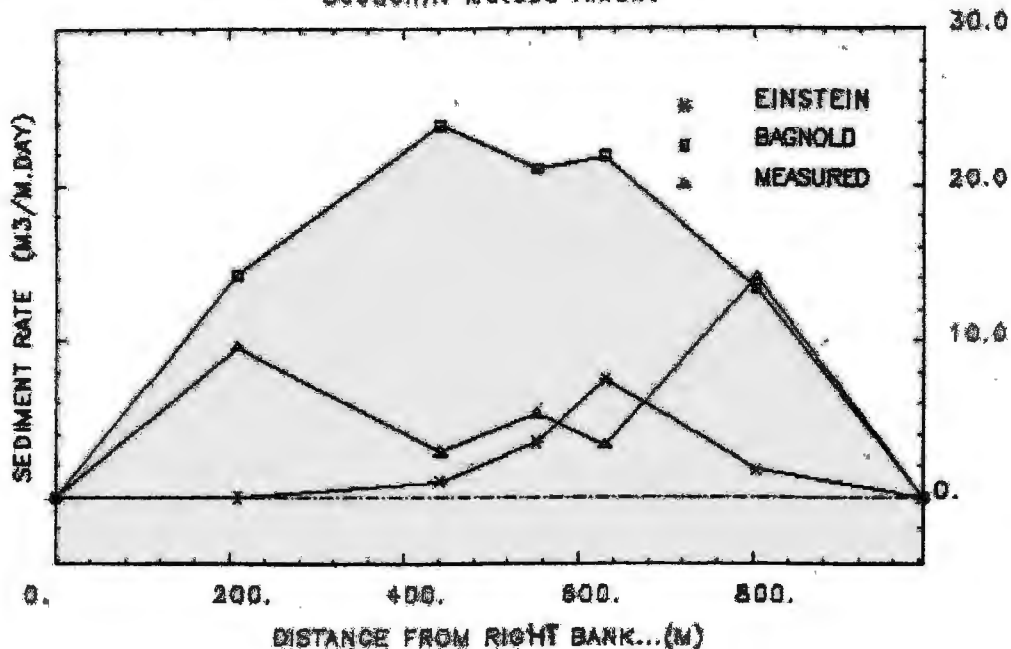


FIGURE SEDIMENT TRANSPORT ALONG THE CROSS SECT.  
Section... Mateba Central

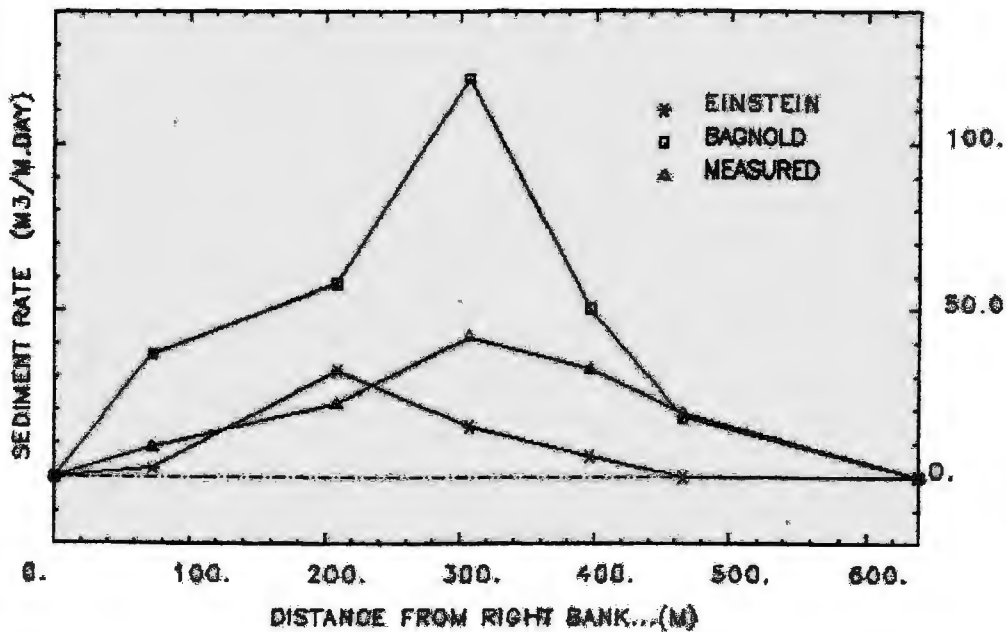




FIGURE 6.5c SEDIMENT TRANSPORT ALONG THE CROSS SECT.  
Section... Mateba Aval

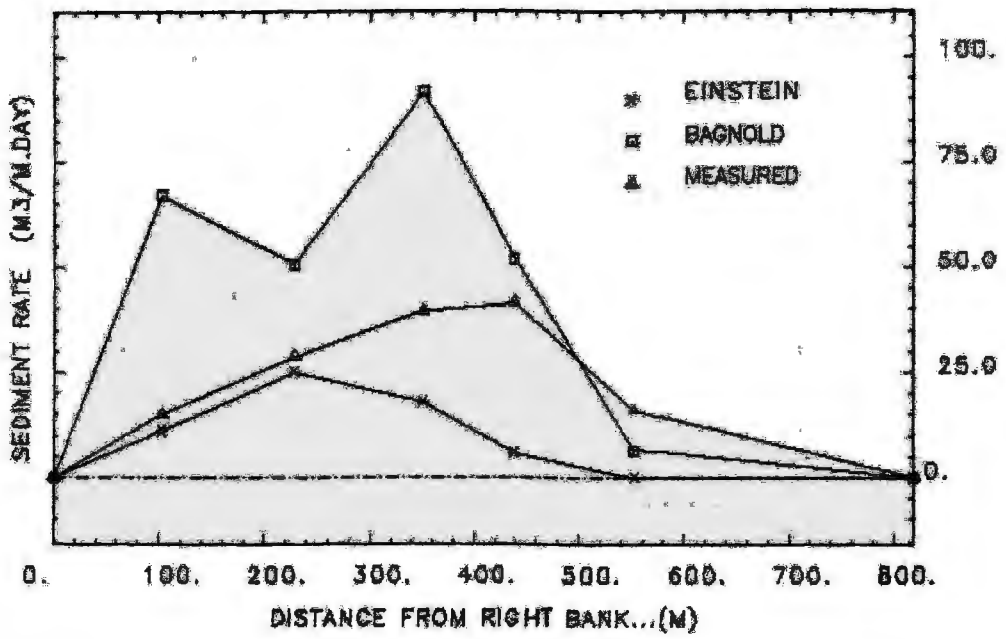
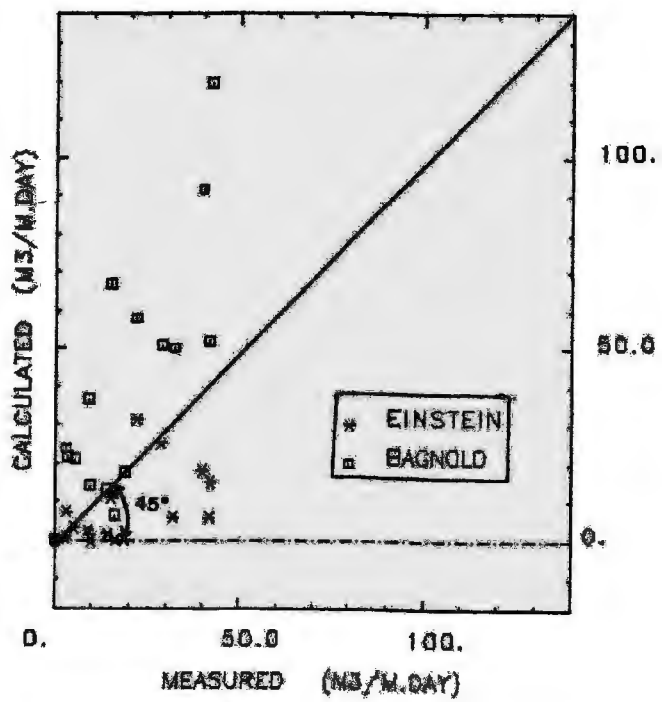


FIGURE 6.6 SEDIMENT TRANSPORT ALONG THE CROSS SECT.  
Mateba STATION 1981



## 7. DISCUSSION OF RESULTS

a.- The accuracy of sediment transport calculation depends upon the degree of precision of the flume experimental graphs that were developed by Einstein. This is due to the inherent errors and the considerations that were adopted during the process of his experiments.

b.- The following are comments concerning the graphs:

- The curve  $x$  against  $Kg/g$  in Figure 3.2 was derived from NIKURADSE'S experiments, which used sand grains glued to steel pipes as roughness. The curve has always been found to describe reliably the roughness of plain sand beds. Some deviations may be expected if the range of grain sizes in the deposit is very large. The assumption that  $Ks = D65$  may not be very reliable if the sediment contains appreciable amount of silt and clay.

- The curve  $u/u_{*c}$  against  $\psi'$  is less reliable than  $x$  curve. Basing on existing river measurements, it attempts to describe the effect of the irregularities of the natural stream channel on channel roughness. This curve seems, however, to describe rather closely the behavior of natural channels not constricted by artificial banks, vegetation or other obstructions.

- The curve of  $\bar{y}$  against  $D/X$  has been derived entirely from flume experiments with graded sediments. No test was made of ungraded mixtures, such as those found in mountain rivers near the upper end of alluvial stream systems where slopes steep.

- The curve of  $Y$  against  $Ks/g$  seems to be well defined

by flume experiments. The scattering of the points is much smaller than that of the  $\psi$  curve.

- The  $\phi_s - \psi_s$  curve is entirely theoretical, which represents equation (3.29). The three constants  $A_s$ ,  $B_s$  and  $\tau_c$  are obtained from reliable experiments.

- Figure 3.8 and Figure 3.9 give respectively integrals  $I_1$  and  $I_2$  in terms of the exponent  $x$  and the limit  $A_s$ .  $I_1$  is always positive and  $I_2$  is always negative. For values of  $x > 5$  the expression  $\text{Pr. } I_1 + I_2$  usually becomes smaller than 0.2. These graphs are solutions of equation 3.47 according to the SIMPSON numerical integration method.

c.- The possible errors in the measurement of velocity, particle size, water depth and geometry of cross section which are normally greater than 40 percent were not taken into account in the calculation of the sediment transport.

d.- From the results obtained by application of Einstein model in the traditional procedure (Table 6.1) it can be said that the bed load is greater than the suspended-load for all cross sections. From Figures 6.1a to 6.1 n we can see that the bed-load and suspended-load vary for specific particle size. The sediment transport is higher for bigger particle size. This phenomena is clear in bed-load, because the intensity of transport,  $\phi_s$  (Figure 3.7) decrease when intensity of shear,  $\psi_s$ , decreases. This is reflected in the  $\psi$  curve (hiding factor), Figure 3.8, which illustrates the fact that the small bed particles always tend to hide behind larger grains or in the laminar sublayer. Rather small

amounts of similar particles in more prominent positions of the bed could support a very large additional transport of these particles without increasing effectively in their overall concentrations  $\bar{c}_p$  in the bed as a whole. This might explain the existence of wash load on the same bed which sustains bed-load function.

e.- The Figures 6.1a to 6.1d show the plots of the bed-load and suspended-load of all grain sizes versus discharge. Each curve corresponds therefore, to an individual point of a cumulative curve. The group of curves mentioned above give the full description of the bed-load function.

f.- Each cross section has a complex geometry, hence, it was analysed separately. In those cases where the cross sections are split into two it is noticed that each part has totally different characteristics such as Chezy coefficient, Manning coefficient and slope (Table 4.1) and these justify separate treatment.

g.- The factors that influence the sediment transport rate according to their importance are mentioned below:

Slope, particle size, geometric characteristic of cross section, and discharge. These are interrelated and in the case of a rectangular channel it gives reliable results.

h.- The shear velocity,  $u_{*c}$ , in the two-dimensional approach was calculated from the vertical velocity distribution. The value of  $u_{*c}$  depends solely on the gradient of the regression line. The measured value of one point, either the velocity or depth would result in a "shift" of the regression line which

in turn would affect the gradient and  $u_g$ .

i.- In the two-dimensional approach the effect of the bedform is small, because the panel corresponding to each vertical is very close to the rectangular shape. This is the reason why this approach could represent in a better way the sediment transport calculation.

j.- The figures 6.5a to 6.5c show the plots of measured sediment transport and calculated sediment transport according to the Bagnold and Einstein models. In general the results obtained from Bagnold model [2] overestimates the measured values. Whereas the results from Einstein model underestimates the measured values. The overestimation of the Bagnold model can be due to the fact that Bagnold had made to arrive at his equation the assumption of fully turbulent flow; the bed-load and suspended-load efficiency factors were experimentally found and which may not be accurate in the natural stream. In the case of Einstein model the laminar sublayer is considered and where the small bed particles always tend to hide behind larger grains.

## B. CONCLUSION

Since Einstein method is basically derived from bed-load measurements, it is not reliable to apply this when a large percentage of the sediment is in suspension. Einstein method considers the dependence of suspended sediment on turbulent intensity.

The results indicate that the sediment discharge increases when water discharge increases.

The total computed sediment rate at MATEBA STATION, varies between 13,169.8 Ton/day (cross section: Amont Dizeaux) and 15,247.681 Ton/day (cross section: Mateba Sud Kapita) for the year 1978 . For the year 1981 the sediment discharge varies between 2,675.862 Ton/day and 6,489.687 Ton/day .

The influence of bed forms is more important than the roughness due to particles in the case of the traditional procedure. In the two-dimensional approach this effect may be very small.

The accuracy of field measurement data and the effect of bed forms on the parameters of flow could not be evaluated properly.

From Figure 4.3 it is clear that the sediment rate varies with energy slope. The sediment transport increases exponentially as the energy slope is increased.

From Figure 4.4 we can conclude that the sediment rate also increases with the largeness of particle sizes. However the increase in sediment transport is less and less significant as the particle sizes increase further.

From figures 6.5 and 6.6 it is clear that the sediment transport calculated by the application of Einstein model underestimates the measured values.

Finally the sediment discharge in streams depends on many variables whose effects are complicated and interrelated.

## REFERENCES

1. AMARAKOON J. "An Approach to two Dimensional Sediment Transport a case-study: The Zaire River". Thesis in partial fulfillment of the requirements for the degree of Master of Science in Hydrology. Vrije Universiteit of Brussels (IUFHY). July 1986, pp. 38-39.
2. DIVER M. "Recente Theorien Over de Stromingweerstand in Alluviale Rivieren en hun Toepassing op de Zaire Stroom Thesis presented for the first degree in the civil engineering at V.U.B. May 1987.
3. EINSTEIN, H. A. "The Bed-Load Function for Sediment Transportation in Open Channel Flows", Technical Bulletin No. 1826. U.S Dept. of Agriculture, Soil Conservation Service. September 1950.
4. EINSTEIN, H. A. and BARBAROSA, N. L. "River channel Roughness", Transactions, ASCE, vol. 117, 1952, pp. 1121 - 1146.
5. GRAF, W. "Hydraulics of Sediment Transport". Mc.Graw-Hill Book Company, 1971, pp. 139 - 227.
6. SIMONS D. and SENTRURK F. "Sediment Transport Technology", Water Resources Publications, Fort Collins Colorado 80522, USA. 1977, pp. 147 - 200 , 497 - 615.
7. VANDONI VITO A. "River Dynamics", California Institute Technology, volume 15. 1975.
8. YALIN, M. S. "Mechanics of Sediment Transport". Queens University, Ontario, second edition. 1977, pp. 75 - 207



## LIST OF FIGURES

- Figure 2.1 Shield's diagram
- Figure 3.1 Assumed velocity distribution near the laminar sublayer along a hydraulically smooth wall.
- Figure 3.2 Correction factor "x" in the logarithmic friction formula in terms of  $Ks/\delta$
- Figure 3.3 Friction  $u_{*c}^2$  due to channel irregularities.
- Figure 3.4 Settling velocity for various sizes of quartz grains according to Rubey.
- Figure 3.5 Pressure reduction in the sublayer.
- Figure 3.6 Pressure correction in the transition to a smooth bed.
- Figure 3.7  $\phi_* - \psi_*$  curve.
- Figure 3.8 Function  $I_1$  in terms of  $A_1$  for values of  $z$ .
- Figure 3.9 Function  $I_2$  in terms of  $A_2$  for values of  $z$ .
- Figure 4.1 The Zaire estuary.
- Figure 4.2 Map of Mateba with cross sections.
- Figure 4.3 Schematic representations of data files.
- Figure 4.4 Cross section geometry.
- Figure 4.5 Rating curve and geometric characteristics.
- Figure 5.1 Flowchart of Einstein model
- Figure 6.1 Bedload and suspended load (traditional Einstein Procedure) .
- Figure 6.2 Velocity variation along the cross section.
- Figure 6.3 Variation of sediment transport along the cross section for different slopes.

Figure 6.4 Variation of sediment transport along the cross section for different particle diameters.

Figure 6.5 Variation of measured and calculated sediment transport along the cross section.

Figure 6.6 Comparison of the results from Einstein and Bagnold model with the measured sediment transport rate.

## LIST OF SYMBOLS

A	Total area of a cross section
a	Thickness of bed layer
$A_B$	$a/d$ , Ratio of bed layer thickness to water depth.
$A_1$	Grain area constant.
$A_2$	Grain volume constant.
$A_3$	Constant of time scale.
$A_4$	Constant of bed layer concentration.
$A_5$	Constant of bed layer concentration.
$A_L$	Constant of the bed load unit step.
$A_{\phi}$	Constant referred to the scale of $\phi$ .
$A'$	Cross sectional area pertaining to the grain.
$A''$	Cross sectional area pertaining to irregularities.
B	Constant referred to the scale of $\psi$ .
$B'$	Constant referred to the scale of $\psi$ .
$B_{\psi}$	Constant referred to the scale of $\psi_x$ .
C	Concentration in dry weight per unit of volume.
$C_a$	Concentration at distance "a" from bed.
$C_L$	Lift coefficient.
$C_y$	Concentration at distance "y" from bed.
d	Water depth.
D	Grain diameter.
D35	Grain size of which 35 percent is finer.
D65	Grain size of which 65 percent is finer.
g	Acceleration due to gravity.
$i_b$	Fraction of bed material in a given grain size.
$i_B$	Fraction of bed load in a given grain size.
$i_s$	Fraction of suspension in a given grain size.
$i_T$	Fraction of total load in a given grain size.

$i_1$	Integral value.
$i_2$	Integral value.
$K_s$	Roughness diameter.
$l$	Distance.
$L$	Lift force on bed particle.
$l_e$	Distance of exchange, mixing length.
$p$	Probability of a grain to be eroded.
$PE$	Parameter of total transport.
$P$	Wetted perimeter of bed load.
$p_s$	Probability of a grain to be eroded per second.
$Q$	Flow discharge.
$q_v$	Vertical exchange discharge per unit area.
$R$	Total hydraulic radius.
$R^1$	Hydraulic radius with respect to the grain.
$R^*$	Hydraulic radius for channel irregularities.
$S$	Energy slope
$t$	Variable of integration.
$t_x$	Exchange time of bed load particles.
$u$	Velocity in direction of the main flow.
$\bar{u}$	Time-average of $u$ , averaged over the vertical.
$\bar{u}_y$	Time average of the velocity $u$ at $y$ above the bed.
$u_s$	Shear velocity.
$u_s^1$	Shear velocity with respect to the grain.
$u_s^*$	Shear velocity for channel irregularities.
$v$	A vertical velocity.
$M^1$	Weight of sediment particle under water.
$W$	Settling velocity of a sediment particle.
$x$	Parameter for transition smooth-rough.
$X$	Characteristic grain size of mixture.
$y$	Distance above the bed.

$\gamma$	Pressure correction in transition smooth-rough.
$z$	Exponent of suspended distribution.
$\theta$	A logarithmic function.
$\theta_*$	A logarithmic function.
$\delta$	The thickness of the laminar sublayer.
$\Delta$	The apparent roughness diameter.
$\eta$	Variability factor of lift.
$\eta_*$	Root mean square value of $\eta$
$\eta_*$	$\eta$ measures in $\eta_*$ values.
$\lambda$	Single step of bed load measured in diameters.
$\nu$	Kinematic viscosity.
$\rho$	"Hiding factor" of grains in a mixture.
$\rho$	Density of the fluid.
$\rho_s$	Density of the solids.
$\tau$	A shear stress.
$\tau_*$	The shear stress at the bed.
$\tau_y$	Shear stress at a distance $y$ from the bed.
$\phi$	Intensity of transport.
$\phi_*$	Intensity of transport for individual grain size.
$\psi$	Intensity of shear on representative particle.
$\psi_*$	Intensity of shear for individual grain size.

## APPENDIX

Appendix 1a: Data file CORDXY	129
Appendix 1b: Plott program PLCROS	129
Appendix 2 : Data file AVAL1	130
Appendix 3 : Data file ZADATA1	130
Appendix 4 : Computer program CHANGE	131
Appendix 5 : Data file ZADATA2	132
Appendix 6 : Computer program RATING	133
Appendix 7 : Data file RATING1	137
Appendix 8 : Data file RATING2	138
Appendix 9 : Data file RATING3	139
Appendix 10: Data file RATING4	140
Appendix 11: Computer program EINSTEIN	141
Appendix 12: Computer program EINSTE1	148
Appendix 13: Plott program PLSED1	153
Appendix 14: Result of Einstein model (two-dimensional approach)	155
Appendix 15: Result of Einstein model (traditional procedure)	159

APPENDIX 1a

	CORDX	CORDY
10	371375	343190
11	371580	343075
12	371585	342880
13	371645	342830
14	371750	342690
15	369220	341655
16	369260	341525
17	369300	341435
18	369375	341370
19	369400	341305
20	367770	340930
21	367875	340860
22	367930	340740
23	367995	340680
24	368015	340550
25	371175	343250
26	371882	342551
27	369197	341724
28	369441	341134
29	367695	341000
30	368118	340299

- 1 - 5 : MATEBA AMONT
- 6 - 10: MATEBA CENTRAL
- 11 - 15: MATEBA AVAL
- 16 - 17: LEFT AND RIGHT BANK (M. AMONT)
- 18 - 19: LEFT AND RIGHT BANK (M. CENTRAL)
- 20 - 21: LEFT AND RIGH BANK (M. AVAL)

APPENDIX 1b

```

PROGRAM PLCROS
INTEGER COD
DIMENSION COD(127),ZR(127),B(127),Z(127)
C*****
C
C PROGRAMME FOR PLOTTING THE CROSS SECTION OF THE ZAIRE RIVER
C IN 6 DIFFERENT PLACES
C*****
OPEN (1,FILE='ZADATA2')
DO 73 J=1,127
READ(1,*)COD(J),ZR(J),B(J)
Z(J)=(-1.)*ZR(J)
73 CONTINUE
NN=121
MM=127
CALL GROPEN
CALL PLSIZE(20.,12.)
CALL BOUNDS(0.,-12.8,818.7,0.)
CALL UPFCHT('CROSS-SECTION GEOMETRY')
CALL LOWCHT('SECTION... M ATEBA A VAL')
CALL XLABEL('DISTANCE FROM RIGH BANK... (M)')
CALL YLABEL('LEVEL ... (M)')
DO 36 J=NN,MM
CALL CUTYPE('SO')
CALL DRAW(B(J),Z(J))
36 CONTINUE
CALL GRCLOS
END

```

APPENDIX 2

J	V(1)	ZT	N	Z(2)	V(2)	Z(3)	V(3)	Z(4)	V(4)	Z(5)	V(5)	Z(6)	V(6)	Z(7)	V(7)	Z(8)	V(8)
	N/S	N	N	N/S	N	N/S	N	N/S	N	N/S	N	N/S	N	N/S	N	N/S	N/S
1	.98	3.30	6	.30	.98	1.00	1.00	1.40	1.01	2.10	.90	2.80	.81	3.10	.65	.00	.00
2	1.16	5.55	6	.30	1.16	1.00	1.13	2.20	1.06	3.30	1.00	4.40	.89	3.10	.63	.00	.00
3	1.18	7.00	7	.30	1.18	1.00	1.16	1.30	1.16	2.80	1.11	4.20	.97	3.50	.94	6.60	.63
4	1.12	7.95	7	.30	1.12	1.00	1.10	1.59	1.14	3.18	1.12	4.77	.99	3.36	.88	7.35	.59
5	1.08	9.05	7	.30	1.08	1.00	1.08	1.81	1.06	3.62	1.05	5.43	1.02	7.24	.81	8.65	.63
6	1.80	11.55	7	.30	1.80	1.00	1.82	2.33	1.48	4.66	1.40	6.99	1.34	9.22	1.17	11.25	.86
7	1.69	12.70	7	.30	1.69	1.00	1.64	2.54	1.36	5.08	1.36	7.62	1.43	10.16	1.27	12.30	.86
8	1.67	9.25	7	.30	1.67	1.00	1.69	1.85	1.42	3.70	1.38	5.55	1.49	7.40	1.23	8.85	.65
9	1.52	7.70	7	.30	1.52	1.00	1.47	1.54	1.45	3.08	1.40	4.62	1.37	6.16	1.06	7.30	.65
10	1.08	6.05	7	.30	1.08	1.00	1.06	1.21	1.09	2.42	1.02	3.62	.76	4.84	.85	5.36	.60
11	1.93	12.80	7	.30	1.93	1.00	1.92	2.56	1.47	5.12	1.39	7.68	1.29	10.24	1.07	12.40	.59
12	1.89	10.80	7	.30	1.89	1.00	1.80	2.16	1.58	4.32	1.32	6.48	1.37	8.64	1.34	10.40	.67
13	1.70	10.50	7	.30	1.70	1.00	1.71	2.10	1.41	4.20	1.44	6.30	1.44	8.40	1.24	10.10	.82
14	1.83	7.40	7	.30	1.83	1.00	1.45	1.48	1.50	2.96	1.42	4.44	1.24	5.92	1.19	7.00	.83
15	.99	5.85	6	.30	.99	1.00	.94	2.34	.88	3.51	.87	4.68	.77	5.45	.66	.00	.00

APPENDIX 3

J	ZT	B	Z(1)	V(1)	Z(2)	V(2)	Z(3)	V(3)	Z(4)	V(4)	Z(5)	V(5)	Z(6)	V(6)	Z(7)	V(7)	
	N	N	N	N/S	N	N/S	N	N/S	N	N/S	N	N/S	N	N/S	N	N/S	
1	.00	0	.00	.000	.00	.000	.00	.000	.00	.000	.00	.000	.00	.000	0	.000	
2	3.30	208	8	.00	.000	3.10	.630	2.80	.810	2.10	.900	1.40	1.010	1.00	1.000	3	.980
3	5.55	441	2	.00	.000	5.10	.630	4.40	.890	3.30	1.000	2.20	1.060	1.00	1.130	3	1.160
4	7.00	532	3	.60	.650	5.60	.940	4.20	.970	2.80	1.110	1.40	1.160	1.00	1.160	3	1.180
5	7.95	630	3	7.55	.590	6.36	.880	4.77	.990	3.18	1.120	1.89	1.140	1.00	1.100	3	1.120
6	9.05	892	4	8.65	.630	7.24	.810	5.43	1.020	3.62	1.090	1.81	1.060	1.00	1.080	3	1.080
7	.00	994	2	.00	.000	.00	.000	.00	.000	.00	.000	.00	.000	.00	.000	0	.000
8	.00	.00	0	.00	.000	.00	.000	.00	.000	.00	.000	.00	.000	.00	.000	0	.000
9	11.55	72	7	11.25	.860	9.38	1.170	6.97	1.340	4.66	1.400	2.33	1.480	1.00	1.320	3	1.300
10	12.70	208	7	12.30	.860	10.16	1.290	7.62	1.430	5.08	1.560	2.34	1.560	1.00	1.640	3	1.690
11	7.25	306	8	8.55	.650	7.40	1.230	5.90	1.450	3.70	1.380	1.85	1.620	1.00	1.690	3	1.630
12	7.70	396	2	7.30	.840	6.16	1.060	4.62	1.270	2.08	1.400	1.54	1.430	1.00	1.470	3	1.520
13	6.05	485	4	3.36	.660	4.84	.860	3.63	.960	2.42	1.020	1.21	1.090	1.00	1.060	3	1.080
14	.00	638	5	.00	.000	.00	.000	.00	.000	.00	.000	.00	.000	.00	.000	0	.000
15	.00	0	0	.00	.000	.00	.000	.00	.000	.00	.000	.00	.000	.00	.000	0	.000
16	12.80	102	6	12.40	.590	10.24	1.070	7.68	1.280	5.12	1.390	2.56	1.470	1.00	1.520	3	1.530
17	10.80	228	0	10.40	.870	8.64	1.340	6.48	1.390	4.32	1.320	2.16	1.580	1.00	1.600	3	1.590
18	10.80	350	5	10.10	.730	8.40	1.240	6.30	1.440	4.20	1.440	2.10	1.610	1.00	1.710	3	1.700
19	7.40	438	6	7.00	.830	5.92	1.150	4.44	1.240	2.96	1.420	1.48	1.500	1.00	1.430	3	1.530
20	3.85	532	2	.00	.000	5.45	.660	4.68	.770	3.51	.870	2.34	.680	1.00	.940	3	.990
21	.00	818	7	.00	.000	.00	.000	.00	.000	.00	.000	.00	.000	.00	.000	0	.000



```

PROGRAM CHANGE
  DIMENSION A(25), D(25), C(25), B(25), JJ(25), N(25)
  DIMENSION ZT(25), ZB(25), VB(25), Z7(25), V7(25), Z6(25), V6(25),
    Z5(25), V5(25), Z4(25), V4(25), Z3(25), V3(25), Z2(25), V2(25)
  OPEN (1, FILE='AVAL1')
  OPEN (2, FILE='CORDXY')
  OPEN (3, FILE='ZADATA1')
  DO 5 J=1, 15
  READ(1, *)JJ(J), V2(J), ZT(J), N(J), Z2(J), V2(J), Z3(J), V3(J),
    Z4(J), V4(J), Z5(J), V5(J), Z6(J), V6(J), Z7(J), V7(J), ZB(J), VB(J)
5 CONTINUE
  DO 1 I=1, 21
  READ(2, *)A(I), D(I), C(I)
1 CONTINUE
  A1=0.
  A2=0.
  A3=0.
  A4=0.
  A5=0.
  A6=0.
  A7=0.
  A8=0.
  A9=0.
  B1=0.
  B2=0.
  B3=0.
  B4=0.
  B5=0.
  B6=0.
  B7=0.
  K1=1
  K2=7
  K3=8
  K4=14
  K5=15
  K6=21
  DO 2 I=1, 5
  B(I) = ((D(I)-D(16))**2+(C(I)-C(16))**2)**0.5
2 CONTINUE
  DO 3 I=6, 10
  B(I) = ((D(I)-D(18))**2+(C(I)-C(18))**2)**0.5
3 CONTINUE
  DO 4 I=11, 15
  B(I) = ((D(I)-D(20))**2+(C(I)-C(20))**2)**0.5
4 CONTINUE
  B(17) = ((D(17)-D(16))**2+(C(17)-C(16))**2)**0.5
  B(19) = ((D(19)-D(18))**2+(C(19)-C(18))**2)**0.5
  B(21) = ((D(21)-D(20))**2+(C(21)-C(20))**2)**0.5
  WRITE(3, 50)K1, A1, A2, A3, A4, A5, A6, A7, A8, A9, B1, B2, B3,
    B4, B5, B6, B7
  DO 6 I=1, 5
  K=I+1
  WRITE(3, 50)K, ZT(I), B(I), ZB(I), VB(I), Z7(I), V7(I), Z6(I), V6(I),
    Z5(I), V5(I), Z4(I), V4(I), Z3(I), V3(I), Z2(I), V2(I)
6 CONTINUE
  WRITE(3, 50)K2, A1, B(17), A3, A4, A5, A6, A7, A8, A9, B1, B2, B3,
    B4, B5, B6, B7
  WRITE(3, 50)K3, A1, A2, A3, A4, A5, A6, A7, A8, A9, B1, B2, B3,
    B4, B5, B6, B7
  DO 7 I=6, 10
  K=I+3
  WRITE(3, 50)K, ZT(I), B(I), ZB(I), VB(I), Z7(I), V7(I), Z6(I), V6(I),
    Z5(I), V5(I), Z4(I), V4(I), Z3(I), V3(I), Z2(I), V2(I)
7 CONTINUE
  WRITE(3, 50)K4, A1, B(19), A3, A4, A5, A6, A7, A8, A9, B1, B2, B3,
    B4, B5, B6, B7
  WRITE(3, 50)K5, A1, A2, A3, A4, A5, A6, A7, A8, A9, B1, B2, B3,
    B4, B5, B6, B7
  DO 8 I=11, 15
  K=I+5
  WRITE(3, 50)K, ZT(I), B(I), ZB(I), VB(I), Z7(I), V7(I), Z6(I), V6(I),
    Z5(I), V5(I), Z4(I), V4(I), Z3(I), V3(I), Z2(I), V2(I)
8 CONTINUE
  WRITE(3, 50)K6, A1, B(21), A3, A4, A5, A6, A7, A8, A9, B1, B2, B3,
    B4, B5, B6, B7
90 FORMAT (I3, F6. 2, F7. 1, F6. 2, F6. 3, F6. 2, F6. 3, F6. 2, F6. 3, F6. 2
  , F6. 3, F6. 2, F6. 3, F6. 2, F6. 3, F6. 3, F6. 3, F6. 3, F6. 3)
  END

```

APPENDIX 5

STATION	TYPE	DESCRIPTION	DATE	TIME	REMARKS
2101	H	...	...	...	...
2102	H	...	...	...	...
2103	H	...	...	...	...
2104	H	...	...	...	...
2105	H	...	...	...	...
2106	H	...	...	...	...
2107	H	...	...	...	...
2108	H	...	...	...	...
2109	H	...	...	...	...
2110	H	...	...	...	...
2111	H	...	...	...	...
2112	H	...	...	...	...
2113	H	...	...	...	...
2114	H	...	...	...	...
2115	H	...	...	...	...
2116	H	...	...	...	...
2117	H	...	...	...	...
2118	H	...	...	...	...
2119	H	...	...	...	...
2120	H	...	...	...	...
2121	H	...	...	...	...
2122	H	...	...	...	...
2123	H	...	...	...	...
2124	H	...	...	...	...
2125	H	...	...	...	...
2126	H	...	...	...	...
2127	H	...	...	...	...
2128	H	...	...	...	...
2129	H	...	...	...	...
2130	H	...	...	...	...
2131	H	...	...	...	...
2132	H	...	...	...	...
2133	H	...	...	...	...
2134	H	...	...	...	...
2135	H	...	...	...	...
2136	H	...	...	...	...
2137	H	...	...	...	...
2138	H	...	...	...	...
2139	H	...	...	...	...
2140	H	...	...	...	...
2141	H	...	...	...	...
2142	H	...	...	...	...
2143	H	...	...	...	...
2144	H	...	...	...	...
2145	H	...	...	...	...
2146	H	...	...	...	...
2147	H	...	...	...	...
2148	H	...	...	...	...
2149	H	...	...	...	...
2150	H	...	...	...	...
2151	H	...	...	...	...
2152	H	...	...	...	...
2153	H	...	...	...	...
2154	H	...	...	...	...
2155	H	...	...	...	...
2156	H	...	...	...	...
2157	H	...	...	...	...
2158	H	...	...	...	...
2159	H	...	...	...	...
2160	H	...	...	...	...
2161	H	...	...	...	...
2162	H	...	...	...	...
2163	H	...	...	...	...
2164	H	...	...	...	...
2165	H	...	...	...	...
2166	H	...	...	...	...
2167	H	...	...	...	...
2168	H	...	...	...	...
2169	H	...	...	...	...
2170	H	...	...	...	...
2171	H	...	...	...	...
2172	H	...	...	...	...
2173	H	...	...	...	...
2174	H	...	...	...	...
2175	H	...	...	...	...
2176	H	...	...	...	...
2177	H	...	...	...	...
2178	H	...	...	...	...
2179	H	...	...	...	...
2180	H	...	...	...	...
2181	H	...	...	...	...
2182	H	...	...	...	...
2183	H	...	...	...	...
2184	H	...	...	...	...
2185	H	...	...	...	...
2186	H	...	...	...	...
2187	H	...	...	...	...
2188	H	...	...	...	...
2189	H	...	...	...	...
2190	H	...	...	...	...
2191	H	...	...	...	...
2192	H	...	...	...	...
2193	H	...	...	...	...
2194	H	...	...	...	...
2195	H	...	...	...	...
2196	H	...	...	...	...
2197	H	...	...	...	...
2198	H	...	...	...	...
2199	H	...	...	...	...
2200	H	...	...	...	...

```

PROGRAM RATING
INTEGER CODE
REAL Z, B, ZL, V, R2, B1, B2, Z1, Z2, G, A, VM, ZM, ZK, BK, MEANV
DIMENSION CODE(127), Z(127), B(127), ZL(127,7), V(127,7), R2(127,7),
B1(127), B2(127), Z1(127), Z2(127), G(127,7), A1(127,7), ZR(127),
RT(127,7), ZT(15), M1(15), M2(15), A2(127,7), R1(127,7), MEANV(127),
US(127), PP(127), RH(127)
OPEN (1, FILE='ZADATA2')
OPEN (2, FILE='RATING1')
OPEN (3, FILE='RATING2')
OPEN (4, FILE='RATING3')
OPEN (5, FILE='RATING4')
DO 14 J=1, 127
14 READ(1, *)CODE(J), Z(J), B(J), (ZL(J, I), V(J, I), I=1, 7)
CONTINUE
DO 15 J=1, 127
N=7
IF(V(J, 1).EQ.0.) K=2
R2(J, 1)=0.0
G(J, 1)=0.0
IF(V(J, 1).NE.0.) K=1
IF(Z(J).EQ.0) THEN
DO 81 I=1, N
R2(J, I)=0.0
G(J, I)=0.0
81 CONTINUE
GO TO 2000
ELSE IF(Z(J).NE.0) THEN
GO TO 160
ENDIF
160 CONTINUE
DO 255 I=K, N
R1(J, I)=Z(J)-ZL(J, I)
255 CONTINUE
X11=0.0
X12=0.0
X13=0.0
X14=0.0
DO 80 I=K, N
X11=X11+ALOG10(R1(J, I))
X12=X12+V(J, I)
X13=X13+ALOG10(R1(J, I))*V(J, I)
X14=X14+ALOG10(R1(J, I))*2
80 CONTINUE
GRAD=(N*X13-X11*X12)/(N*X14-X11**2)
A=X12/N-GRAD*X11/N
US(J)=ABS(GRAD)/5.75
IF(Z(J-1).EQ.0.AND.B(J).NE.B(J-1)) GO TO 200
IF(Z(J+1).EQ.0.AND.B(J).NE.B(J+1)) GO TO 500
IF(Z(J-1).EQ.0.AND.B(J).EQ.B(J-1)) GO TO 900
IF(Z(J+1).EQ.0.AND.B(J).EQ.B(J+1)) GO TO 900
IF(Z(J-1).NE.0.AND.Z(J+1).NE.0) GO TO 800
200 IF(Z(J)-Z(J+1)) 300,300,400
300 IF(Z(J)-Z(J-1)) 600,600,700
300 Z1(J)=0.0
Z2(J)=(Z(J+1)+Z(J))/2.
B1(J)=B(J)-B(J-1)
B2(J)=(B(J+1)-B(J))/2.
I=K
R2(J, I)=Z(J)-(ZL(J, I)+ZL(J, I+1))/2.
A1(J, I)=B2(J)*(Z2(J)-Z(J))/2.+R2(J, I)*B1(J)*R2(J, I)/(2.*Z(J))
+R2(J, I)*B2(J)
A2(J, I)=A1(J, I)
G(J, I)=ABS(A1(J, I))*V(J, I)
340 I=I+1
R2(J, I)=Z(J)-(ZL(J, I)+ZL(J, I+1))/2.
A1(J, I)=B2(J)*(R2(J, I)-R2(J, I-1))+(R2(J, I)-R2(J, I-1))*(B1(J)
*(R2(J, I)-R2(J, I-1)))/(2.*Z(J))
A2(J, I)=A2(J, I-1)+A1(J, I)
G(J, I)=G(J, I-1)+A1(J, I)*V(J, I)
IF(I.EQ.(N-1)) GO TO 360
IF(I.NE.(N-1)) GO TO 340
360 I=N
R2(J, I)=Z(J)
A1(J, I)=B2(J)*(R2(J, I)-R2(J, I-1))+(R2(J, I)-R2(J, I-1))*(B1(J)+
B1(J)=R2(J, I-1)/Z(J))/2.
A2(J, I)=A2(J, I-1)+A1(J, I)
G(J, I)=G(J, I-1)+A1(J, I)*V(J, I)

```

```

GO TO 2000
400 Z1(J)=0.0
   B1(J)=B(J)-B(J-1)
   Z2(J)=(Z(J+1)+Z(J))/2.
   B2(J)=(B(J+1)-B(J))/2.
   I=K
   R2(J,I)=Z(J)-(ZL(J,I)+ZL(J,I+1))/2.
   A1(J,I)=B2(J)*((Z(J)-Z2(J))/2.+(Z1(J)-(ZL(J,I)+ZL(J,I+1))/2.))
   +R2(J,I)*B1(J)*R2(J,I)/(2.*Z(J))
   A2(J,I)=A1(J,I)
   Q(J,I)=ABS(A1(J,I)*V(J,I))
440 I=I+1
   R2(J,I)=Z(J)-(ZL(J,I)+ZL(J,I+1))/2.
   A1(J,I)=((ZL(J,I-1)-ZL(J,I+1))/2.)*B2(J)+B1(J)*(R2(J,I-1)+
   R2(J,I))*(ZL(J,I-1)-ZL(J,I+1))/(4.*Z(J))
   A2(J,I)=A2(J,I-1)+A1(J,I)
   Q(J,I)=Q(J,I-1)+A1(J,I)*V(J,I)
   IF(I.EQ.(N-1)) GO TO 460
   IF(I.NE.(N-1)) GO TO 440
460 I=N
   R2(J,I)=Z(J)
   A1(J,I)=(ZL(J,I)+(ZL(J,I-1)-ZL(J,I))/2.)*B2(J)+((B1(J)+(B1(J)
   +R2(J,I-1))/(2.*Z(J)))*(ZL(J,I)+(ZL(J,I-1)-ZL(J,I))/2.))
   A2(J,I)=A2(J,I-1)+A1(J,I)
   Q(J,I)=Q(J,I-1)+A1(J,I)*V(J,I)
GO TO 2000
600 Z1(J)=(Z(J)+Z(J-1))/2.
   B1(J)=(B(J)-B(J-1))/2.
   Z2(J)=0.0
   B2(J)=B(J+1)-B(J)
   I=K
   R2(J,I)=Z(J)-(ZL(J,I)+ZL(J,I+1))/2.
   A1(J,I)=B1(J)*((Z(J)-Z1(J))/2.+R2(J,I)*B2(J)*R2(J,I)/(2.*Z(J))
   +R2(J,I)*B1(J))
   A2(J,I)=A1(J,I)
   Q(J,I)=ABS(A1(J,I)*V(J,I))
640 I=I+1
   R2(J,I)=Z(J)-(ZL(J,I)+ZL(J,I+1))/2.
   A1(J,I)=B1(J)*(ZL(J,I-1)-ZL(J,I+1))/2.+B2(J)*(R2(J,I-1)+R2(J,I))
   *(ZL(J,I-1)-ZL(J,I+1))/(4.*Z(J))
   A2(J,I)=A2(J,I-1)+A1(J,I)
   Q(J,I)=Q(J,I-1)+A1(J,I)*V(J,I)
   IF(I.EQ.(N-1)) GO TO 660
   IF(I.NE.(N-1)) GO TO 640
660 I=N
   R2(J,I)=Z(J)
   A1(J,I)=(ZL(J,I)+(ZL(J,I-1)-ZL(J,I))/2.)*B1(J)+((B2(J)+(B2(J)
   +R2(J,I-1))/(2.*Z(J)))*(ZL(J,I)+(ZL(J,I-1)-ZL(J,I))/2.))
   A2(J,I)=A2(J,I-1)+A1(J,I)
   Q(J,I)=Q(J,I-1)+A1(J,I)*V(J,I)
GO TO 2000
700 Z1(J)=(Z(J)+Z(J-1))/2.
   B1(J)=(B(J)-B(J-1))/2.
   Z2(J)=0.0
   B2(J)=B(J+1)-B(J)
   I=K
   R2(J,I)=Z(J)-(ZL(J,I)+ZL(J,I+1))/2.
   A1(J,I)=B1(J)*((Z(J)-Z1(J))/2.+(Z1(J)-(ZL(J,I)+ZL(J,I+1))/2.))
   +R2(J,I)*B2(J)*R2(J,I)/(2.*Z(J))
   A2(J,I)=A1(J,I)
   Q(J,I)=ABS(A1(J,I)*V(J,I))
740 I=I+1
   R2(J,I)=Z(J)-(ZL(J,I)+ZL(J,I+1))/2.
   A1(J,I)=B1(J)*(ZL(J,I-1)-ZL(J,I+1))/2.+B2(J)*(R2(J,I-1)+R2(J,I))
   *(ZL(J,I-1)-ZL(J,I+1))/(4.*Z(J))
   A2(J,I)=A2(J,I-1)+A1(J,I)
   Q(J,I)=Q(J,I-1)+A1(J,I)*V(J,I)
   IF(I.EQ.(N-1)) GO TO 760
   IF(I.NE.(N-1)) GO TO 740
760 I=N
   R2(J,I)=Z(J)
   A1(J,I)=B1(J)*(ZL(J,I)+(ZL(J,I-1)-ZL(J,I))/2.)+(B2(J)+(B2(J)
   +R2(J,I-1))/(2.*Z(J)))*(ZL(J,I)+(ZL(J,I-1)-ZL(J,I))/2.))
   A2(J,I)=A2(J,I-1)+A1(J,I)
   Q(J,I)=Q(J,I-1)+A1(J,I)*V(J,I)
GO TO 2000
800 Z1(J)=(Z(J)+Z(J-1))/2.
   Z2(J)=(Z(J)+Z(J+1))/2.
   IF(B(J-1).EQ.B(J-2)) THEN
   B1(J)=B(J)-B(J-1)

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B2(J)=(B(J+1)-B(J))/2.
ELSE IF(B(J-1).NE.B(J-2)) THEN
B1(J)=(B(J)-B(J-1))/2
B2(J)=(B(J+1)-B(J))/2
ENDIF
IF(B(J+1).EQ.B(J+2)) THEN
B1(J)=(B(J)-B(J-1))/2
B2(J)=B(J+1)-B(J)
ELSE IF(B(J+1).NE.B(J+2)) THEN
B1(J)=(B(J)-B(J-1))/2
B2(J)=(B(J+1)-B(J))/2
ENDIF
I=K
R2(J,I)=Z(J)-(ZL(J,I)+ZL(J,I+1))/2.
A1(J,I)=(B1(J)+B2(J))*((Z1(J)+Z(J)+Z2(J))/3.-
(ZL(J,I)+ZL(J,I+1))/2.)
A2(J,I)=A1(J,I)
G(J,I)=ABS(A1(J,I)*V(J,I))
840 I=I+1
R2(J,I)=Z(J)-(ZL(J,I)+ZL(J,I+1))/2.
A1(J,I)=(B1(J)+B2(J))*((ZL(J,I-1)-ZL(J,I+1))/2.
A2(J,I)=A2(J,I-1)+A1(J,I)
G(J,I)=G(J,I-1)+A1(J,I)*V(J,I)
IF(I.EQ.(N-1)) GO TO 860
IF(I.NE.(N-1)) GO TO 840
860 I=N
R2(J,I)=Z(J)
A1(J,I)=(B1(J)+B2(J))*((ZL(J,I)+(ZL(J,I-1)-ZL(J,I))/Z)
A2(J,I)=A2(J,I-1)+A1(J,I)
G(J,I)=G(J,I-1)+A1(J,I)*V(J,I)
GO TO 2000
900 DO 82 I=1,N
R2(J,I)=0.0
G(J,I)=0.0
A2(J,I)=0.0
82 CONTINUE
GO TO 2000
2000 WRITE(2,5000)J,Z(J),B(J),(R2(J,I),G(J,I),I=1,7)
IF(A2(J,7).EQ.0) THEN
MEANV(J)=0.0
PP(J)=0.0
RH(J)=0.0
ELSE IF(A2(J,7).NE.0) THEN
MEANV(J)=G(J,7)/A2(J,7)
PP(J)=B1(J)+B2(J)
RH(J)=A2(J,7)/PP(J)
ENDIF
WRITE(5,7000)J,Z(J),G(J,7),A2(J,7),MEANV(J),US(J),PP(J),RH(J)
IF(J.GE.1.AND.J.LE.6) ZR(J)=12.20-Z(J)
IF(J.GE.7.AND.J.LE.22) ZR(J)=9.04-Z(J)
IF(J.GE.23.AND.J.LE.28) ZR(J)=17.58-Z(J)
IF(J.GE.29.AND.J.LE.40) ZR(J)=9.77-Z(J)
IF(J.GE.41.AND.J.LE.47) ZR(J)=14.04-Z(J)
IF(J.GE.48.AND.J.LE.63) ZR(J)=14.77-Z(J)
IF(J.GE.64.AND.J.LE.72) ZR(J)=10.09-Z(J)
IF(J.GE.73.AND.J.LE.82) ZR(J)=8.89-Z(J)
IF(J.GE.83.AND.J.LE.95) ZR(J)=9.69-Z(J)
IF(J.GE.96.AND.J.LE.106) ZR(J)=18.31-Z(J)
IF(J.GE.107.AND.J.LE.113) ZR(J)=9.05-Z(J)
IF(J.GE.114.AND.J.LE.120) ZR(J)=12.7-Z(J)
IF(J.GE.121.AND.J.LE.127) ZR(J)=12.8-Z(J)
DO 10 I=1,7
RT(J,I)=R2(J,I)+ZR(J)
10 CONTINUE
WRITE(3,5000)J,ZR(J),B(J),(RT(J,I),G(J,I),I=1,7)
15 CONTINUE
DATA (ZT(K),K=1,13)/12.2,9.04,17.58,9.77,14.04,14.77,
10.09,8.89,9.69,18.31,9.05,12.7,12.8/
DATA(M1(K),K=1,13)/1,7,23,29,41,48,64,73,83,
96,107,114,121/
DATA(M2(K),K=1,13)/6,22,28,40,47,63,72,82,95,
106,113,120,127/
DO 35 K=1,13
24 KK=M1(K)
MM=M2(K)
WD=ZT(K)
H=1.0
25 S=0.0
A=0.0

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P=0.0
DG 60 J=NA.MM
IF(G(J,2).EQ.0) THEN
GO TO 17
ELSE IF(G(J,2).NE.0) THEN
IF(H.GT.ZR(J).AND.H.LE.RT(J,1)) THEN
S=S+(H-ZR(J))*G(J,1)/(RT(J,1)-ZR(J))
ENDIF
IF(H.LE.RT(J,2).AND.H.GT.RT(J,1)) THEN
S=S+G(J,1)*(H-RT(J,1))*G(J,2)-G(J,1))/(RT(J,2)-RT(J,1))
ENDIF
IF(H.LE.RT(J,3).AND.H.GT.RT(J,2)) THEN
S=S+G(J,2)*(H-RT(J,2))*G(J,3)-G(J,2))/(RT(J,3)-RT(J,2))
ENDIF
IF(H.LE.RT(J,4).AND.H.GT.RT(J,3)) THEN
S=S+G(J,3)*(H-RT(J,3))*G(J,4)-G(J,3))/(RT(J,4)-RT(J,3))
ENDIF
IF(H.LE.RT(J,5).AND.H.GT.RT(J,4)) THEN
S=S+G(J,4)*(H-RT(J,4))*G(J,5)-G(J,4))/(RT(J,5)-RT(J,4))
ENDIF
IF(H.LE.RT(J,6).AND.H.GT.RT(J,5)) THEN
S=S+G(J,5)*(H-RT(J,5))*G(J,6)-G(J,5))/(RT(J,6)-RT(J,5))
ENDIF
IF(H.LE.WD.AND.H.GT.RT(J,6)) THEN
S=S+G(J,6)*(H-RT(J,6))*G(J,7)-G(J,6))/(WD-RT(J,6))
ENDIF
ENDIF
17 CONTINUE
IF(H.LT.ZR(J)) THEN
GO TO 41
ENDIF
IF(H.GE.ZR(J)) GO TO 44
44 CONTINUE
IF(H.EQ.WD) THEN
IF(J.EQ.MM) GO TO 41
IF(J.NE.MM) GO TO 43
43 A=A+(2*WD-ZR(J)-ZR(J+1))*(B(J+1)-B(J))/2
P=P+B(J+1)-B(J)
ELSE IF(H.NE.WD) THEN
IF(H.LT.ZR(J-1).AND.H.GE.ZR(J+1)) THEN
X=(H-ZR(J))*(B(J)-B(J-1))/(ZR(J-1)-ZR(J))
A=A+X*(H-ZR(J))/2
P=P+X
ENDIF
IF(H.LT.ZR(J-1).AND.H.LT.ZR(J+1)) THEN
X=(H-ZR(J))*(B(J+1)-B(J))/(ZR(J+1)-ZR(J))
+(B(J)-B(J-1))*(H-ZR(J))/(ZR(J-1)-ZR(J))
A=A+X*(H-ZR(J))/2
P=P+X
ENDIF
IF(H.GE.ZR(J-1).AND.H.GE.ZR(J+1)) THEN
X=B(J)-B(J-1)
A=A+((H-ZR(J))*(H-ZR(J-1)))*X/2
P=P+X
ENDIF
IF(H.GE.ZR(J-1).AND.H.LT.ZR(J+1)) THEN
X1=(H-ZR(J))*(B(J+1)-B(J))/(ZR(J+1)-ZR(J))
X=B(J)-B(J-1)
A=A+X1*(H-ZR(J))/2+(H-ZR(J)+H-ZR(J-1))*X/2
P=P+X1+X
ENDIF
ENDIF
41 CONTINUE
60 CONTINUE
R=A/P
WRITE(4,6000)H,S,A,P,R
IF((WD-H).GE.1.) THEN
H=H+1.0
GO TO 25
ELSE IF((WD-H).LT.1.) THEN
GO TO 40
ENDIF
40 IF(WD-H) 71,71,70
70 H=WD
GO TO 25
71 CONTINUE
35 CONTINUE
6000 FORMAT (1X,F5.2,3X,F9.2,3X,F9.2,3X,F7.2,3X,F6.2)
5000 FORMAT (13,F6.2,F7.1,F6.2,F8.2,F6.2,F8.2,F6.2,F8.2,F6.2,F8.2,
F6.2,F8.2,F6.2,F8.2)
7000 FORMAT (13,F8.2,3X,F8.2,3X,F8.2,1X,F8.2,F7.3,1X,F8.2,3X,F8.2)
END

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PROGRAM EINSTEN
REAL ISIG, ISTIG, IST, M, NG, ISRP2, AS, N
DIMENSION HS(200), BS(200), PS(200), AS(200), RS(200)
      , R1(50), U1(50), DELTA1(50), M(50), X1(50), DELTA2(50)
10 ABEL(50), U1(50), C(50), M2(50), U2(50), R2(50), R(50), H(50), Q1(50)
      , X2(50), Y(50), BX(50), B(50), FE(50), DO(50, 5), FF(50, 5)
      , ISTIG(50, 5), BLSF(50, 5), BLEC(50, 5), TELEC(50, 5), IHIG(50, 5)
      , AE(50, 5), N(50), Z(50, 5), F11(50, 5), F12(50, 5), IST(50, 5)
      , BPLSF(50, 5), BPREC(50, 5), BPRAS(50, 5), TELSL(50, 5)
      , D(5), T(5), FBN(5), R7(6), H7(6), Y7(4), A(50), F(50), G(50), UC(50)
      , ISRP2(50, 5), SB(50), PFF(200), GOG(200), RENE(10)
CHARACTER*30 AYDE
OPEN (1, FILE='RATING3')
OPEN (2, FILE='BEDIN1')
OPEN (3, FILE='BEDIN2')
DATA R7(1), R7(2), R7(3), R7(4), R7(5), R7(6)
/ 10, 010, 0010, 00010, 000010, 0000010/
DATA H7(1), H7(2), H7(3), H7(4), H7(5), H7(6)
/ 020, 0020, 00020, 000020, 0000020, 00000020/
DATA Y7(1), Y7(2), Y7(3), Y7(4), Y7(5), Y7(6)
/ 0, 0000000, 0, 000, 0, 00000, 0, 0000000, 0, 0000000/
DATA V15, D65, D35, G/O, 000000841, 0, 000325, 0, 0001567, 7, 81/
DATA D(1), D(2), D(3), D(4)
/ 000212, 000254, 000312, 000370/
DATA FBN(1), FBN(2), FBN(3), FBN(4) / .25, .25, .25, .25/
DATA SD, FD/2650, 0, 1000, 0/
WRITE(*,*) 'XXXXXXXXXXXXXXXXXXXXX'
WRITE(*,*) 'DATA FILE: RATING3'
WRITE(*,*) 'XXXXXXXXXXXXXXXXXXXXX'
N=166
DO 10 J=1, N
READ(1, *)M4, GG, AA, PP, RR
HS(J)=M4
BS(J)=GG
PS(J)=PP
AS(J)=AA
RS(J)=RR
CONTINUE
10 WRITE(*,*) 'NAME OF CROSS SECTION'
WRITE(*,*) '
WRITE(*,*) 'AMONT OISEAUX 1 ..... N1=1, N2=13'
WRITE(*,*) 'AMONT OISEAUX 2 ..... N1=14, N2=23'
WRITE(*,*) 'OISEAUX NORD 1 ..... N1=24, N2=41'
WRITE(*,*) 'OISEAUX NORD 2 ..... N1=42, N2=51'
WRITE(*,*) 'BARRAJE 1 ..... N1=52, N2=65'
WRITE(*,*) 'BARRAJE 2 ..... N1=67, N2=81'
WRITE(*,*) 'MATEBA AMONT SUD 1 .. N1=92, N2=92'
WRITE(*,*) 'MATEBA AMONT SUD 2 .. N1=93, N2=101'
WRITE(*,*) 'MATEBA SUD MANDUO ... N1=102, N2=111'
WRITE(*,*) 'MATEBA SUD KAPITA ... N1=112, N2=130'
WRITE(*,*) 'MATEBA AMONT ... N1=131, N2=140'
WRITE(*,*) 'MATEBA CENTRAL ... N1=141, N2=155'
WRITE(*,*) 'MATEBA AVAL ... N1=154, N2=166'
WRITE(*,*) 'XXXXXXXXXXXXXXXXXXXXXXXXXXXXX'
WRITE(*,*) 'WRITE THE NAME OF THE CROSS SECTION'
READ(*, I)AYDE
11 FORMAT(A30)
WRITE(*,*) 'GIVE THE VALUES OF: N1, N2'
READ(*, *)N1, N2
WRITE(*,*) 'THE TOTAL WATER DEPTH (HT) IS:'
READ(*, *)HT
WRITE(*,*) 'THE ENERGY LINE SLOPE (S) IS:'
READ(*, *)S
WRITE(*,*) 'THE INCREMENT FOR R1(I), IS:'
READ(*, *)DIP
X11=0.0
X12=0.0
X13=0.0
X14=0.0
DO 60 J=N1, N2
X11=X11+RS(J)
X12=X12+HS(J)
X13=X13+RS(J)*HS(J)
X14=X14+RS(J)**2
60 CONTINUE
RS=(X13-X11*X12/(N2-N1+1))/(X14-X11**2/(N2-N1+1))
AS=X12/(N2-N1+1)-RS*X11/(N2-N1+1)
NS=0.65
I=1

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NN=1
R1(I)=DIF
26 U1(I)=(R1(I)*G*E)*E 5
DELTA1(I)=(11.8*VIS)/U1(I)
N(I)=KB/DELTA1(I)
XXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXX
X CALCULATION OF CORRECTION FACTOR IN THE X
X LOGARITHMIC VELOCITY DISTRIBUTION (X) X
XXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXX
C C C C
IF(N(I), LE. 0.5) X1(I)=.7+(ALOG10(N(I))+.899)*1.7085
IF((N(I), LE. 0.8), AND. (N(I), GT. 0.5)) THEN
X1(I)=1.38+(ALOG10(N(I))+.3010)*1.2626
ENDIF
IF((N(I), LE. 0.7), AND. (N(I), GT. 0.6)) THEN
X1(I)=1.48+(ALOG10(N(I))+.2218)*1.0463
ENDIF
IF((N(I), LE. 0.8), AND. (N(I), GT. 0.7)) THEN
X1(I)=1.55+(ALOG10(N(I))+.1349)*.9172
ENDIF
IF((N(I), LE. 0.9), AND. (N(I), GT. 0.8)) THEN
X1(I)=1.58+(ALOG10(N(I))+.0969)*.8871
ENDIF
IF((N(I), LE. 1.0), AND. (N(I), GT. 0.9)) THEN
X1(I)=1.61+(ALOG10(N(I))+.0458)*.8283
ENDIF
IF((N(I), LE. 1.2), AND. (N(I), GT. 1.0)) THEN
X1(I)=1.62-(ALOG10(N(I))+.2525)
ENDIF
IF((N(I), LE. 1.4), AND. (N(I), GT. 1.2)) THEN
X1(I)=1.6-(ALOG10(N(I))-0.072)*.8979
ENDIF
IF((N(I), LE. 3.0), AND. (N(I), GT. 1.4)) THEN
X1(I)=1.55-(ALOG10(N(I))-1.461)*1.0876
ENDIF
IF((N(I), LE. 3.5), AND. (N(I), GT. 3.0)) THEN
X1(I)=1.2-(ALOG10(N(I))-4.771)*.8955
ENDIF
IF((N(I), LE. 4.5), AND. (N(I), GT. 3.5)) THEN
X1(I)=1.14-(ALOG10(N(I))-5.441)*.85
ENDIF
IF((N(I), LE. 6.0), AND. (N(I), GT. 4.5)) THEN
X1(I)=1.08-(ALOG10(N(I))-6.502)*.84
ENDIF
IF((N(I), LE. 9.0), AND. (N(I), GT. 6.0)) THEN
X1(I)=1.03-(ALOG10(N(I))-7.782)*.8705
ENDIF
IF((N(I), GT. 9.0)) X1(I)=1.0
DELTA2(I)=KB/X1(I)
U(I)=U1(I)*(5.75+ALOG10(12.27*N(I)/DELTA2(I)))
C(I)=(50-FD)*D35/(FD*R1(I)*B)
XXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXX
X FRICTION U* DUE TO CHANNEL IRREGULARITIES X
X FOR BAR RESISTANCE X
XXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXX
C C C C
IF(C(I), LE. 1.0) M2(I)=10**((2.0-(ALOG10(C(I))+0.3188))*1.2654)
IF((C(I), LE. 1.5), AND. (C(I), GT. 1.0)) THEN
M2(I)=10**((1.5766-(ALOG10(C(I))-0.0000)*1.0775)
ENDIF
IF((C(I), LE. 2.0), AND. (C(I), GT. 1.5)) THEN
M2(I)=10**((1.4065-(ALOG10(C(I))-0.1761)*0.8447)
ENDIF
IF((C(I), LE. 3.0), AND. (C(I), GT. 2.0)) THEN
M2(I)=10**((1.3010-(ALOG10(C(I))-0.3010)*0.664)
ENDIF
IF((C(I), LE. 4.0), AND. (C(I), GT. 3.0)) THEN
M2(I)=10**((1.1847-(ALOG10(C(I))-0.4771)*0.3664)
ENDIF
IF((C(I), LE. 6.0), AND. (C(I), GT. 4.0)) THEN
M2(I)=10**((1.1137-(ALOG10(C(I))-0.6021)*0.5031)
ENDIF
IF((C(I), LE. 10.), AND. (C(I), GT. 6.0)) THEN
M2(I)=10**((1.0253-(ALOG10(C(I))-0.7782)*0.4094)
ENDIF
IF((C(I), LE. 100.), AND. (C(I), GT. 10.)) THEN
M2(I)=10**((0.7343-(ALOG10(C(I))-1.01)*.3781)
ENDIF
IF(C(I), GT. 100.) M2(I)=10**((.5441-(ALOG10(C(I))-2.0)*.3784)
U2(I)=U(I)/M2(I)
R2(I)=(U2(I)*E2)/(D*E)

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R(I)=R1(I)+R2(I)
DC(I)=(R(I)+0.05)*.3
H(I)=AS+DS*R(I)
IF(I.EQ.1) THEN
  ABEL(I)=0.0
  GO TO 17
ELSE IF(I.NE.1) THEN
  ENDOF
18 IF(H(I).LE.H(I-1)) THEN
  RM=4W
  RENE(RM)=H(I-1)
  ABEL(I)=100.
  GO TO 29
ELSE IF(H(I).GT.H(I-1)) THEN
  ABEL(I)=0.0
  GO TO 29
ENDIF
29 IF(H(I).LE.RENE(I)) THEN
  ABEL(I)=100.
  GO TO 16
ELSE IF(H(I).GT.RENE(I)) THEN
  ABEL(I)=0.0
  GO TO 17
ENDIF
17 CONTINUE
RT=(HT-AS)/DS
XXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXX
% CALCULATION OF DISCHARGE Q(I), WETTED PERIMETER
% P(I), AREA A(I), AND HYDRAULIC RADIUS R(I), FROM %
% DATA FILE ..... RATING CURVE %
XXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXX
IF(H(I).LE.HS(I)) THEN
  Q(I)=H(I)*QS(I)/HS(I)
  P(I)=H(I)*PS(I)/HS(I)
  A(I)=H(I)*AS(I)/HS(I)
ENDIF
IF(H(I).GT.HS(I).AND.H(I).LE.HS(N2)) THEN
  DO 250 J=N1,N2-1
  IF(H(I).LE.HS(J+1).AND.H(I).GT.HS(J)) THEN
    Q(I)=QS(J)+(H(I)-HS(J))*(QS(J+1)-QS(J))/(HS(J+1)-HS(J))
    P(I)=PS(J)+(H(I)-HS(J))*(PS(J+1)-PS(J))/(HS(J+1)-HS(J))
    A(I)=AS(J)+(H(I)-HS(J))*(AS(J+1)-AS(J))/(HS(J+1)-HS(J))
  ENDOF
250 CONTINUE
ENDIF
IF(H(I).GT.HS(N2)) THEN
  Q(I)=QS(N2)+(H(I)-HS(N2))*(QS(N2)-QS(N2-1))/(HS(N2)-HS(N2-1))
  P(I)=PS(N2)+(H(I)-HS(N2))*(PS(N2)-PS(N2-1))/(HS(N2)-HS(N2-1))
  A(I)=AS(N2)+(H(I)-HS(N2))*(AS(N2)-AS(N2-1))/(HS(N2)-HS(N2-1))
ENDIF
Q(I)=A(I)*U(I)
IF((DELTA2(I)/DELTA1(I)).GE.1.50) X2(I)=0.77*DELTA2(I)
IF((DELTA2(I)/DELTA1(I)).LT.1.50) X2(I)=1.37*DELTA1(I)
XXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXX
% PRESSURE CORRECTION FACTOR Y(I), IN THE %
% TRANSITION TO A SMOOTH BED %
XXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXX
IF(N(I).LE.0.1) Y(I)=10**(-2.+(ALOG10(H(I))+1.6576)*1.226)
IF((N(I).LE.145).AND.(N(I).GT.0.1)) THEN
  Y(I)=10**(-1.1938+(ALOG10(H(I))+1.0000)*1.2007)
ENDIF
IF((N(I).LE.55).AND.(N(I).GT.145)) THEN
  Y(I)=10**(-1.000+(ALOG10(H(I))+0.8386)*1.2073)
ENDIF
IF((N(I).LE.70).AND.(N(I).GT.55)) THEN
  Y(I)=10**(-.3010+(ALOG10(H(I))+0.2596)*1.0879)
ENDIF
IF((N(I).LE.76).AND.(N(I).GT.70)) THEN
  Y(I)=10**(-.1871+(ALOG10(H(I))+0.1549)*0.9020)
ENDIF
IF((N(I).LE.90).AND.(N(I).GT.76)) THEN
  Y(I)=10**(-.1549+(ALOG10(H(I))+0.1192)*0.7891)
ENDIF
IF((N(I).LE.1.0).AND.(N(I).GT.90)) THEN
  Y(I)=10**(-.0969+(ALOG10(H(I))+0.0457)*0.4639)
ENDIF
IF((N(I).LE.1.1).AND.(N(I).GT.1.0)) THEN
  Y(I)=10**(-.0757+(ALOG10(H(I))+0.0000)*0.1232)

```

```

ENDIF
IF((N(I).LE.1.2).AND.(M(I).GT.1.1)) THEN
Y(I)=10**(-.0706-(ALOG10(M(I))-0.0414)*0.1349)
ENDIF
IF((N(I).LE.1.4).AND.(M(I).GT.1.2)) THEN
Y(I)=10**(-.0737-(ALOG10(M(I))-0.0792)*0.3991)
ENDIF
IF((N(I).LE.1.65).AND.(M(I).GT.1.4)) THEN
Y(I)=10**(-.1024-(ALOG10(M(I))-0.1461)*0.7353)
ENDIF
IF((N(I).LE.1.8).AND.(M(I).GT.1.65)) THEN
Y(I)=10**(-.1549-(ALOG10(M(I))-0.2175)*1.0291)
ENDIF
IF((N(I).LE.2.0).AND.(M(I).GT.1.8)) THEN
Y(I)=10**(-.1938-(ALOG10(M(I))-0.2553)*0.6127)
ENDIF
IF((N(I).LE.2.5).AND.(M(I).GT.2.0)) THEN
Y(I)=10**(-.2218-(ALOG10(M(I))-0.3010)*0.3096)
ENDIF
IF((N(I).LE.3.0).AND.(M(I).GT.2.5)) THEN
Y(I)=10**(-.2518-(ALOG10(M(I))-0.3779)*0.0985)
ENDIF
IF((N(I).LE.3.5).AND.(M(I).GT.3.0)) THEN
Y(I)=10**(-.2596-(ALOG10(M(I))-0.4771)*0.1194)
ENDIF
IF(M(I).GT.3.5) Y(I)=10**(-.2676-(ALOG10(M(I))-0.5441)*0.0523)
BX(I)=ALOG10(10.6*X2(I)/DELTA2(I))
B(I)=ALOG10(10.6)
BB(I)=(B(I)/BX(I))*2
PE(I)=2.303*ALOG10(30.2*H(I)/DELTA2(I))
IF(R(I)-RT) 28,27,27
28 I=I+1
R1(I)=R1(I-1)+DIF
GO TO 26
16 NN=NN+1
I=I+1
R1(I)=R1(I-1)+DIF
GO TO 26
27 N3=I
WRITE(2,1000)AYDE
WRITE(2,2500)
DO 710 I=1,N3
WRITE(2,2000) R1(I),U1(I),DELTA1(I),M(I),X1(I),DELTA2(I)
,U(I),C(I),M2(I),U2(I),R2(I),R(I)
710 CONTINUE
WRITE(2,1100)AYDE
WRITE(2,3500)
DO 720 I=1,N3
WRITE(2,3000) US(I),H(I),A(I),P(I),G(I),G1(I),X2(I),Y(I)
,BX(I),BB(I),PE(I)
720 CONTINUE
*****
N BED MATERIAL LOAD CALCULATION
*****
DO 110 K=1,4
DO 120 I=1,N3
IF(ABEL(I).EQ.100.) GO TO 120
ISRP2(I,K)=(SD-FD)*D(K)/(FD*R1(I)+6)
DD(I,K)=D(K)/X2(I)
XXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXX
X HIDING FACTOR FF(I,K), IN TERMS OF D(K)/X2(I) X
XXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXX
IF(DD(I,K).LE.0.10) FF(I,K)=10**(.2.9542-(ALOG10(DD(I,K))
+.1.3468)*2.2436)
IF((DD(I,K).LE.12).AND.(DD(I,K).GT.0.10)) THEN
FF(I,K)=10**(.2.1761-(ALOG10(DD(I,K))+1.0000)*2.2235)
ENDIF
IF(DD(I,K).LE.255).AND.(DD(I,K).GT.12)) THEN
FF(I,K)=10**(.2.0000-(ALOG10(DD(I,K))+0.9208)*2.1350)
ENDIF
IF(DD(I,K).LE.64).AND.(DD(I,K).GT.255)) THEN
FF(I,K)=10**(.1.3010-(ALOG10(DD(I,K))+0.5934)*2.5025)
ENDIF
IF(DD(I,K).LE.7).AND.(DD(I,K).GT.64)) THEN
FF(I,K)=10**(.0.3010-(ALOG10(DD(I,K))+0.1938)*2.0129)
ENDIF
IF(DD(I,K).LE.0.8).AND.(DD(I,K).GT.0.7)) THEN
FF(I,K)=10**(.0.2227-(ALOG10(DD(I,K))+0.1549)*1.3207)
ENDIF
IF(DD(I,K).LE.0.9).AND.(DD(I,K).GT.0.8)) THEN

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IF(T(K), LE. 0. 10) M(I)=10**(-1. 0767+(ALOG10(T(K))+Z. 0)*0. 9602)
IF(T(K), LE. 12), AND. (T(K), GT. 0. 10)) THEN
M(I)=10**(-1. 1367+(ALOG10(T(K))+1. 0)*1. 7260)
ENDIF
IF(T(K), LE. 2), AND. (T(K), GT. 12)) THEN
M(I)=10**(-0. 0000+(ALOG10(T(K))+0. 9202)*1. 6300)
ENDIF
IF(T(K), LE. 0. 3), AND. (T(K), GT. 0. 2)) THEN
M(I)=10**(-0. 3617+(ALOG10(T(K))+0. 6787)*1. 3034)
ENDIF
IF(T(K), LE. 0. 5), AND. (T(K), GT. 0. 3)) THEN
M(I)=10**(-0. 5711+(ALOG10(T(K))+0. 5227)*0. 9072)
ENDIF
IF(T(K), LE. 1. 0), AND. (T(K), GT. 0. 5)) THEN
M(I)=10**(-0. 7924+(ALOG10(T(K))+0. 3010)*0. 6897)
ENDIF
IF(T(K), LE. 10. ), AND. (T(K), GT. 1. 0)) THEN
M(I)=10**(-1. 0000+(ALOG10(T(K))-0. 0000)*0. 5563)
ENDIF
IF(T(K), LE. 100. ), AND. (T(K), GT. 10. )) THEN
M(I)=10**(-1. 5563+(ALOG10(T(K))-1. 0000)*0. 4647)
ENDIF
IF(T(K), GT. 100. ) M(I)=10**(-2. 0212+(ALOG10(T(K))-2. )*0. 4973)
Z(I,K)=M(I)/(100*. 4973)
XXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXX
IN  CALCULATION OF FUNCTIONS F11(I,K) AND F12(I,K)
IN  IN TERMS OF AE(I,K) AND Z(I,K)
XXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXX
L=1
A7=1. 0
SUM1=0. 0
SUM2=0. 0
35 R6=A7(L)
H6=A7(L)
Y6=Y7(L)
20 A7=A7-H6
Y11=((1. -Y6)/Y6)**Z(I,K)
Y12=((1. -Y6-0. 5*H6)/(Y6+0. 5*H6))**Z(I,K)
Y13=((1. -Y6+0. 5*H6)/(Y6-0. 5*H6))**Z(I,K)
Y21=((1. -Y6)/Y6)**Z(I,K)*ALOG(Y6)
Y22=((1. -Y6-0. 5*H6)/(Y6+0. 5*H6))**Z(I,K)*ALOG(Y6+0. 5*H6)
Y23=((1. -Y6+0. 5*H6)/(Y6-0. 5*H6))**Z(I,K)*ALOG(Y6-0. 5*H6)
SUM1=SUM1+(Y12+4*Y11+Y13)*H6/6.
SUM2=SUM2+(Y22+4*Y21+Y23)*H6/6.
IF(A7. EQ. (H6+AE(I,K))) GO TO 200
IF(A7. NE. (H6+AE(I,K))) GO TO 300
300 CONTINUE
IF((A7-AE(I,K))-H6) 30, 30, 40
40 IF(A7-(R6+H6)) 15, 25, 25
25 Y6=A7-0. 5*H6
GO TO 20
30 Y6=(A7+AE(I,K))/2.
H6=A7-AE(I,K)
Y6=A7-0. 5*H6
GO TO 20
15 L=L+1
GO TO 35
200 G1=(. 216)*(AE(I,K)**(Z(I,K)-1. ))/((1. -AE(I,K))**Z(I,K))
F11(I,K)=SUM1*G1
F12(I,K)=SUM2*G1
IST(I,K)=PE(I)*F11(I,K)+F12(I,K)
BMLSP(I,K)=BLSP(I,K)*IST(I,K)
BMREC(I,K)=BMLSP(I,K)*P(I)
IF(K. EQ. 1) THEN
JJ=1
FFF(JJ)=BMREC(I,K)
ELSE IF(K. NE. 1) THEN
JJ=(K-1)*N3+1
FFF(JJ)=BMREC(I,K)+FFF(JJ-N3)
ENDIF
IF(K. EQ. 1) BMRAS(I,K)=BMREC(I,K)
IF(K. NE. 1) BMRAS(I,K)=BMREC(I,K)+FFF(JJ-N3)
TBLSL(I,K)=TBLEC(I,K)+BMRAS(I,K)
WRITE(3, 8000)H(I), G(I), BLEC(I,K), TBLEC(I,K), BMREC(I,K)
, BMRAS(I,K), TBLSL(I,K)
120 CONTINUE
110 CONTINUE
WRITE(2, 1200)AYDE
WRITE(2, 4300)
DO 730 K=1,4
DO 740 I=1,N3

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WRITE(2, 4000) D(K), FBM(K), R1(I), ISRP2(I, K), DD(I, K), FF(I, K)
, ISIG(I, K), ISTIG(I, K), BLSF(I, K), BLEC(I, K), TBLEC(I, K)
740 CONTINUE
WRITE(2, 1500)
730 CONTINUE
WRITE(2, 1300) AYDE
WRITE(2, 5500)
DO 750 K=1, 4
DO 760 I=1, N3
WRITE(2, 5000) AE(I, K), W(I), Z(I, K), FI1(I, K), FI2(I, K), IST(I, K)
, BMLSF(I, K), BNREC(I, K), BMRAS(I, K), TBLBL(I, K)

760 CONTINUE
WRITE(2, 1500)
750 CONTINUE
1000 FORMAT(///24X, 'CROSS SECTION: ', 2X, A30, //
15X, 'HYDRAULICS CALCULATIONS...')
1100 FORMAT(///18X, 'CROSS SECTION: ', 2X, A30, //
7X, 'HYDRAULICS CALCULATIONS...'
, 1X, '(CONTINUED)')
1200 FORMAT(///24X, 'CROSS SECTION: ', 2X, A30, //
13X, 'SEDIMENT TRANSPORT CALCUL', 1X,
'ATION...')
1300 FORMAT(///20X, 'CROSS SECTION: ', 2X, A30, //
5X, 'SEDIMENT TRANSPORT CAL', 1X,
'ULATION. (CONTINUED)')
2500 FORMAT(2X, 'R1', 4X, 'U1', 3X, 'DELTA1', 5X, 'W', 4X, 'X1', 3X, 'DELTA2'
, 4X, 'U', 6X, 'C', 8X, 'W2', 5X, 'U2', 6X, 'R2', 5X, 'R'
, 1X, '(1)', 3X, '(2)', 4X, '(3)', 6X, '(4)', 3X, '(5)', 4X, '(6)', 4X, '(7)'
, 4X, '(8)', 7X, '(9)', 3X, '(10)', 3X, '(11)', 3X, '(12)')
2000 FORMAT(F4. 2, 1X, F6. 4, 1X, F7. 5, 1X, F6. 3, 1X, F5. 3, 1X, F7. 5, 1X,
F5. 2, 2X, F7. 3, 2X, F6. 2, 2X, F5. 3, 3X, F4. 2, 3X, F5. 2, /)
3500 FORMAT(2X, 'UC', 4X, 'H', 5X, 'A', 7X, 'P', 6X, 'G', 8X, 'Q1', 7X, 'X2',
6X, 'Y', 4X, 'X', 5X, 'BB', 3X, 'PE'
, 1X, '(13)', 2X, '(14)', 2X, '(15)', 4X, '(16)', 4X, '(17)'
, 4X, '(18)', 5X, '(19)', 3X, '(20)', 2X, '(21)', 2X, '(22)', 3X, '(23)')
3000 FORMAT(F5. 3, 1X, F5. 2, 1X, F7. 1, 1X, F6. 1, 1X, F7. 1,
1X, F6. 6, 1X, F5. 3, 1X, F5. 3, 1X, F5. 3, 1X, F6. 3, /)
1500 FORMAT(2X, 90(' '))
4500 FORMAT(4X, 'D', 3X, 'FBM', 4X, 'R1', 4X, 'ISRP2', 4X, 'DD', 6X, 'FF',
7X, 'ISIG', 4X, 'ISTIG', 4X, 'BLSF', 6X, 'BLEC', 5X, 'TBLEC'
, 3X, '(1)', 4X, '(2)', 4X, '(3)', 4X, '(4)', 5X, '(5)', 5X, '(6)', 7X, '(7)'
, 6X, '(8)', 5X, '(9)', 6X, '(10)', 7X, '(11)')
4000 FORMAT(F8. 6, 1X, F4. 2, 1X, F5. 2, 3X, F7. 3, 1X
, F5. 3, 3X, F7. 3, 2X, F6. 3, 1X, F6. 3, 1X, F6. 5, 1X, F6. 3, 3X, F9. 3)
5500 FORMAT(4X, 'AE', 5X, 'W', 6X, 'Z', 7X, 'FI1', 7X, 'FI2',
6X, 'IST', 5X, 'BMLSF', 4X, 'BNREC', 5X, 'BMRAS', 5X, 'TBLBL'
, 3X, '(12)', 3X, '(13)', 2X, '(14)', 5X, '(15)', 6X, '(16)', 6X, '(17)'
, 4X, '(18)', 5X, '(19)', 6X, '(20)', 7X, '(21)')
5000 FORMAT(F8. 6, 1X, F4. 2, 1X, F5. 2, 1X, F9. 4, 1X, F9. 4, 1X,
F8. 3, 1X, F7. 3, 1X, F9. 3, 1X, F9. 3, 1X, F10. 3)
8000 FORMAT(1X, F5. 2, 2X, F7. 1, 2X, F9. 3, 2X, F9. 3, 2X, F9. 3, 2X, F10. 3)
WRITE(*, *) 'DO YOU WANT TO RUN FOR AN OTHER CROSS SECTION?'
WRITE(*, *) 'WRITE (1) IF YES OR (0) IF NOT'
READ(*, *) LAURA
IF(LAURA.EQ.1) GO TO 9
IF(LAURA.EQ.0) GO TO 799
799 CONTINUE
WRITE(*, *) 'FILE: SEDIM1 (RESULTS OF EINSTEIN MODEL)'
WRITE(*, *) 'FILE: SEDIM2 (OUTPUT FOR PLOTTING)'
END

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PROGRAM EINSTE1
REAL ISIG, ISTIG, IST, M, M2, ISRP2, KS, N
DIMENSION H5(200), G5(200), P5(200), A5(200), R5(200)
, III(130), R1(130), U1(130), DELTA1(130), M(130), X1(130), DELTA2(130)
, U(130), C(130), M2(130), U2(130), R2(130), R(130), H(130), G1(130)
, US(130), X2(130), Y(130), BX(130), B(130), PE(130), DD(130), FF(130)
, R3(130), ISTIG(130), BLSF(130), BLEC(130), TBLEC(130), ISIG(130)
, AE(130), W(130), Z(130), F11(130), F12(130), IST(130)
, BMSLF(130), BMREC(130), BMRAS(130), TBLSL(130)
, T(5), R7(6), H7(6), Y7(6), A(130), P(130), Q(130), UC(130)
, ISRP2(130), BB(130), FFF(200), GGG(200), RENE(10)
OPEN (1, FILE='RATING4')
OPEN (2, FILE='SEDIM3')
OPEN (3, FILE='SEDIM4')
DATA R7(1), R7(2), R7(3), R7(4), R7(5), R7(6)
/ .10, .010, .0010, .00010, .000010, .0000010/
DATA H7(1), H7(2), H7(3), H7(4), H7(5), H7(6)
/ .020, .0020, .00020, .000020, .0000020, .00000020/
DATA Y7(1), Y7(2), Y7(3), Y7(4), Y7(5), Y7(6)
/ 0.9899999, 0.999, 0.9999, 0.99999, 0.999999, 0.9999999/
DATA VIS, D65, D35, FBM, G/O, 000000841, 0.000329,
0.0001567, 1.0, 9.81/
DATA SD, FD/2650, 0, 1000, 0/
N=127
DO 10 I=1,N
READ(1,*)III(I), H(I), G(I), A(I), U(I), US(I), P(I), R(I)
10 CONTINUE
WRITE(*,*) 'THE ENERGY LINE SLOPE (S) IS:'
READ(*,*)S
WRITE(*,*) 'THE DIAMETER OF THE PARTICLE (D50) IS:'
READ(*,*)D50
KS=D65
DO 50 I=1,N
IF(US(I).EQ.0) GO TO 55
65 U1(I)=(R(I)*0*S)**.5
DELTA1(I)=(11.6*VIS)/U1(I)
M(I)=KS/DELTA1(I)
C %XXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXX
C %
C % CALCULATION OF CORRECTION FACTOR IN THE %
C % LOGARITHMIC VELOCITY DISTRIBUTION (X1) %
C %XXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXX
IF(M(I).LE.0.5) X1(I)=7+(ALOG10(M(I))+.699)*1.7085
IF((M(I).LE.0.6).AND.(M(I).GT.0.5)) THEN
X1(I)=1.38+(ALOG10(M(I))+.3010)*1.2626
ENDIF
IF((M(I).LE.0.7).AND.(M(I).GT.0.6)) THEN
X1(I)=1.48+(ALOG10(M(I))+.2218)*1.0463
ENDIF
IF((M(I).LE.0.8).AND.(M(I).GT.0.7)) THEN
X1(I)=1.55+(ALOG10(M(I))+.1549)*.5172
ENDIF
IF((M(I).LE.0.9).AND.(M(I).GT.0.8)) THEN
X1(I)=1.58+(ALOG10(M(I))+.0969)*.5871
ENDIF
IF((M(I).LE.1.0).AND.(M(I).GT.0.9)) THEN
X1(I)=1.61+(ALOG10(M(I))+.0458)*.2183
ENDIF
IF((M(I).LE.1.2).AND.(M(I).GT.1.0)) THEN
X1(I)=1.62-(ALOG10(M(I)))*.2525
ENDIF
IF((M(I).LE.1.4).AND.(M(I).GT.1.2)) THEN
X1(I)=1.6-(ALOG10(M(I)))-.0792)*.5979
ENDIF
IF((M(I).LE.3.0).AND.(M(I).GT.1.4)) THEN
X1(I)=1.56-(ALOG10(M(I))-1.461)*1.0876
ENDIF
IF((M(I).LE.3.5).AND.(M(I).GT.3.0)) THEN
X1(I)=1.2-(ALOG10(M(I))-4.771)*.8933
ENDIF
IF((M(I).LE.4.5).AND.(M(I).GT.3.5)) THEN
X1(I)=1.14-(ALOG10(M(I))-5.441)*.55
ENDIF
IF((M(I).LE.6.0).AND.(M(I).GT.4.5)) THEN
X1(I)=1.08-(ALOG10(M(I))-6.532)*.4
ENDIF
IF((M(I).LE.9.0).AND.(M(I).GT.6.0)) THEN
X1(I)=1.03-(ALOG10(M(I))-7.782)*.1705

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ENDIF
IF((M(I).GT.9.0)) X1(I)=1.0
DELTA2(I)=KS/X1(I)
C(I)=(SD-FD)*D35/(FD*R1(I)*5)
C % FRICTION U** DUE TO CHANNEL IRREGULARITIES %
C % FOR BAR RESISTANCE %
C %XXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXX%
IF((C(I).LE.1.0) M2(I)=10**((2.0-(ALOG10(C(I))+0.3188)*1.2654)
IF((C(I).LE.1.5).AND.(C(I).GT.1.0)) THEN
M2(I)=10**((1.5966-(ALOG10(C(I))-0.0000)*1.0795)
ENDIF
IF((C(I).LE.2.0).AND.(C(I).GT.1.5)) THEN
M2(I)=10**((1.4065-(ALOG10(C(I))-0.1761)*0.8447)
ENDIF
IF((C(I).LE.3.0).AND.(C(I).GT.2.0)) THEN
M2(I)=10**((1.3010-(ALOG10(C(I))-0.3010)*0.664)
ENDIF
IF((C(I).LE.4.0).AND.(C(I).GT.3.0)) THEN
M2(I)=10**((1.1847-(ALOG10(C(I))-0.4771)*0.5664)
ENDIF
IF((C(I).LE.6.0).AND.(C(I).GT.4.0)) THEN
M2(I)=10**((1.1139-(ALOG10(C(I))-0.6021)*0.5031)
ENDIF
IF((C(I).LE.10.)AND.(C(I).GT.6.0)) THEN
M2(I)=10**((1.0253-(ALOG10(C(I))-0.7782)*0.4094)
ENDIF
IF((C(I).LE.100.)AND.(C(I).GT.10.)) THEN
M2(I)=10**((0.9345-(ALOG10(C(I))-1.0)*.3984)
ENDIF
IF(C(I).GT.100.) M2(I)=10**((.5441-(ALOG10(C(I))-2.0)*.3984)
U2(I)=U(I)/M2(I)
R2(I)=R(I)-R1(I)
R(I)=R1(I)+R2(I)
IF((DELTA2(I)/DELTA1(I)).GE.1.80) X2(I)=0.77*DELTA2(I) }
IF((DELTA2(I)/DELTA1(I)).LT.1.80) X2(I)=1.39*DELTA1(I) }
C %XXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXX%
C % PRESSURE CORRECTION FACTOR Y(I), IN THE %
C % TRANSITION TO A SMOOTH BED %
C %XXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXX%
IF(M(I).LE.0.1) Y(I)=10**((-2.+(ALOG10(M(I))+1.6576)*1.226)
IF((M(I).LE.145).AND.(M(I).GT.0.1)) THEN
Y(I)=10**((-1.1938+(ALOG10(M(I))+1.0000)*1.2007)
ENDIF
IF((M(I).LE.55).AND.(M(I).GT.145)) THEN
Y(I)=10**((-1.000+(ALOG10(M(I))+0.8386)*1.2073)
ENDIF
IF((M(I).LE.70).AND.(M(I).GT.55)) THEN
Y(I)=10**((-3.010+(ALOG10(M(I))+0.2596)*1.0879)
ENDIF
IF((M(I).LE.76).AND.(M(I).GT.70)) THEN
Y(I)=10**((-1.871+(ALOG10(M(I))+0.1549)*0.9020)
ENDIF
IF((M(I).LE.90).AND.(M(I).GT.76)) THEN
Y(I)=10**((-1.549+(ALOG10(M(I))+0.1192)*0.7891)
ENDIF
IF((M(I).LE.1.0).AND.(M(I).GT.90)) THEN
Y(I)=10**((-0.9889+(ALOG10(M(I))+0.0457)*0.4639)
ENDIF
IF((M(I).LE.1.1).AND.(M(I).GT.1.0)) THEN
Y(I)=10**((-0.0757+(ALOG10(M(I))+0.0000)*0.1232)
ENDIF
IF((M(I).LE.1.2).AND.(M(I).GT.1.1)) THEN
Y(I)=10**((-0.0706-(ALOG10(M(I))-0.0414)*0.1349)
ENDIF
IF((M(I).LE.1.4).AND.(M(I).GT.1.2)) THEN
Y(I)=10**((-0.0737-(ALOG10(M(I))-0.0792)*0.3991)
ENDIF
IF((M(I).LE.1.65).AND.(M(I).GT.1.4)) THEN
Y(I)=10**((-1.024-(ALOG10(M(I))-0.1461)*0.7353)
ENDIF
IF((M(I).LE.1.8).AND.(M(I).GT.1.65)) THEN
Y(I)=10**((-1.549-(ALOG10(M(I))-0.2175)*1.0291)
ENDIF
IF((M(I).LE.2.0).AND.(M(I).GT.1.8)) THEN
Y(I)=10**((-1.935-(ALOG10(M(I))-0.2553)*0.6127)
ENDIF
IF((M(I).LE.2.5).AND.(M(I).GT.2.0)) THEN
Y(I)=10**((-2.218-(ALOG10(M(I))-0.3010)*0.3096)

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ENDIF
IF((M(I).LE.3.0).AND.(M(I).GT.2.5)) THEN
Y(I)=10**(-.2518-(ALOG10(M(I))-0.3979)*0.0985)
ENDIF
IF((M(I).LE.3.5).AND.(M(I).GT.3.0)) THEN
Y(I)=10**(-.2596-(ALOG10(M(I))-0.4771)*0.1194)
ENDIF
IF(M(I).GT.3.5) Y(I)=10**(-.2676-(ALOG10(M(I))-0.5441)*0.0523)
BX(I)=ALOG10(10.6*X2(I)/DELTA2(I))
B(I)=ALOG10(10.6)
BB(I)=(B(I)/BX(I))**2
PE(I)=2.303*ALOG10(30.2*H(I)/DELTA2(I))
CXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXX
C% BED MATERIAL LOAD CALCULATION %
CXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXX
ISRP2(I)=(SD-FD)*D50/(FD*R1(I)*S)
DD(I)=D50/X2(I)
CXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXX
C% HIDING FACTOR FF(I), IN TERMS OF D50/X2(I) %
CXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXX
IF(DD(I).LE.0.10) FF(I)=10**((2.9542-(ALOG10(DD(I))
+.1.3468)*2.2436)
IF((DD(I).LE.12).AND.(DD(I).GT.0.10)) THEN
FF(I)=10**((2.1761-(ALOG10(DD(I)))+1.0000)*2.2235)
ENDIF
IF((DD(I).LE.255).AND.(DD(I).GT.12)) THEN
FF(I)=10**((2.0000-(ALOG10(DD(I)))+0.9208)*2.1350)
ENDIF
IF((DD(I).LE.64).AND.(DD(I).GT.255)) THEN
FF(I)=10**((1.3010-(ALOG10(DD(I)))+0.5934)*2.5025)
ENDIF
IF((DD(I).LE.71).AND.(DD(I).GT.64)) THEN
FF(I)=10**((0.3010-(ALOG10(DD(I)))+0.1936)*2.0129)
ENDIF
IF((DD(I).LE.0.8).AND.(DD(I).GT.0.7)) THEN
FF(I)=10**((0.2227-(ALOG10(DD(I)))+0.1549)*1.3207)
ENDIF
IF((DD(I).LE.0.9).AND.(DD(I).GT.0.8)) THEN
FF(I)=10**((0.1461-(ALOG10(DD(I)))+0.0969)*1.0313)
ENDIF
IF((DD(I).LE.1.0).AND.(DD(I).GT.0.9)) THEN
FF(I)=10**((0.0934-(ALOG10(DD(I)))+0.0458)*0.7140)
ENDIF
IF((DD(I).LE.1.1).AND.(DD(I).GT.1.0)) THEN
FF(I)=10**((0.0607-(ALOG10(DD(I)))-0.0000)*0.4662)
ENDIF
IF((DD(I).LE.1.3).AND.(DD(I).GT.1.1)) THEN
FF(I)=10**((0.0414-(ALOG10(DD(I)))-0.0414)*0.3945)
ENDIF
IF((DD(I).LE.1.5).AND.(DD(I).GT.1.3)) THEN
FF(I)=10**((0.0128-(ALOG10(DD(I)))-0.1139)*0.2058)
ENDIF
IF(DD(I).GT.1.5) FF(I)=1.0
ISIG(I)=FF(I)*Y(I)*ISRP2(I)*(B(I)/BX(I))**2
CXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXX
C% INTENSITY OF TRANSPORT FOR INDIVIDUAL GRAIN %
C% SIZE. . . . . ISTIG(I) %
CXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXX
IF(ISTIG(I).LE.0.26) THEN
ISTIG(I)=10**((2.0-(ALOG10(ISTIG(I)))+1.0969)*1.0215)
ENDIF
IF(ISTIG(I).LE.0.75).AND.(ISTIG(I).GT.0.26)) THEN
ISTIG(I)=10**((1.4771-(ALOG10(ISTIG(I)))+0.5850)*1.0369)
ENDIF
IF(ISTIG(I).LE.1.70).AND.(ISTIG(I).GT.0.75)) THEN
ISTIG(I)=10**((1.0-(ALOG10(ISTIG(I)))+0.1249)*1.1199)
ENDIF
IF(ISTIG(I).LE.2.7).AND.(ISTIG(I).GT.1.70)) THEN
ISTIG(I)=10**((0.6021-(ALOG10(ISTIG(I)))-0.2304)*1.4980)
ENDIF
IF(ISTIG(I).LE.4.1).AND.(ISTIG(I).GT.2.7)) THEN
ISTIG(I)=10**((0.3010-(ALOG10(ISTIG(I)))-0.4314)*1.6593)
ENDIF
IF(ISTIG(I).LE.5.0).AND.(ISTIG(I).GT.4.1)) THEN
ISTIG(I)=10**((0.0-(ALOG10(ISTIG(I)))-0.6128)*2.5761)
ENDIF
IF(ISTIG(I).LE.7.6).AND.(ISTIG(I).GT.5.0)) THEN
ISTIG(I)=10**((-2218-(ALOG10(ISTIG(I)))-0.6989)*2.6229)
ENDIF

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IF((ISIG(I). LE. 7. 40). AND. (ISIG(I). GT. 7. 6)) THEN
ISTIG(I)=10**(-. 6989-(ALOG10(ISIG(I))-0. 8808)*3. 2622)
ENDIF
IF((ISIG(I). LE. 12. ). AND. (ISIG(I). GT. 9. 40)) THEN
ISTIG(I)=10**(-1. 000-(ALOG10(ISIG(I))-. 9731)*3. 7502)
ENDIF
IF((ISIG(I). LE. 16. 5). AND. (ISIG(I). GT. 12. )) THEN
ISTIG(I)=10**(-1. 3979-(ALOG10(ISIG(I))-1. 0792)*4. 3536)
ENDIF
IF((ISIG(I). LE. 19. 5). AND. (ISIG(I). GT. 16. 5)) THEN
ISTIG(I)=10**(-2. 000-(ALOG10(ISIG(I))-1. 2175)*7. 2124)
ENDIF
IF((ISIG(I). LE. 22. ). AND. (ISIG(I). GT. 19. 5)) THEN
ISTIG(I)=10**(-2. 5227-(ALOG10(ISIG(I))-1. 2900)*9. 1050)
ENDIF
IF((ISIG(I). LE. 24. ). AND. (ISIG(I). GT. 22. )) THEN
ISTIG(I)=10**(-3. 000-(ALOG10(ISIG(I))-1. 3424)*10. 4987)
ENDIF
IF((ISIG(I). GT. 24. 0)) THEN
ISTIG(I)=10**(-3. 3979-(ALOG10(ISIG(I))-1. 3802)*11. 7598)
ENDIF
BLSF(I)=FBM*ISTIG(I)*SD*(G**1. 5)*(D50**1. 5)*((SD/FD-1)
** 5)*86. 4/9. 81
BLEC(I)=BLSF(I)*P(I)
AE(I)=2*D50/H(I)
C XXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXX
C % CALCULATION OF SETTLING VELOCITY... W(I) %
C XXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXX
T(K)=1000*D50
IF(T(K). LE. 0. 10) W(I)=10**(-1. 0969+(ALOG10(T(K))+2. 0)*0. 9602)
IF(T(K). LE. . 12). AND. (T(K). GT. 0. 10)) THEN
W(I)=10**(-. 1367+(ALOG10(T(K))+1. 0)*1. 7260)
ENDIF
IF(T(K). LE. . 2). AND. (T(K). GT. . 12)) THEN
W(I)=10**(. 0. 0000+(ALOG10(T(K))+0. 9208)*1. 6300)
ENDIF
IF(T(K). LE. 0. 3). AND. (T(K). GT. 0. 2)) THEN
W(I)=10**(. 0. 3617+(ALOG10(T(K))+0. 6989)*1. 3034)
ENDIF
IF(T(K). LE. 0. 5). AND. (T(K). GT. 0. 3)) THEN
W(I)=10**(. 0. 5911+(ALOG10(T(K))+0. 5229)*0. 9072)
ENDIF
IF(T(K). LE. 1. 0). AND. (T(K). GT. 0. 5)) THEN
W(I)=10**(. 0. 7924+(ALOG10(T(K))+0. 3010)*0. 6897)
ENDIF
IF(T(K). LE. 10. ). AND. (T(K). GT. 1. 0)) THEN
W(I)=10**(. 1. 0000+(ALOG10(T(K))-0. 0000)*0. 5563)
ENDIF
IF(T(K). LE. 100. ). AND. (T(K). GT. 10. )) THEN
W(I)=10**(. 1. 5563+(ALOG10(T(K))-1. 0000)*0. 4649)
ENDIF
IF(T(K). GT. 100. )) W(I)=10**(. 2. 0212+(ALOG10(T(K))-2. )*. 4973)
Z(I)=W(I)/(100*. 4*U1(I))
C XXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXX
C % CALCULATION OF FUNCTIONS F1(I) AND F2(I) %
C % IN TERMS OF AE(I) AND Z(I) %
C XXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXX
L=1
A7=1. 0
SUM1=0. 0
SUM2=0. 0
35 R6=R7(L)
H6=H7(L)
Y6=Y7(L)
20 A7=A7-H6
Y11=((1. -Y6)/Y6)**Z(I)
Y12=((1. -Y6-0. 5*H6)/(Y6+0. 5*H6))**Z(I)
Y13=((1. -Y6+0. 5*H6)/(Y6-0. 5*H6))**Z(I)
Y21=((1. -Y6)/Y6)**Z(I)*ALOG(Y6)
Y22=((1. -Y6-0. 5*H6)/(Y6+0. 5*H6))**Z(I)*ALOG(Y6+0. 5*H6)
Y23=((1. -Y6+0. 5*H6)/(Y6-0. 5*H6))**Z(I)*ALOG(Y6-0. 5*H6)
SUM1=SUM1+(Y12+4*Y11+Y13)*H6/6.
SUM2=SUM2+(Y22+4*Y21+Y23)*H6/6.
IF(A7. EQ. (H6+AE(I))) GO TO 200
IF(A7. NE. (H6+AE(I))) GO TO 300
300 CONTINUE
IF((A7-AE(I))-H6) 30, 30, 40
40 IF(A7-(R6+H6)) 15, 25, 25
25 Y6=A7-0. 5*H6
GO TO 20
30 Y6=(A7+AE(I))/2.
H6=A7-AE(I)

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Y6-A7-0.5*H6
GO TO 20
15 L=L+1
GO TO 35
200 Q1=(.216)*(AE(I)**(Z(I)-1.))/((1.-AE(I))**Z(I))
F11(I)=SUM1*Q1
F12(I)=SUM2*Q1
IST(I)=FE(I)*F11(I)+F12(I)
BMLSF(I)=BLSF(I)*IST(I)
BMREC(I)=BMLSF(I)*P(I)
TBLSL(I)=BLEC(I)+BMREC(I)
55 CONTINUE
50 CONTINUE
WRITE(2,1000)
WRITE(2,2500)
DO 710 I=1,N
WRITE(2,2000)I,R1(I),U1(I),DELTA1(I),M(I),X1(I),DELTA2(I)
,U(I),C(I),M2(I),U2(I),R2(I),R(I)
710 CONTINUE
WRITE(2,1100)
WRITE(2,3500)
DO 720 I=1,N
WRITE(2,3000)I,US(I),H(I),A(I),P(I),G(I),X2(I),Y(I)
,BX(I),BB(I),PE(I)
720 CONTINUE
WRITE(2,1200)
WRITE(2,4500)
DO 740 I=1,N
WRITE(2,4000)I,D50,FBM,R1(I),ISRP2(I),DD(I),FF(I)
,ISIG(I),ISTIG(I),BLSF(I),BLEC(I)
740 CONTINUE
WRITE(2,1300)
WRITE(2,5500)
DO 740 I=1,N
WRITE(2,5000)I,AE(I),W(I),Z(I),F11(I),F12(I),IST(I)
,BMLSF(I),BMREC(I),TBLSL(I)
760 CONTINUE
DO 730 I=1,127
WRITE(3,6000)I,BLSF(I),BLEC(I),BMLSF(I),BMREC(I),TBLSL(I)
730 CONTINUE
1000 FORMAT(///24X,'EINSTEIN MODEL: TWO DIMENSIONAL APPROACH'///
15X,'HYDRAULICS CALCULATIONS...')
1100 FORMAT(///20X,'EINSTEIN MODEL: TWO DIMENSIONAL APPROACH'///
7X,'HYDRAULICS CALCULATIONS...')
1X,'(CONTINUED)')
1200 FORMAT(///20X,'EINSTEIN MODEL: TWO DIMENSIONAL APPROACH'///
13X,'SEDIMENT TRANSPORT CALCUL',1X,
'A T I O N...')
1300 FORMAT(///20X,'EINSTEIN MODEL: TWO DIMENSIONAL APPROACH'///
5X,'SEDIMENT TRANSPORT CAL',1X,
'C U L A T I O N... (CONTINUED)')
2500 FORMAT(8X,'R1',4X,'U1',3X,'DELTA1',5X,'M',4X,'X1',3X,
'DELTA2',4X,'U',6X,'C',8X,'M2',5X,'U2',5X,'R2',6X,'R',
7X,'(1)',3X,'(2)',4X,'(3)',6X,'(4)',3X,'(5)',4X,'(6)',4X,'(7)',
4X,'(8)',7X,'(9)',3X,'(10)',3X,'(11)',3X,'(12)')
2000 FORMAT(14,2X,F4,2,1X,F6,4,1X,F7,5,1X,F6,3,1X,F5,3,1X,F7,5,1X,
F5,2,2X,F7,3,2X,F6,2,2X,F5,3,3X,F5,2,3X,F5,2)
3500 FORMAT(8X,'US',4X,'H',5X,'A',8X,'P',6X,'G',7X,'X2',
6X,'Y',4X,'BX',5X,'BB',4X,'PE',
7X,'(13)',2X,'(14)',2X,'(15)',4X,'(16)',4X,'(17)',
5X,'(18)',3X,'(19)',2X,'(20)',2X,'(21)',3X,'(22)')
3000 FORMAT(14,2X,F5,3,1X,F5,2,1X,F7,1,1X,F6,1,1X,F7,1,
1X,F8,6,1X,F5,3,1X,F5,3,1X,F5,3,1X,F6,3)
4500 FORMAT(10X,'D',5X,'FBM',4X,'R1',4X,'ISRP2',4X,'DD',6X,'FF',
7X,'ISIG',4X,'ISTIG',4X,'BLSF',6X,'BLEC',
9X,'(1)',4X,'(2)',4X,'(3)',4X,'(4)',5X,'(5)',5X,'(6)',7X,'(7)',
6X,'(8)',5X,'(9)',6X,'(10)')
4000 FORMAT(14,2X,F8,6,1X,F4,2,1X,F5,2,3X,F7,3,1X,
F5,3,3X,F7,3,2X,F8,3,1X,F6,3,1X,F8,5,1X,F8,3)
5500 FORMAT(10X,'AE',5X,'W',6X,'Z',7X,'F11',7X,'F12',
6X,'IST',5X,'BMLSF',4X,'BMREC',5X,'TBLSL',
9X,'(12)',3X,'(13)',2X,'(14)',5X,'(15)',6X,'(16)',6X,'(17)',
4X,'(18)',5X,'(19)',7X,'(20)')
5000 FORMAT(14,2X,F8,6,1X,F4,2,1X,F5,2,1X,F9,4,1X,F9,4,1X,
F8,3,1X,F7,3,1X,F9,3,1X,F10,3)
6000 FORMAT(14,5F10,3)
WRITE(*,*)'FILE: SEDIM3 (RESULTS OF EINSTEIN MODEL)'
WRITE(*,*)'FILE: SEDIM4 (OUTPUT FOR PLOTTING)'
END

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PROGRAM PLSSEDIM
DIMENSION H(200), G(200), B(200), TB(200), S(200), TS(200), TBS(200)
, G1(200), B1(200), TB1(200), S1(200), TS1(200), TBS1(200)
OPEN (1, FILE='SEDPL10')
C*****
C
C   PROGRAM FOR PLOTTING THE BED LOAD SEDIMENT DISCHARGE
C   AND SUSPENDED SEDIMENT DISCHARGE
C*****
N=52
N1=13
N2=N1+1
N3=N2+N1-1
N4=N3+1
N5=N4+N1-1
N6=N5+1
DO 73 I=1, N
READ(1, *) H(I), G1(I), B1(I), TB1(I), S1(I), TS1(I), TBS1(I)
G(I)=ALOG10(G1(I))
IF(B1(I).EQ.0.) THEN
B(I)=-4.
ELSE IF(B1(I).NE.0.) THEN
B(I)=ALOG10(B1(I))
ENDIF
IF(S1(I).EQ.0.) THEN
S(I)=-4.
ELSE IF(S1(I).NE.0.) THEN
S(I)=ALOG10(S1(I))
ENDIF
IF(TS1(I).EQ.0.) THEN
TS(I)=-4.
ELSE IF(TS1(I).NE.0.) THEN
TS(I)=ALOG10(TS1(I))
ENDIF
IF(TB1(I).EQ.0.) THEN
TB(I)=-4.
ELSE IF(TB1(I).NE.0.) THEN
TB(I)=ALOG10(TB1(I))
ENDIF
73 CONTINUE
CALL GROPEN
CALL PLSIZE(12, 20.)
CALL OPTION('LX, LY')
CALL BOUNDS(3, -3, 4, 3, 4.)
CALL YLABEL('GB... (TON./DAY)')
CALL XLABEL('DISCHARGE... (CUMEC)')
CALL UPDCMT('BED LOAD SEDIMENT TRANSPORT')
CALL LOWCMT('S_SECTION... M_ATEBA_ S_UD_ K_APITA')
DO 36 I=1, N1
CALL CUTYPE('AS, SQ')
CALL DRAW(G(I), B(I))
36 CONTINUE
CALL CUNEXT
DO 46 I=N2, N3
CALL CUTYPE('SQ, SO')
CALL DRAW(G(I), B(I))
46 CONTINUE
CALL CUNEXT
DO 56 I=N4, N5
CALL CUTYPE('DE, SO')
CALL DRAW(G(I), B(I))
56 CONTINUE
CALL CUNEXT
DO 66 I=N6, N
CALL CUTYPE('CI, SQ')
CALL DRAW(G(I), B(I))
66 CONTINUE
CALL CUNEXT
DO 76 I=N6, N
CALL CUTYPE('GR, SO')
CALL DRAW(G(I), TB(I))
76 CONTINUE
CALL CUNEXT
CALL DRAWCU(3, 1, 1, 4, 1, 1, 1, 'AS')
CALL ADDCMT(3, 15, 4, 1, 'D=. 000212 _MM')
CALL DRAWCU(3, 1, 1, 3, 8, 1, 1, 'SQ')
CALL ADDCMT(3, 15, 3, 8, 'D=. 000254 _MM')
CALL DRAWCU(3, 1, 1, 3, 5, 1, 1, 'DE')
CALL ADDCMT(3, 15, 3, 5, 'D=. 000312 _MM')
CALL DRAWCU(3, 1, 1, 3, 2, 1, 1, 'CI')
CALL ADDCMT(3, 15, 3, 2, 'D=. 000370 _MM')
CALL DRAWCU(3, 1, 1, 2, 9, 1, 1, 'CR')
CALL ADDCMT(3, 15, 2, 9, 'T_DTAL_ B_ED_ L_LOAD')
CALL GRNEXT
CALL PLSIZE(12, 20.)
CALL OPTION('LX, LY')

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CALL BOUNDS(3., 3., 4.3, 4.)
CALL YLABEL('QS... (TON./DAY)')
CALL XLABEL('DISCHARGE... (CUMEC)')
CALL UPFCMT('SUSPENDED LOAD SEDIMENT TRANSPORT')
CALL LOWCMT('SECTION... M_ATEBA_S_UD_K_APITA')
DO 86 I=1,N1
CALL CUTYPE('AS,SO')
CALL DRAW(G(I),S(I))
86 CONTINUE
CALL CUNEXT
DO 96 I=N2,N3
CALL CUTYPE('SG,SO')
CALL DRAW(G(I),S(I))
96 CONTINUE
CALL CUNEXT
DO 97 I=N4,N5
CALL CUTYPE('DE,SO')
CALL DRAW(G(I),S(I))
97 CONTINUE
CALL CUNEXT
DO 98 I=N6,N
CALL CUTYPE('CI,SO')
CALL DRAW(G(I),S(I))
98 CONTINUE
CALL CUNEXT
DO 99 I=N6,N
CALL CUTYPE('CR,SO')
CALL DRAW(G(I),TS(I))
99 CONTINUE
CALL CUNEXT
CALL DRAWCU(3.1,1,4.1,1,1,1,'AS')
CALL ADDCMT(3.15,4.1,'D=,000212 _MM')
CALL DRAWCU(3.1,1,3.8,1,1,1,'SG')
CALL ADDCMT(3.15,3.8,'D=,000254 _MM')
CALL DRAWCU(3.1,1,3.5,1,1,1,'DE')
CALL ADDCMT(3.15,3.5,'D=,000312 _MM')
CALL DRAWCU(3.1,1,3.2,1,1,1,'CI')
CALL ADDCMT(3.15,3.2,'D=,000370 _MM')
CALL DRAWCU(3.1,1,2.9,1,1,1,'CR')
CALL ADDCMT(3.15,2.9,'TOTAL_S_USPENDED_LOAD')
CALL GRCLDS
END

```











CROSS SECTION: MONT OISEAUX 1  
HYDRAULICS CALCULATIONS...

R1 (1)	U1 (2)	DELTA1 (3)	H (4)	X1 (5)	DELTA2 (6)	U (7)	Q (8)	R2 (9)	U2 (10)	R2 (11)	Q (12)
.15	.0097	.00101	.323	1.056	.00031	.21	26.733	5.50	.036	2.11	2.26
.30	.0137	.00071	.457	1.314	.00025	.33	13.466	7.64	.043	2.96	2.26
.45	.0168	.00058	.560	1.442	.00023	.42	8.778	9.97	.047	3.55	4.00
.60	.0194	.00050	.647	1.514	.00021	.51	6.733	10.11	.050	3.97	4.39
.75	.0217	.00045	.723	1.557	.00021	.58	5.387	11.17	.052	4.27	5.02
.90	.0238	.00041	.792	1.578	.00021	.65	4.489	12.27	.053	4.42	5.32
1.05	.0257	.00038	.855	1.597	.00020	.71	3.848	13.27	.053	4.53	5.58
1.20	.0274	.00036	.914	1.612	.00020	.77	3.367	14.33	.054	4.57	5.77
1.35	.0291	.00034	.970	1.617	.00020	.82	2.993	15.39	.054	4.61	5.94
1.50	.0307	.00032	1.023	1.618	.00020	.88	2.693	16.41	.053	4.53	6.03
1.65	.0322	.00030	1.072	1.612	.00020	.93	2.448	17.48	.053	4.46	6.11
1.80	.0336	.00029	1.120	1.608	.00020	.97	2.244	18.52	.053	4.40	6.20
1.95	.0350	.00028	1.166	1.603	.00020	1.02	2.072	19.54	.052	4.35	6.30

CROSS SECTION: MONT OISEAUX 2  
HYDRAULICS CALCULATIONS... (CONTINUED)

UC (13)	H (14)	Q (15)	F (16)	V (17)	Q1 (18)	Q2 (19)	Y (20)	HY (21)	HE (22)	FE (23)
.038	4.18	439.3	192.2	323.4	92.4	.001397	.263	1.683	.371	12.929
.045	4.18	890.7	238.0	732.8	292.3	.000988	.400	1.427	.577	13.536
.050	7.45	1300.8	300.4	1140.9	591.8	.000807	.510	1.579	.422	13.842
.054	8.82	1872.9	334.4	1492.7	846.7	.000699	.596	1.538	.445	14.034
.056	9.67	1979.7	374.6	1814.5	1147.2	.000625	.667	1.502	.466	14.154
.058	10.27	2217.5	415.2	2067.5	1433.4	.000570	.723	1.468	.488	14.228
.059	10.79	2438.2	445.9	2299.9	1728.3	.000528	.768	1.439	.507	14.289
.060	11.16	2605.7	467.9	2484.7	2000.0	.000494	.806	1.414	.523	14.331
.061	11.52	2788.0	490.2	2696.6	2294.4	.000466	.828	1.390	.544	14.368
.062	11.68	2862.6	499.3	2783.3	2506.3	.000442	.842	1.368	.562	14.381
.062	11.83	2941.6	509.0	2875.1	2723.0	.000421	.847	1.345	.581	14.392
.062	12.02	3027.5	519.4	2976.4	2948.8	.000403	.848	1.325	.597	14.403
.063	12.21	3124.4	530.4	3104.3	3188.3	.000388	.843	1.307	.616	14.416



CROSS SECTION: AMONT DISEAUX 2  
HYDRAULICS CALCULATIONS...

R1 (1)	U1 (2)	DELTA1 (3)	H (4)	X1 (5)	DELTA2 (6)	U (7)	C (8)	M2 (9)	U2 (10)	R2 (11)	R (12)
.15	.0102	.00095	.341	1.095	.00030	.22	24.277	6.04	.037	1.96	2.11
.30	.0148	.00067	.482	1.352	.00024	.35	12.139	7.96	.044	2.74	3.04
.45	.0177	.00055	.590	1.471	.00022	.45	8.092	9.38	.048	3.27	3.72
.60	.0204	.00048	.681	1.538	.00021	.53	6.069	10.55	.051	3.68	4.28
.75	.0229	.00043	.761	1.549	.00021	.61	4.855	11.79	.052	3.85	4.60
.90	.0250	.00039	.834	1.591	.00020	.68	4.046	12.92	.053	3.99	4.89
1.05	.0270	.00036	.901	1.610	.00020	.75	3.468	14.09	.053	4.04	5.09
1.20	.0289	.00034	.963	1.616	.00020	.81	3.035	15.20	.053	4.06	5.26
1.35	.0307	.00032	1.022	1.618	.00020	.87	2.697	16.39	.053	4.01	5.36
1.50	.0323	.00030	1.077	1.612	.00020	.92	2.428	17.58	.052	3.95	5.45
1.65	.0339	.00029	1.129	1.607	.00020	.97	2.207	18.73	.052	3.89	5.54

CROSS SECTION: AMONT DISEAUX 2  
HYDRAULICS CALCULATIONS... (CONTINUED)

UC (13)	H (14)	A (15)	P (16)	Q (17)	G1 (18)	I2 (19)	Y (20)	BY (21)	BB (22)	PE (23)
.038	3.71	2427.8	1144.3	2260.2	541.1	.001327	.280	1.676	.374	12.843
.046	5.19	4731.6	1882.4	4634.0	1645.9	.000938	.426	1.617	.402	13.390
.051	6.26	6809.6	1979.6	6906.3	3048.5	.000766	.539	1.565	.429	13.662
.055	7.14	8377.2	2038.8	8894.8	4579.5	.000663	.631	1.522	.454	13.838
.057	7.65	9654.4	2105.3	10127.8	5896.8	.000593	.701	1.482	.478	13.928
.058	8.11	10616.1	2146.5	11238.8	7233.3	.000542	.753	1.449	.501	13.999
.060	8.42	11289.2	2174.3	12113.7	8433.0	.000501	.800	1.420	.521	14.049
.061	8.70	11903.6	2199.8	12894.1	9626.3	.000469	.826	1.393	.542	14.086
.061	8.86	12247.9	2214.1	13331.3	10616.7	.000442	.842	1.368	.562	14.104
.062	8.99	12536.5	2226.0	13697.9	11557.7	.000420	.848	1.344	.582	14.116
.062	9.13	12854.8	2238.9	14113.2	12329.7	.000400	.847	1.321	.602	14.128

17. 31. 38. UCLP, 58. DEFTERM, 0.064MLNS.

CROSS SECTION: AMONT DISEAUX 2  
SEDIMENT TRANSPORT CALCULATION..

Table with 11 columns: B (11), FBH (12), R1 (13), 188P2 (14), DD (15), FF (16), 1818 (17), 1819 (18), 1819 (19), 1819 (10), TBLT (11). Rows contain numerical data for various sediment transport calculations.

CROSS SECTION: AMONT DISEAUX 2  
SEDIMENT TRANSPORT CALCULATION.. (CONTINUED)

Table with 10 columns: AX (12), M (13), T (14), F11 (15), F12 (16), 181 (17), 1819 (18), 1819 (19), 1819 (20), TBLT (21). Rows contain numerical data for various sediment transport calculations.



CROSS SECTION: DISEAUX NORD 1  
HYDRAULICS CALCULATION S..

H1 (1)	U1 (2)	DELTA1 (3)	H (4)	K1 (5)	DELTA2 (6)	U (7)	S (8)	H2 (9)	U2 (10)	H2 (11)	R (12)
.25	.0077	.00100	.324	1.058	.00031	.22	26.793	3.81	.037	3.92	4.17
.50	.0138	.00071	.458	1.315	.00025	.33	13.397	7.65	.045	8.48	5.75
.75	.0167	.00058	.561	1.443	.00023	.42	8.931	9.01	.050	6.50	7.25
1.00	.0178	.00050	.648	1.513	.00021	.52	6.598	10.13	.053	7.29	8.29
1.25	.0218	.00045	.723	1.558	.00021	.61	5.357	11.22	.054	7.77	9.02
1.50	.0238	.00041	.794	1.578	.00021	.68	4.466	12.30	.055	8.04	9.54
1.75	.0257	.00038	.858	1.598	.00020	.74	3.828	13.33	.056	8.22	9.97
2.00	.0275	.00035	.917	1.612	.00020	.80	3.349	14.37	.056	8.28	10.28
2.25	.0292	.00033	.972	1.617	.00020	.86	2.977	15.36	.056	8.33	10.58
2.50	.0308	.00032	1.025	1.617	.00020	.92	2.677	16.47	.056	8.19	10.67
2.75	.0323	.00030	1.075	1.612	.00020	.97	2.436	17.54	.055	8.06	10.81
3.00	.0337	.00029	1.123	1.607	.00020	1.02	2.233	18.57	.055	7.94	10.94
3.25	.0351	.00028	1.169	1.603	.00020	1.07	2.061	19.60	.054	7.84	11.09

CROSS SECTION: DISEAUX NORD 1  
HYDRAULICS CALCULATION S.. (CONTINUED)

UC (13)	H (14)	R (15)	F (16)	d (17)	Q1 (18)	K2 (19)	Y (20)	HX (21)	SH (22)	PE (23)
.040	6.31	1111.0	237.5	1270.0	248.4	.001374	.264	1.682	.371	13.371
.047	9.38	1864.7	283.3	2130.2	448.4	.000985	.401	1.626	.378	13.933
.052	11.48	2502.2	370.5	2854.7	1118.1	.000805	.511	1.578	.422	14.249
.056	13.15	3177.8	428.9	3721.2	1691.4	.000697	.598	1.537	.448	14.434
.058	14.34	3700.7	441.8	4271.5	2252.9	.000623	.671	1.501	.467	14.548
.060	15.17	4068.3	443.9	4662.8	2760.4	.000567	.725	1.467	.489	14.618
.061	15.87	4378.6	445.7	4971.2	3258.8	.000527	.770	1.437	.508	14.675
.062	16.36	4598.8	446.9	5223.2	3700.6	.000493	.807	1.413	.526	14.714
.063	16.85	4817.4	448.1	5433.4	4154.2	.000465	.829	1.389	.545	14.747
.064	17.03	4897.6	448.6	5538.7	4491.5	.000441	.843	1.366	.563	14.758
.064	17.22	4983.2	449.1	5637.7	4832.0	.000420	.848	1.344	.582	14.766
.064	17.44	5081.8	449.6	5746.8	5180.5	.000402	.848	1.324	.600	14.775
.065	17.67	5185.7	450.2	5864.2	5537.4	.000387	.843	1.306	.617	14.786

CROSS SECTION: OISEAUX NORD 1

SEDIMENT TRANSPORT CALCULATION..

Table with 11 columns: D (1), FRI (2), RI (3), IRRP2 (4), DR (5), RP (6), S110 (7), S210 (8), SLP (9), SLEC (10), TLEC (11). Rows contain numerical data for various sediment transport calculations.

CROSS SECTION: OISEAUX NORD 1

SEDIMENT TRANSPORT CALCULATION.. (CONTINUED)

Table with 11 columns: RI (12), M (13), I (14), FT1 (15), FT2 (16), IST (17), SLEP (18), SREC (19), SRRAS (20), TRLM (21). Rows contain numerical data for sediment transport calculations, continuing from the previous table.

CROSS SECTION: DISEAUX NORD 2  
HYDRAULICS CALCULATIONS...

R1 (1)	U1 (2)	DELTA1 (3)	H (4)	X1 (5)	DELTA2 (6)	U (7)	C (8)	H2 (9)	U2 (10)	R2 (11)	R (12)
.15	.0108	.00090	.341	1.137	.00029	.24	21.546	6.33	.038	1.79	1.94
.30	.0153	.00064	.511	1.392	.00023	.37	10.773	8.35	.044	2.51	2.81
.45	.0188	.00052	.626	1.499	.00022	.48	7.182	9.85	.048	2.98	3.43
.60	.0217	.00043	.723	1.557	.00021	.57	5.387	11.19	.051	3.28	3.88
.75	.0243	.00040	.808	1.583	.00021	.65	4.309	12.32	.052	3.42	4.17
.90	.0266	.00037	.885	1.606	.00020	.72	3.591	13.32	.052	3.56	4.40
1.05	.0287	.00034	.956	1.616	.00020	.79	3.078	13.08	.053	3.53	4.58
1.20	.0307	.00032	1.022	1.618	.00020	.86	2.693	12.41	.052	3.49	4.69
1.35	.0325	.00030	1.084	1.611	.00020	.92	2.394	17.75	.052	3.42	4.77

CROSS SECTION: DISEAUX NORD 2  
HYDRAULICS CALCULATIONS... (CONTINUED)

UC (13)	H (14)	A (15)	F (16)	S (17)	G1 (18)	X2 (19)	Y (20)	BX (21)	BS (22)	PE (23)
.039	4.27	1480.1	780.0	1348.9	351.8	.001230	.301	1.667	.078	13.025
.047	5.98	3298.5	1243.8	3131.9	1221.4	.000884	.438	1.603	.409	13.361
.052	7.20	5299.8	1796.3	5132.0	2523.3	.000722	.576	1.348	.439	13.821
.058	8.08	6910.5	1862.9	6826.9	3921.0	.000625	.669	1.302	.466	13.975
.057	8.67	8014.4	1907.1	8049.3	5203.6	.000559	.739	1.460	.493	14.061
.059	9.11	8867.4	1940.3	8999.9	6418.9	.000510	.790	1.427	.516	14.125
.060	9.47	9566.1	1967.4	9819.9	7589.4	.000472	.823	1.396	.539	14.170
.061	9.68	9988.0	1983.6	10314.9	8574.3	.000442	.842	1.368	.562	14.194
.061	9.85	10322.2	1996.4	10707.1	9494.2	.000417	.848	1.340	.585	14.207

17.45.27.UCLP. 38. DEPTERH. 0.064KLNS.



CROSS SECTION: BARRAJE 1  
HYDRAULICS CALCULATIONS...

R1 (1)	U1 (2)	DELTA1 (3)	H (4)	X1 (5)	DELTA2 (6)	U (7)	C (8)	M2 (9)	U2 (10)	R2 (11)	R (12)
.20	.0102	.00096	.340	1.093	.00030	.23	24.392	6.03	.038	2.79	2.79
.40	.0144	.00068	.480	1.350	.00024	.36	12.196	7.99	.049	3.89	4.29
.60	.0177	.00058	.588	1.469	.00022	.46	8.131	9.36	.049	4.63	5.23
.80	.0204	.00048	.679	1.536	.00021	.53	6.098	10.53	.052	5.19	5.79
1.00	.0228	.00043	.760	1.568	.00021	.63	4.878	11.76	.053	5.44	6.44
1.20	.0250	.00039	.832	1.590	.00020	.70	4.065	12.89	.054	5.63	6.83
1.40	.0270	.00036	.899	1.610	.00020	.76	3.485	14.06	.054	5.69	7.09
1.60	.0288	.00034	.961	1.616	.00020	.83	3.049	15.16	.055	5.73	7.33
1.80	.0306	.00032	1.019	1.618	.00020	.89	2.710	16.34	.054	5.66	7.46
2.00	.0322	.00030	1.074	1.612	.00020	.94	2.439	17.53	.054	5.57	7.37
2.20	.0338	.00029	1.127	1.607	.00020	1.00	2.217	18.67	.053	5.48	7.68
2.40	.0353	.00028	1.177	1.602	.00020	1.05	2.033	19.78	.053	5.40	7.80

CROSS SECTION: BARRAJE 1  
HYDRAULICS CALCULATIONS... (CONTINUED)

UC (13)	H (14)	A (15)	F (16)	Q (17)	Q1 (18)	X2 (19)	Y (20)	BK (21)	BB (22)	BE (23)
.039	4.88	954.0	293.9	744.3	219.1	.001330	.279	1.676	.374	13.116
.047	7.41	1833.5	418.0	1663.7	663.1	.000940	.429	1.617	.402	13.745
.052	9.23	2701.1	508.3	2400.5	1240.5	.000768	.538	1.566	.429	14.050
.056	10.72	3511.9	582.0	2997.5	1921.9	.000665	.629	1.523	.433	14.244
.058	11.59	4085.1	678.8	3490.4	2536.1	.000595	.700	1.483	.478	14.342
.060	12.35	4629.6	738.9	3939.3	3230.0	.000543	.752	1.450	.500	14.419
.061	12.86	5007.9	742.3	4210.4	3830.0	.000503	.799	1.421	.520	14.472
.062	13.32	5350.1	743.3	4482.4	4427.1	.000470	.829	1.394	.541	14.511
.062	13.57	5541.3	746.9	4641.0	4913.9	.000443	.842	1.369	.561	14.531
.063	13.78	5692.3	748.2	4766.2	5367.5	.000421	.847	1.345	.581	14.543
.063	14.00	5837.6	749.7	4903.3	5838.4	.000401	.847	1.323	.601	14.555
.064	14.24	6035.4	731.3	5063.3	6327.9	.000384	.842	1.302	.620	14.569



CROSS SECTION: BARRAJE 2  
HYDRAULICS CALCULATIONS..

R1 (1)	U1 (2)	DELTA1 (3)	H (4)	X1 (5)	DELTA2 (6)	U (7)	C (8)	H2 (9)	U2 (10)	R2 (11)	H (12)
.15	.0100	.00098	.333	1.079	.00030	.22	25.349	5.94	.037	2.02	2.17
.30	.0141	.00069	.471	1.336	.00024	.34	12.674	7.83	.043	2.83	3.13
.45	.0173	.00056	.577	1.499	.00022	.44	8.490	9.21	.048	3.38	3.83
.60	.0209	.00049	.666	1.528	.00021	.52	6.337	10.37	.050	3.80	4.40
.75	.0224	.00044	.745	1.544	.00021	.60	5.070	11.54	.052	4.02	4.77
.90	.0245	.00040	.816	1.555	.00021	.67	4.223	12.65	.053	4.16	5.06
1.05	.0265	.00037	.882	1.605	.00020	.73	3.621	13.75	.053	4.24	5.29
1.20	.0283	.00034	.943	1.614	.00020	.79	3.169	14.83	.053	4.27	5.47
1.35	.0300	.00033	1.000	1.620	.00020	.85	2.817	15.93	.053	4.25	5.60
1.50	.0316	.00031	1.054	1.614	.00020	.90	2.535	17.09	.053	4.18	5.68
1.65	.0332	.00029	1.105	1.609	.00020	.95	2.304	18.20	.052	4.12	5.77

CROSS SECTION: BARRAJE 2  
HYDRAULICS CALCULATIONS.. (CONTINUED)

UC (13)	H (14)	A (15)	P (16)	Q (17)	Q1 (18)	X2 (19)	Y (20)	BX (21)	BB (22)	PE (23)
.038	5.63	794.3	293.4	294.0	173.0	.001356	.273	1.679	.373	13.243
.046	8.11	1826.3	878.6	925.2	621.0	.000989	.415	1.621	.400	13.823
.051	9.92	3715.7	1316.9	2307.4	1626.6	.000783	.527	1.571	.426	14.114
.054	11.39	5817.9	1958.1	4317.2	3037.9	.000678	.616	1.529	.450	14.299
.056	12.34	7336.5	1713.7	6020.5	4384.1	.000606	.688	1.490	.473	14.402
.058	13.10	8646.3	1737.5	7386.4	5763.5	.000553	.741	1.457	.496	14.473
.059	13.66	9459.1	1792.4	8446.8	7060.8	.000512	.787	1.428	.515	14.530
.060	14.13	10498.8	1821.0	9335.6	8307.9	.000479	.817	1.402	.535	14.570
.061	14.48	11138.1	1842.3	10030.7	9449.7	.000452	.840	1.378	.554	14.597
.062	14.69	11516.0	1854.9	10441.5	10391.4	.000429	.845	1.354	.574	14.608
.062	14.91	11927.4	1868.6	10888.8	11379.0	.000409	.849	1.331	.593	14.620

18.00.45.UCLP. 58. DEPTERM. 0.064MLNS.

CROSS SECTION: BARRAJE 2  
SEDIMENT TRANSPORT CALCULATION.

Table with 11 columns: (1) S, (2) FRI, (3) F1, (4) ISSP2, (5) DP, (6) FF, (7) IS10, (8) IS10, (9) BLSP, (10) BLEC, (11) TRES. Rows include numerical data for various sediment transport calculations.

CROSS SECTION: BARRAJE 2

SEDIMENT TRANSPORT CALCULATION. (CONTINUED)

Table with 11 columns: (12) S, (13) FRI, (14) F1, (15) F12, (16) IS1, (17) IS1, (18) BLSP, (19) BLEC, (20) BRES, (21) TRES. Rows include numerical data for various sediment transport calculations.

18. 04. 24. UCLP. 38. DEPTERM. 0. 123ALMS.



CROSS SECTION: MATEBA AMONT BUD 1  
HYDRAULICS CALCULATIONS...

H1 (1)	H1 (2)	DELTA1 (3)	H (4)	K1 (5)	DELTA2 (6)	U (7)	C (8)	H2 (9)	U2 (10)	H2 (11)	H (12)
.15	.0099	.00099	.328	1.028	.00030	.21	26.117	5.87	.037	2.06	2.21
.30	.0139	.00070	.464	1.328	.00025	.33	15.858	7.73	.043	2.89	3.19
.45	.0171	.00057	.569	1.491	.00022	.43	8.704	9.10	.047	3.46	3.91
.60	.0197	.00049	.657	1.521	.00021	.51	4.529	10.24	.050	3.89	4.49
.75	.0220	.00044	.734	1.541	.00021	.59	3.223	11.37	.052	4.14	4.89
.90	.0241	.00040	.804	1.551	.00021	.66	4.353	12.46	.053	4.29	5.19
1.05	.0261	.00037	.869	1.561	.00020	.72	3.731	13.52	.053	4.38	5.42
1.20	.0279	.00035	.929	1.563	.00020	.78	3.265	14.58	.053	4.41	5.61
1.35	.0294	.00033	.983	1.569	.00020	.84	2.902	15.62	.054	4.42	5.77
1.50	.0312	.00031	1.038	1.576	.00020	.89	2.612	16.75	.052	4.39	5.85

CROSS SECTION: MATEBA AMONT BUD 1  
HYDRAULICS CALCULATIONS... (CONTINUED)

UC (13)	H (14)	S (15)	P (16)	Q (17)	Q1 (18)	X2 (19)	Y (20)	SE (21)	SE (22)	PE (23)
.038	4.39	909.7	431.2	560.9	194.9	.001376	.248	1.481	.372	12.984
.045	5.73	1832.7	762.8	1136.1	413.7	.000973	.408	1.624	.397	13.304
.050	7.07	2721.3	793.9	1689.6	1178.0	.000924	.518	1.573	.424	13.764
.054	7.98	3454.4	810.9	2133.2	1776.3	.000688	.606	1.533	.447	13.938
.056	8.60	3966.4	822.3	2481.3	2334.8	.000613	.678	1.496	.470	14.039
.058	9.07	4352.3	831.2	2731.7	2857.7	.000562	.732	1.462	.492	14.104
.059	9.45	4648.1	838.1	2957.1	3361.1	.000520	.778	1.434	.511	14.139
.060	9.74	4910.2	843.4	3130.6	3827.7	.000486	.812	1.408	.530	14.194
.061	9.99	5121.7	848.1	3280.9	4280.6	.000459	.834	1.384	.547	14.223
.062	10.11	5226.3	850.4	3361.0	4646.3	.000435	.844	1.360	.568	14.236

18. 10. 13. UCLP. 58. DEPTERM. 0. 064KLNS.



CROSS SECTION: MATEBA AMONT BUD 2  
HYDRAULICS CALCULATIONS..

R1 (1)	U1 (2)	DELTA1 (3)	H (4)	X1 (5)	DELTA2 (6)	U (7)	C (8)	M2 (9)	U (10)	R2 (11)	R (12)
.15	.0099	.00099	.328	1.068	.00030	.21	26.117	5.87	.037	2.06	2.21
.30	.0139	.00070	.464	1.325	.00025	.33	13.058	7.73	.043	2.89	3.19
.45	.0171	.00057	.569	1.451	.00022	.43	8.706	9.10	.047	3.46	3.91
.60	.0197	.00049	.657	1.521	.00021	.51	6.529	10.24	.050	3.89	4.49
.75	.0220	.00044	.734	1.561	.00021	.59	5.223	11.37	.052	4.14	4.89
.90	.0241	.00040	.804	1.581	.00021	.66	4.353	12.46	.053	4.29	5.19
1.05	.0261	.00037	.867	1.601	.00020	.72	3.731	13.52	.053	4.38	5.43
1.20	.0279	.00035	.929	1.613	.00020	.78	3.265	14.58	.053	4.41	5.61
1.35	.0296	.00033	.985	1.619	.00020	.84	2.902	15.62	.054	4.42	5.77
1.50	.0312	.00031	1.038	1.616	.00020	.89	2.612	16.75	.053	4.35	5.85
1.65	.0327	.00030	1.089	1.611	.00020	.94	2.374	17.84	.053	4.29	5.94

CROSS SECTION: MATEBA AMONT BUD 2  
HYDRAULICS CALCULATIONS.. (CONTINUED)

UC (13)	H (14)	A (15)	P (16)	Q (17)	Q1 (18)	K2 (19)	Y (20)	BX (21)	BB (22)	PE (23)
.038	3.42	1987.0	880.5	1722.2	425.8	.001376	.268	1.681	.372	12.736
.045	4.87	3459.5	1161.5	3109.2	1157.8	.000973	.408	1.624	.399	13.307
.050	5.94	4748.3	1253.5	4466.4	2046.7	.000794	.518	1.575	.424	13.595
.054	6.80	5854.8	1310.8	5607.2	3010.6	.000688	.606	1.533	.447	13.778
.056	7.39	6639.4	1349.9	6430.5	3907.9	.000615	.678	1.496	.470	13.887
.058	7.83	7238.0	1379.2	7065.2	4752.2	.000562	.732	1.462	.492	13.958
.059	8.18	7729.8	1402.8	7599.1	5565.6	.000520	.778	1.434	.511	14.014
.060	8.45	8115.5	1420.8	8026.8	6326.4	.000486	.812	1.408	.530	14.054
.061	8.69	8492.4	1436.6	8400.4	7064.3	.000459	.834	1.384	.549	14.086
.062	8.80	8617.3	1444.3	8583.3	7661.2	.000435	.844	1.360	.568	14.097
.063	8.93	8794.6	1452.6	8779.9	8266.6	.000415	.849	1.338	.587	14.108

18. 11. 13. UCLP. 38. DEFTERM. 0.064KLNS.

CROSS SECTION: MATEBA AMONT BUD 2  
SEDIMENT TRANSPORT CALCULATION..

Table with 11 columns: D (1), FRI (2), S1 (3), ISTR2 (4), S2 (5), F7 (6), ISTR (7), ISTR (8), M.F. (9), SLEC (10), TALE (11). Rows contain numerical data for various sediment transport calculations.

CROSS SECTION: MATEBA AMONT BUD 2  
SEDIMENT TRANSPORT CALCULATION.. (CONTINUED)

Table with 11 columns: AK (12), H (13), S1 (14), F11 (15), F12 (16), F17 (17), M.F. (18), SLEC (19), SFRAS (20), TALE (21). Rows contain numerical data for various sediment transport calculations.

CROSS SECTION: MATEBA SUD MANDUDI  
HYDRAULICS CALCULATIONS...

R1 (1)	U1 (2)	DELTA1 (3)	M (4)	X1 (5)	DELTA2 (6)	U (7)	C (8)	M2 (9)	U2 (10)	R2 (11)	R (12)
.20	.0099	.00098	.331	1.074	.00030	.22	23.701	3.90	.038	2.90	3.10
.40	.0140	.00069	.468	1.331	.00024	.35	12.851	7.78	.045	4.04	4.44
.60	.0172	.00057	.573	1.455	.00022	.45	8.567	9.16	.049	4.82	5.42
.80	.0199	.00049	.662	1.525	.00021	.53	6.425	10.31	.052	5.41	6.21
1.00	.0222	.00044	.740	1.562	.00021	.61	5.140	11.46	.053	5.73	6.73
1.20	.0243	.00040	.811	1.583	.00021	.68	4.284	12.56	.054	5.93	7.13
1.40	.0263	.00037	.876	1.603	.00020	.74	3.672	13.65	.055	6.04	7.44
1.60	.0281	.00035	.936	1.614	.00020	.81	3.213	14.72	.055	6.08	7.68
1.80	.0298	.00033	.993	1.619	.00020	.86	2.836	15.79	.055	6.07	7.87
2.00	.0314	.00031	1.047	1.615	.00020	.92	2.570	16.93	.054	5.97	7.97
2.20	.0329	.00030	1.098	1.610	.00020	.97	2.336	18.04	.054	5.88	8.08

CROSS SECTION: MATEBA SUD MANDUDI  
HYDRAULICS CALCULATIONS... (CONTINUED)

UC (13)	H (14)	A (15)	F (16)	Q (17)	Q1 (18)	X2 (19)	Y (20)	BX (21)	BB (22)	FE (23)
.039	3.55	4001.6	1234.2	3348.9	893.3	.001365	.271	1.679	.373	12.781
.047	5.24	6254.9	1454.1	5338.1	2174.3	.000965	.411	1.622	.399	13.384
.052	6.47	8074.6	1504.4	7387.8	3609.3	.000768	.523	1.573	.425	13.684
.055	7.46	9585.3	1544.9	8955.3	5106.3	.000683	.612	1.531	.449	13.873
.058	8.11	10593.9	1571.4	10006.3	6455.5	.000610	.683	1.493	.472	13.981
.059	8.61	11388.7	1591.9	10840.0	7737.6	.000557	.737	1.459	.494	14.035
.061	8.99	11998.2	1607.6	11479.4	8936.0	.000516	.783	1.431	.513	14.111
.062	9.30	12488.9	1619.9	12060.3	10066.3	.000483	.815	1.405	.533	14.150
.062	9.54	12879.0	1629.8	12523.0	11126.8	.000455	.837	1.381	.551	14.179
.063	9.66	13078.4	1634.8	12759.6	12015.6	.000432	.845	1.357	.571	14.189
.063	9.80	13297.3	1640.3	13019.2	12913.8	.000412	.856	1.335	.590	14.200

18.21.31.UCLP, 58. DEFTERM, 0.064KLN5.



CROSS SECTION: MATERA SUD KAPITA  
HYDRAULIC CALCULATIONS...

SI (1)	U1 (2)	DELTA1 (3)	M (4)	SI (5)	DELTA2 (6)	U (7)	S (8)	M2 (9)	U2 (10)	RS (11)	H (12)
.20	.0099	.00098	.331	1.074	.00030	.22	23.701	2.90	.038	2.90	2.10
.40	.0140	.00069	.468	1.351	.00024	.35	12.851	7.78	.045	4.04	4.44
.60	.0172	.00057	.573	1.455	.00022	.48	8.267	9.16	.049	4.82	5.42
.80	.0199	.00049	.662	1.523	.00021	.53	6.425	10.31	.052	5.41	6.21
1.00	.0222	.00044	.740	1.562	.00021	.61	5.140	11.46	.053	5.73	6.73
1.20	.0242	.00040	.811	1.583	.00021	.68	4.384	12.56	.054	5.93	7.13
1.40	.0263	.00037	.875	1.603	.00020	.74	3.872	13.63	.055	6.04	7.44
1.60	.0281	.00035	.935	1.614	.00020	.81	3.213	14.72	.055	6.08	7.68
1.80	.0298	.00033	.993	1.619	.00020	.86	2.856	15.79	.055	6.07	7.87
2.00	.0314	.00031	1.047	1.613	.00020	.92	2.570	16.93	.054	5.97	7.97
2.20	.0329	.00030	1.098	1.610	.00020	.97	2.336	18.04	.054	5.88	8.08
2.40	.0344	.00028	1.146	1.603	.00020	1.02	2.142	19.11	.053	5.79	8.19
2.60	.0358	.00027	1.193	1.601	.00020	1.07	1.977	20.19	.053	5.69	8.29

CROSS SECTION: MATERA SUD KAPITA  
HYDRAULIC CALCULATIONS... (CONTINUED)

US (13)	H (14)	S (15)	F (16)	S (17)	Q1 (18)	M2 (19)	Y (20)	BX (21)	BS (22)	PE (23)
.039	9.08	982.6	258.4	1020.2	212.4	.001363	.271	1.679	.573	13.470
.047	10.00	2278.5	663.1	2489.5	792.5	.000965	.411	1.622	.399	14.030
.052	12.12	3852.0	823.2	4260.4	1726.3	.000788	.523	1.573	.425	14.312
.055	12.84	5347.4	901.6	5967.6	2848.8	.000683	.612	1.531	.449	14.491
.058	14.76	6583.8	947.2	7158.1	3890.1	.000610	.683	1.493	.472	14.594
.059	15.83	7322.2	987.4	8115.5	4906.8	.000557	.737	1.459	.494	14.663
.061	16.49	7896.6	1030.5	8904.5	5881.1	.000516	.783	1.431	.513	14.717
.062	17.01	8437.5	1068.0	9543.4	6800.7	.000483	.815	1.408	.533	14.755
.062	17.43	8996.3	1097.7	10113.5	7683.7	.000453	.837	1.381	.551	14.782
.063	17.66	9130.8	1112.8	10404.7	8388.8	.000432	.848	1.357	.571	14.792
.063	17.88	9388.2	1129.5	10724.8	9117.4	.000412	.850	1.333	.590	14.802
.064	18.13	9672.4	1147.9	11103.6	9880.7	.000394	.845	1.314	.608	14.813
.064	18.34	9981.6	1163.0	11456.6	10618.8	.000379	.841	1.296	.626	14.822





CROSS SECTION: MATEBA AMONT  
HYDRAULICS CALCULATIONS..

R1 (1)	U1 (2)	DELTA1 (3)	N (4)	T1 (5)	DELTA2 (6)	U (7)	C (8)	M2 (9)	U2 (10)	R2 (11)	N (12)
.10	.0085	.00113	.282	.938	.00034	.17	35.418	5.20	.033	1.58	1.43
.20	.0120	.00082	.397	1.212	.00027	.27	17.709	6.83	.040	2.21	2.41
.30	.0147	.00067	.488	1.362	.00024	.35	11.806	8.03	.044	2.67	2.77
.40	.0169	.00058	.564	1.444	.00022	.42	8.835	9.04	.047	3.03	3.48
.50	.0189	.00052	.630	1.502	.00022	.48	7.084	9.70	.049	3.34	3.84
.60	.0207	.00047	.691	1.544	.00021	.54	5.903	10.27	.051	3.59	4.19
.70	.0224	.00044	.746	1.564	.00021	.59	5.060	11.55	.051	3.70	4.40
.80	.0237	.00041	.797	1.577	.00021	.64	4.427	12.33	.052	3.77	4.57
.90	.0234	.00038	.846	1.594	.00020	.67	3.935	13.12	.053	3.87	4.77
1.00	.0248	.00036	.892	1.608	.00020	.74	3.542	13.73	.053	3.90	4.90
1.10	.0281	.00035	.935	1.614	.00020	.78	3.220	14.70	.053	3.92	5.02

CROSS SECTION: MATEBA AMONT  
HYDRAULICS CALCULATIONS.. (CONTINUED)

UC (13)	H (14)	A (15)	P (16)	S (17)	Q1 (18)	Q2 (19)	Y (20)	BX (21)	BB (22)	PE (23)
.034	3.05	683.0	391.8	814.1	118.2	.001602	.223	1.498	.365	12.507
.042	4.41	1337.7	558.2	1136.2	364.7	.001183	.337	1.681	.386	13.118
.046	5.44	1977.4	685.6	1733.9	698.7	.000923	.433	1.814	.404	13.443
.050	6.27	2581.7	769.1	2345.7	1070.2	.000801	.514	1.877	.423	13.646
.052	6.97	3137.8	826.2	2895.6	1820.2	.000717	.580	1.845	.440	13.791
.055	7.59	3670.4	873.7	3404.3	1987.8	.000654	.640	1.818	.456	13.903
.056	7.97	4000.3	906.6	3718.7	2377.4	.000606	.688	1.490	.474	13.965
.057	8.32	4332.8	935.2	4075.0	2787.7	.000567	.727	1.465	.490	14.018
.058	8.64	4637.7	961.1	4403.1	3204.7	.000534	.762	1.444	.504	14.063
.059	8.87	4852.5	979.4	4637.6	3571.4	.000507	.794	1.424	.518	14.099
.060	9.08	5061.0	996.8	4869.3	3941.8	.000483	.814	1.405	.532	14.127



CROSS SECTION: MATEBA CENTRAL  
HYDRAULICS CALCULATIONS...

R1 (1)	U1 (2)	DELTA1 (3)	H (4)	X1 (5)	DELTA2 (6)	U (7)	C (8)	H2 (9)	U2 (10)	H2 (11)	R (12)
.15	.0088	.00110	.294	.984	.00033	.19	32.523	5.38	.05	2.41	2.56
.30	.0125	.00078	.416	1.244	.00026	.30	16.261	7.09	.042	3.40	3.70
.45	.0193	.00064	.510	1.390	.00023	.38	10.841	8.33	.046	4.10	4.35
.60	.0177	.00055	.588	1.469	.00022	.46	8.131	9.36	.049	4.63	5.23
.75	.0197	.00049	.638	1.522	.00021	.53	6.505	10.26	.051	5.06	5.81
.90	.0216	.00045	.721	1.557	.00021	.59	5.420	11.16	.053	5.33	6.23
1.05	.0234	.00042	.778	1.574	.00021	.64	4.644	12.06	.053	5.49	6.54
1.20	.0250	.00039	.832	1.590	.00020	.70	4.065	12.89	.054	5.63	6.83
1.35	.0265	.00037	.883	1.605	.00020	.75	3.614	13.77	.054	5.68	7.03
1.50	.0279	.00035	.930	1.613	.00020	.80	3.232	14.62	.055	5.71	7.21
1.65	.0293	.00033	.976	1.618	.00020	.84	2.937	15.43	.055	5.74	7.39
1.80	.0306	.00032	1.019	1.618	.00020	.89	2.710	16.24	.054	5.64	7.46
1.95	.0318	.00031	1.061	1.614	.00020	.93	2.502	17.24	.054	5.59	7.54
2.10	.0330	.00030	1.101	1.609	.00020	.97	2.323	18.11	.054	5.52	7.62
2.25	.0342	.00029	1.139	1.606	.00020	1.01	2.168	18.95	.053	5.46	7.71

CROSS SECTION: MATEBA CENTRAL  
HYDRAULICS CALCULATIONS... (CONTINUED)

UC (13)	H (14)	A (15)	F (16)	Q (17)	Q1 (18)	Q2 (19)	Y (20)	EX (21)	BE (22)	PE (23)
.036	4.17	785.1	293.0	799.9	149.4	.001336	.233	1.696	.366	12.856
.044	4.06	1446.0	399.1	1380.0	430.8	.001086	.357	1.644	.389	13.462
.049	7.48	2057.6	436.6	2327.2	791.4	.000887	.456	1.604	.408	13.783
.052	8.60	2592.3	495.7	2996.8	1190.6	.000768	.538	1.566	.429	13.978
.055	9.57	3088.3	529.4	3621.6	1625.1	.000687	.608	1.533	.448	14.120
.057	10.26	3464.9	593.7	4094.8	2035.7	.000627	.667	1.503	.466	14.213
.058	10.77	3792.0	571.5	4454.3	2417.1	.000580	.713	1.474	.484	14.273
.060	11.23	4030.5	588.2	4806.0	2812.0	.000543	.752	1.450	.500	14.327
.060	11.59	4229.5	599.8	5058.9	3165.3	.000512	.788	1.428	.515	14.369
.061	11.89	4408.6	610.2	5286.7	3511.8	.000486	.812	1.407	.531	14.396
.062	12.16	4588.0	620.4	5546.2	3866.0	.000463	.831	1.388	.546	14.423
.062	12.30	4664.1	624.6	5664.2	4135.9	.000443	.842	1.369	.561	14.433
.063	12.43	4744.1	629.1	5788.3	4407.8	.000426	.846	1.350	.576	14.441
.063	12.57	4830.1	633.9	5921.8	4683.7	.000410	.850	1.333	.591	14.449
.063	12.71	4921.3	638.9	6063.2	4969.8	.000396	.846	1.317	.606	14.458

CROSS SECTION: NATEVA CENTRAL  
SEDIMENT TRANSPORT CALCULATION...

Table with 11 columns: (1), (2), (3), (4), (5), (6), (7), (8), (9), (10), (11). Multiple rows of numerical data.

CROSS SECTION: NATEVA CENTRAL

SEDIMENT TRANSPORT CALCULATION... (CONTINUED)

Table with 11 columns: (1), (2), (3), (4), (5), (6), (7), (8), (9), (10), (11). Multiple rows of numerical data.

CROSS SECTION: MATEBA AVAL  
HYDRAULICS CALCULATIONS..

R1 (1)	U1 (2)	DELTA1 (3)	H (4)	X1 (5)	DELTA2 (6)	U (7)	C (8)	H2 (9)	U2 (10)	R2 (11)	R (12)
.15	.0091	.00107	.302	1.007	.00032	.20	30.780	5.50	.036	2.32	2.47
.30	.0128	.00076	.428	1.264	.00026	.31	13.390	7.24	.042	3.27	3.57
.45	.0157	.00062	.524	1.405	.00023	.40	10.260	8.51	.046	3.94	4.39
.60	.0182	.00054	.603	1.484	.00022	.47	7.499	9.57	.049	4.43	5.03
.75	.0203	.00048	.676	1.534	.00021	.54	6.154	10.47	.052	4.85	5.60
.90	.0222	.00044	.741	1.563	.00021	.60	5.130	11.47	.053	5.05	5.95
1.05	.0240	.00041	.800	1.580	.00021	.66	4.377	12.37	.053	5.20	6.23
1.20	.0257	.00038	.833	1.597	.00020	.72	3.848	13.27	.054	5.31	6.51
1.35	.0272	.00036	.907	1.611	.00020	.77	3.420	14.21	.054	5.34	6.69
1.50	.0287	.00034	.956	1.616	.00020	.82	3.078	15.08	.054	5.37	6.87
1.65	.0301	.00032	1.003	1.620	.00020	.87	2.798	16.00	.054	5.34	6.99
1.80	.0314	.00031	1.048	1.615	.00020	.91	2.565	16.95	.054	5.26	7.06
1.95	.0327	.00030	1.090	1.611	.00020	.95	2.368	17.88	.053	5.19	7.14

CROSS SECTION: MATEBA AVAL  
HYDRAULICS CALCULATIONS.. (CONTINUED)

UC (13)	H (14)	A (15)	P (16)	Q (17)	Q1 (18)	X2 (19)	Y (20)	BX (21)	BB (22)	PE (23)
.037	4.58	910.5	349.6	930.7	178.5	.001494	.243	1.671	.368	12.971
.044	6.54	1700.7	471.6	1928.8	521.7	.001036	.369	1.639	.391	13.535
.049	8.01	2453.1	561.9	2914.3	970.9	.000862	.471	1.597	.412	13.863
.053	9.17	3143.3	624.0	3792.8	1485.3	.000747	.554	1.558	.433	14.052
.055	10.17	3777.5	677.8	4637.4	2055.7	.000668	.626	1.524	.453	14.190
.057	10.80	4234.7	711.3	5232.0	2558.3	.000610	.684	1.493	.472	14.268
.059	11.33	4624.2	740.1	5731.3	3063.2	.000563	.729	1.464	.491	14.327
.060	11.79	4966.4	764.6	6211.0	3563.0	.000528	.768	1.439	.507	14.378
.061	12.12	5221.2	782.4	6574.3	4017.9	.000495	.803	1.418	.523	14.414
.061	12.44	5473.3	799.3	6867.3	4482.3	.000472	.823	1.396	.537	14.443
.062	12.65	5635.7	810.4	7223.1	4884.6	.000450	.840	1.376	.553	14.462
.062	12.78	5746.6	817.7	7373.2	5237.3	.000431	.845	1.356	.571	14.469
.063	12.93	5863.3	823.3	7573.4	5599.1	.000414	.849	1.338	.587	14.478

